

HYDROLOGY & HYDRAULICS REPORT

**Quinte Conservation Authority
And
The Township of South and Central
Frontenac**

**FHIMP ON22-46
For the
Napanee River Upper Lakes
Flood Hazard Mapping**

June 10, 2024



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1 Introduction

Quinte Conservation Authority (QC) has partnered with the Townships of South and Central Frontenac (the Townships) along with federal and provincial partners to lead the Napanee River Upper Lakes Flood Hazard Mapping.

With acquired funding through the federal Flood Hazard Identification Mapping Program (FHIMP), QC has undertaken a leadership role in the production of updated mapping for the Napanee River Upper Lakes. The objective is to provide a floodplain mapping update that will allow QC and the Township staff to make informed planning and regulation decisions. Jewell Engineering Inc. (Jewell) is pleased to support this initiative through the technical analysis and reporting described herein.

The driving forces for this project include climate change, improved modelling techniques and software programs, improved data acquisition tools, land use changes, and updated infrastructure that can dramatically influence flood behaviour and floodplain extents.

The need for accurate, detailed floodplain mapping that factors in climate change forecasting has become increasingly evident as flood damages become the largest cost to the Canadian economy out of any other natural hazard. Updated floodplain maps are needed to protect human life, property, and infrastructure from the damaging effects of flooding that is occurring with increased frequency.

The funds deployed by the federal and local governments to complete this updated floodplain mapping provide a dual benefit; it protects the local community from potential flood hazards *and* reduces the dependence on provincial and federal funds associated with the Disaster Financial Assistance Arrangements (DFAA) administered by Public Safety Canada.

This report is intended to describe the hydrologic and hydraulic analysis that was completed to produce the 2024 floodplain mapping set.

2 Background

The following background documents were reviewed in preparation of this Hydrology & Hydraulics Report.

The **1981 Napanee Salmon Upstream Lakes Watershed Study** prepared by Crysler & Lathem Limited was issued to establish fill line mapping for several lakes of interest in the upper reaches of the Salmon and Napanee watersheds. The 1981 study offers insight into previous hydrologic modeling and mapping methods. It includes hydrologic calculations for rainfall and snowmelt conditions, with outlet descriptions for particular lakes of interest, including those controlled by the Hardwood Dam. Additional controls, such as the storage-discharge for the Cameron Swamp and old Thirteen Island Lake Dam were also included. The hydrologic model in the 1981 study used TR20 software that is no longer of standard use among practitioners. The 1981 report served a useful purpose to the conservation authority and the 2024 floodplain mapping includes additional lakes within the Napanee River Upper Lakes system as well as updated outlet surveys, crossing surveys, and the application of current modeling software and technical guidance.

The **1978 Napanee River Basin Study** prepared by Crysler & Lathem Limited includes a “structural, economic, and environmental analysis of each dam and its headpond.” The report notes that the Cameron Swamp is a wetland that has significant impacts on the flows in the Napanee River (this was further confirmed with information in the 1981 study as well as the current hydrology review at the Camden East flow gauge). A fill line was included for the Cameron Swamp with the intent to ensure its environmental features and natural flood hazard controls receive maximum protection.

The **13 Island Lake Dam Hazard Potential Classification and Structural Design Report** was prepared by Jewell Engineering in September of 2022. This document was reviewed since the information describes the changes in the old dam at 13 Island Lake to the new dam. It describes that the old dam was constructed in 1975 to control summer lake levels for recreational purposes. The old dam was comprised of four (4) 1200mm steel culverts that were adapted to permit installation of four 8” high logs. Due to a loss of road granular materials and observations of failure of the steel culverts, three (3) 1.8m x 0.9m concrete box culverts were selected as the replacement structure after going through the public process. The normal operating water level behind the dam is 150.43m with 2 - 0.15m (6”) logs in place.

The **Dam Safety Review Report for the Colebrook Dam** was prepared by Hatch Ltd. in February of 2022. While outside of the subject study area for the Napanee River Upper Lakes floodplain mapping, the Dam Safety Review was investigated to determine the potential impacts it has on the local flow gauge at the Napanee River at Camden East station. The 1% AEP inflows and outflows for the Colebrook Dam are unchanged based on Table 3-1 of the Hatch report, indicating the Colebrook Dam would provide no appreciable flow attenuation in a regulatory level flood event. Therefore, flow attenuation impacts on the Camden East gauge are expected to be driven by the Cameron Swamp, Hardwood Dam, and several other outlet controls within the watershed that contributes to the Camden East gauge.

3 Study Area

The study area for the Napanee River Upper Lakes was outlined by QC at the beginning of the project and is reproduced in Appendix A-1.

The study area focuses on the major lakes within the watershed; this includes Potspoon Lake, White Lake, St. Andrew Lakes, Cole Lake, Thirty Island Lake, Thirteen Island Lake, Fourteen Island Lake, Little John Lake, Sigsworth Lake, Hambly Lake, Van Luven Lake, Howes Lake, and Verona Lake.

The study area also includes four (4) river branches that connect the Napanee River Upper Lakes to their confluence at Verona Lake. Runoff from the four river branches and thirteen (13) lakes are subject to flow controls from the Hardwood Dam immediately downstream of County Road 38 in Verona for low flow conditions, and the Cameron Swamp for moderate and high flow conditions.

The northwest branch includes the tributary that connects St. Andrew Lakes and Cole Lake before draining as Cole Creek through Godfrey towards Howes Lake and Verona Lake.

The north branch includes White Lake that outlets to White Creek. White Creek crosses north of Glendower before meeting the northeast branch at Thirteen Island Lake.

The northeast branch includes Potspoon Lake that drains in series to Thirty Island Lake and then Thirteen Island Lake. The north/northeast branches outlet from Thirteen Island Lake into Howes Lake, where it meets the runoff from the northwest branches of Cole Creek.

The east branch has relatively short channel sections as it includes Little John Lake, Sigsworth Lake, and Fourteen Island Lake, each of which are in close proximity to one another. The outlet of Fourteen Island Lake drains through two smaller lakes (Little Mud Lake and Spring Lake) before meeting the confluence with the other three main branches of the Napanee River Upper Lakes at Verona Lake.

The study area is evidently dominated by the thirteen lakes identified above. Each lake has an outlet control, whether man-made or natural, which encourages these lakes to act as a system of reservoirs and connector channels. The study area focuses on the water levels and flood hazard limits produced by the regulatory peak flows that contribute to these lake reservoirs and river channels.

4 Hydrology

The hydrology assessment was prepared for the lake outlets and several nodes of interest throughout study area. Various methodologies were applied and compared to determine representative peak flows. Each methodology was carefully considered prior to the selection of the peak flows for use in the hydraulic model, including potential increases in flows due to spring-melt conditions.

The Napanee River Upper Lakes watershed is within *Zone 2 of Flood Hazard Criteria Zones for Ontario Conservation Authorities*. Therefore, the flood standard is the 1% annual exceedance probability (AEP).

The detailed hydrologic analysis for the purpose of quantifying the peak flow rates is described below.

4.1 Data Sources

Data collection is an integral component of the hydrologic assessment. A description of each primary data source applied in the analysis is provided below.

4.1.1 LiDAR, Catchment Areas & Terrain

The subject watershed for the Upper Lakes has a total area of 148.5km².

The watershed is discretized into sixteen (16) sub-catchments based on confluence points, lake outlets, and dam locations. The catchment boundaries are identified in Appendix A-2.

External catchments beyond the subject watershed were included for calibration purposes to allow the hydrologic model results to be compared to flow gauge observations of the Napanee River at the Camden East station (see further discussion in Section 4.3).

Catchment areas were delineated using topographic information from the following sources.

- 1) Provincial LiDAR developed and published by *Land Information Ontario* was reviewed in combination with ESRI server data information to assist in delineation of the sub-catchment boundaries. The sub-catchment configurations are similar to those delineated in the 1981 Chrysler and Lathem study, however Jewell's assessment is more localized (i.e. more sub-catchments) and with updated LiDAR data. The 2023 catchment delineations included a detailed review of the contour information with GIS software extents set as dense as 0.5m contours in select locations areas to ensure the accuracy of the sub-catchment boundaries.
- 2) A site-specific topographic survey for lake outlets and hydraulic structures was completed to assist in the stage-storage-discharge relationship supplied to the hydrologic and hydraulic models.

4.1.2 Soils and Land Cover

A soils map is provided in Appendix B. Soils information was obtained from the Soil Survey Complex database produced by the *Ontario Ministry of Agriculture, Food and Rural Affairs* in cooperation with the *Ontario Ministry of Natural Resources and Forestry*.

The HSG classification for soils is used to identify drainage characteristics for various soil types. An excerpt from Chapter 8 of the *1997 MTO Drainage Management Manual* that describes drainage characteristics for each HSG is provided below.

The soils in the Napanee River Upper Lakes watershed are predominantly Hydrologic Soils Groups (HSG) B, as shown in Appendix B and Table 4-1.

Table 4-1: Napanee River Upper Lakes Overall Watershed Hydrologic Soils Group Summary

HSG Soils Group	Area (km ²)	Land coverage (%)
A	1.2	1
B	117.1	79
C	24.3	16
D	5.9	4

The hydrologic soil group is used to classify soils into groups of various runoff potential.

The Soil Conservation Service (SCS) classifies bare thoroughly wet soils into four hydrologic soil groups (A, B, C and D). SCS descriptions of the four groups, modified slightly to suit Ontario conditions, are as follows: (Design Chart 1.09)

- A: High infiltration and transmission rates when thoroughly wet, eg. deep, well drained to excessively-drained sands and gravels. These soils have a low runoff potential.
- B: Moderate infiltration and transmission rates when thoroughly wet, such as moderately deep to deep open textured loam.
- C: Slow infiltration and transmission rates when thoroughly wet, eg. fine to moderately fine-textured soils such as silty clay loam.
- D: Very slow infiltration and transmission rates when thoroughly wet, eg. clay loams with a high swelling potential. These soils have the highest runoff potential.

In Ontario, soils have been found to lie between the main groups given above, and have therefore been interpolated as AB, BC, CD as appropriate, such as Guelph loam, which is classified as BC.

Figure 4-1: Excerpt from 1997 MTO DMM Describing Hydrologic Soils Group Classifications

The soils data is used to develop curve numbers (CNs) that are a key modelling parameter used in the Soil Conservation Service (now known as the *National Resources Conservation Service*) methodology for estimating the proportion of precipitation that will runoff the lands and the portion that will infiltrate. CNs are a function of soil type, land cover, slope, and land use. The higher the CN – the greater the proportion of precipitation that is expected to runoff the lands. CNs are representative of the pervious portion of the watershed. Jewell followed the guidance in *MTO Design Chart 1.09* to determine curve numbers for the discretized catchments.

Land cover information was obtained from the Ontario Land Cover Compilation (OLCC), a database owned by *Land Information Ontario*, and provided by the *Ontario Ministry of Natural Resources and*

Forestry. A review of land coverage for the watershed shows that the land use is predominantly woods, water, and cultivated land. A summary of land coverage percentage is provided in Table 4-2 below.

Table 4-2: Napanee River Upper Lakes Overall Watershed Land Cover Summary

Land Cover	Area (km ²)	Land Coverage (%)
Woods	84.13	57
Cultivated	22.77	15
Urban	1.69	1
Water	30.90	21
Bedrock	9.04	6

4.1.3 Meteorologic Inputs

Environment and Climate Change Canada (ECCC) intensity-duration frequency (IDF) curves for data collected at the Kingston station is the best available data record (see Appendix F). Jewell reviewed the rainfall data at Kingston, Tweed, Smith Falls, and Hartington. The Tweed and Smith Falls records do not have a sufficient duration with only 21 years of data. The Hartington station has a sufficient length of data record at 45 years, however it does not have published IDF curves. The Hartington station *does* have snowmelt plus rainfall frequency values which were applied in the AEP snowmelt events (see Appendix C-1). It is also noted that the rainfall frequency values for the Hartington station have a published 1% AEP, 1-day rainfall total of 92.1mm.

The Kingston station is the closest station that has a set of IDF curves *and* a sufficient length (63 years) of record. The Kingston station has a 1% AEP, 1-day rainfall total of 97.1mm. This is greater than the 24-hr total from the Hartington station that is closer to Verona, suggesting the Kingston IDFs offer a conservative estimate of rainfall depths for the subject region.

Additionally, Quinte Conservation (QC) provided precipitation records for use in the rainfall-runoff model calibration and validation process. QC provided records at the 2nd Depot, Camden, and Tamworth rainfall gauges. Since the precipitation at these gauges is not verified by ECCC and does not have standardized sensor selection, calibration, and placement under the control of ECCC, the *distribution* of the rainfall gauges was the primary interest from the received rain gauge data set. The rainfall totals from the QC records were compared to the total depth observed at the Environment Canada Hartington station.

ECCC provided stream flow gauge data with shorter (5-minute) time intervals than the publicly available daily records for the Napanee River at Camden East station (02HM007). The discharge values are part of the Water Survey of Canada's primary products and considered a reliable data source.

Major rainfall events occurred locally in May of 2017 and November of 2006. These rainfall events were used to calibrate and validate the rainfall-runoff model. The rainfall record for each of these events is shown in Figures 4-2 and 4-3. These events are discussed further in Section 4.3.

An important consideration in the precipitation data is the potential impacts on rainfall depths due to climate change. QC, in partnership with FHIMP representatives, provided the recommended approach to quantify increased rainfall depths due to climate change. This approach is completed by scaling rainfall intensities based on projected temperature increases. The methodology, rainfall depths, and impacts on peak flows are discussed further in Section 4.6.

Figure 4-2: Rainfall Data Applied in May 2017 Calibration Event for Rainfall-Runoff Model

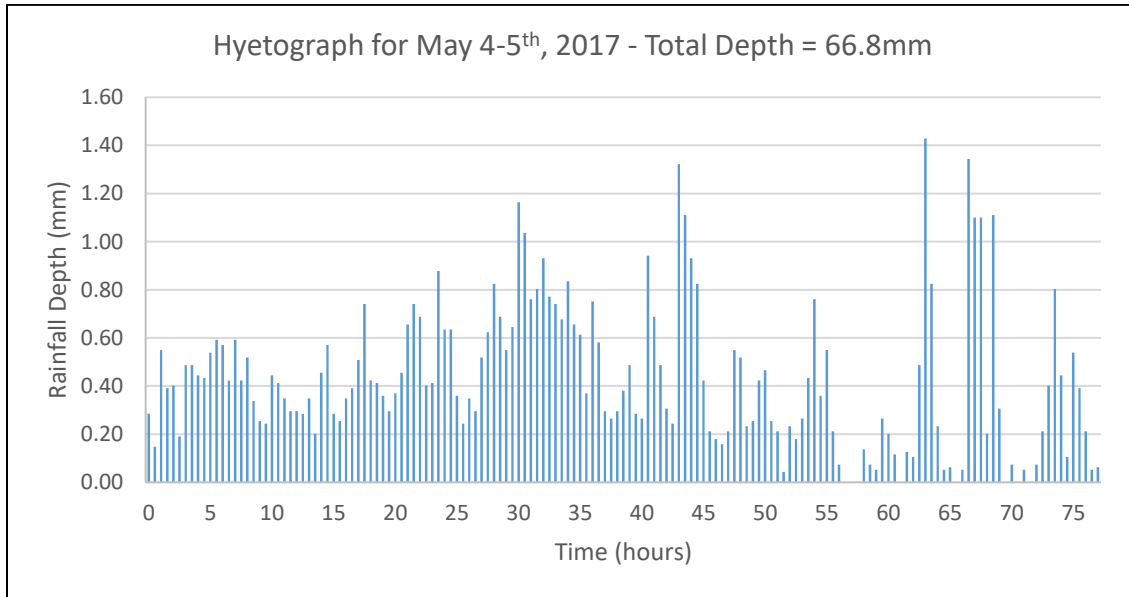
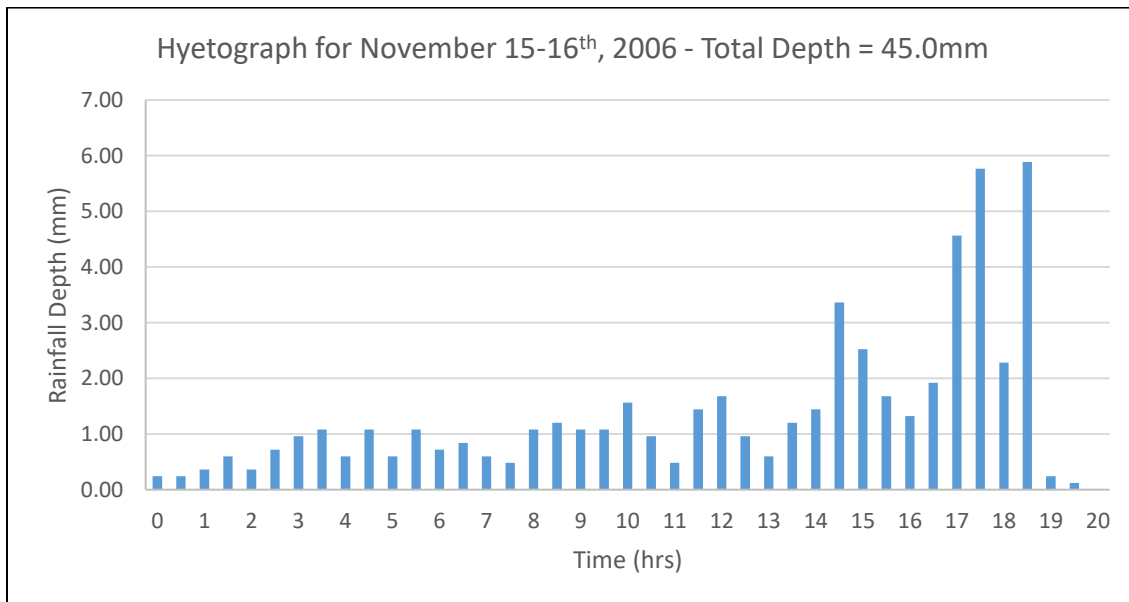


Figure 4-3: Rainfall Data Applied in November 2006 Validation Event for Rainfall-Runoff Model



Jewell participated in recent discussions with ECCC staff regarding precipitation statistics. As part of these discussions, Jewell acquired and reviewed the ECCC precipitation statistics tool. This review confirmed Jewell's in-house spreadsheet is consistent with the ECCC methodology. Jewell's in-house precipitation tool was used to determine the 0.5% AEP statistical rainfall since this AEP is not included in the standard Environment Canada IDF curves. The spreadsheet calculates the precipitation frequency curve using a Gumbel distribution.

The recommended AEP storms for floodplain mapping are derived from the SCS distribution based on the 2002 *MNR Technical Guide for Ontario Flood Hazard Limits* with varying durations. In an assessment of the critical AEP storm, Jewell compared peak outflows from a HEC-HMS model (see Section 4.3) for the 12 and 24-hr duration events. Any event less than 12 hours is not recommended for the Napanee River Upper Lakes since shorter duration events do not produce significant enough rainfall volumes to govern as the regulatory storm event. This is particularly pertinent to the subject watershed as the storage in the reservoirs, and subsequently the peak outflows from the lakes, will be governed by high runoff volumes that occupy storage within the lakes and create a driving head on the outflow controls that drain to the receiving channels.

In the rainfall simulations, the 24-hr duration produced the largest peak runoff rate. Therefore, the 24-hr duration was considered the critical storm for the rainfall-runoff model. A comparison of statistical rainfall volumes for the 12-hr and 24-hr durations is provided in Table 4-3.

Table 4-3: Runoff Volumes for 12- and 24-Hr Storm Durations for Kingston IDF Curves

AEP	Rainfall Volume (mm)	
	12-Hr	24-Hr
50	41.2	47.1
10	63.8	69.3
1	92.0	97.1
0.5	99.7	105.3

4.1.4 Water Survey of Canada Stream Flows

There is a stream flow gauge located along Napanee River at *Water Survey of Canada (WSOC) Station 02HM007* titled 'Napanee River at Camden East'.

The flow data of interest is the *Annual Maximum Instantaneous Peak Discharge*. The record length for the gauge is from 1974 to 2022, with 46 years of annual instantaneous maximum peaks.

Discharge data was received from ECCC in 5-minute intervals for use in the calibration and validation model runs. The Depot Creek at Bellrock (02HM002) WSOC flow gauge was also applied in the calibration/validation process. Table 4-4 and Figure 4-4 summarize the station data and gauge locations.

Table 4-4: List of Local Stream Flow Gauges for Extended Data Record

Name	Station ID	Length of Record	Gross Drainage Area (km ²)
Depot Creek at Bellrock	02HM002	1957 - 2023	203
Napanee River at Camden East	02HM007	1974 - 2022	718
Napanee River at Hardwood Dam	N/A	N/A	149

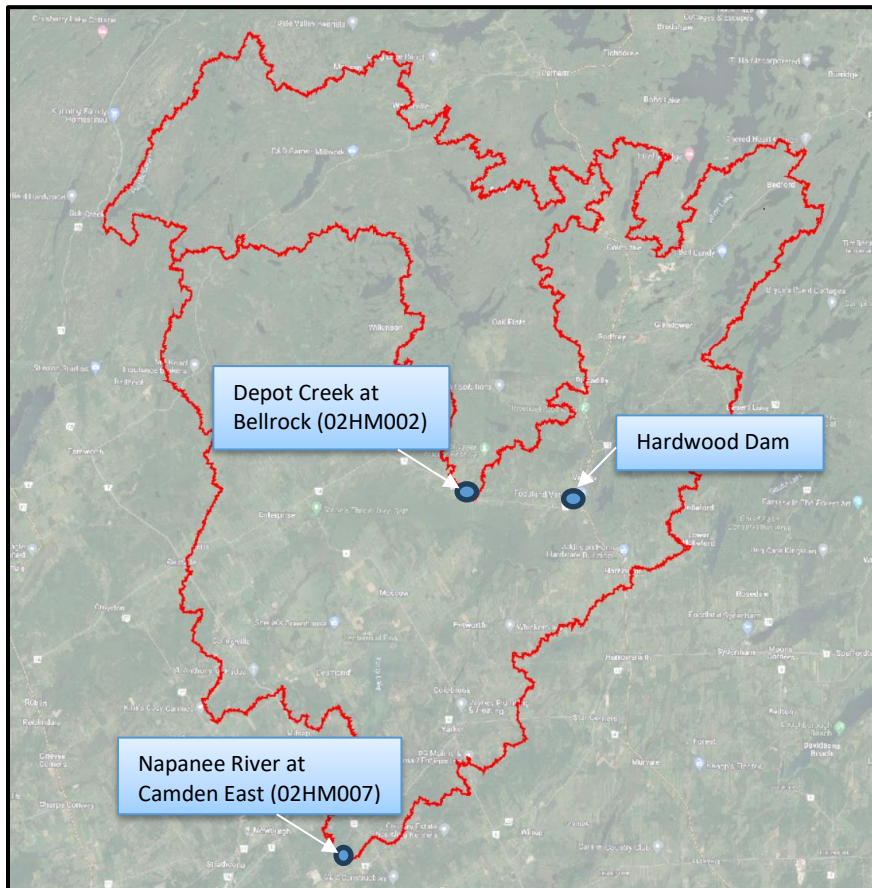


Figure 4-4: Napanee River at Camden East Water Survey of Canada Stream Flow Gauge Locations

4.2 Flood Frequency Analysis

A primary benefit of updated hydrologic models is that they can be calibrated to observed stream flow gauge data that has occurred since the previous hydrologists completed their assessment of the peak flows. While models are a simplification of the complex hydrologic system, the flow gauge provides the true answers. Fortunately, the WSOC follows strict procedures to ensure the flow observations are accurate and suitable for use in statistical analysis programs. The previous study of the Upper Napanee Lakes occurred in 1981. The *Napanee River at Camden East* WSOC flow gauge has records from 1974 to present. Therefore, 40+ years of “new” flow data has been incorporated into this floodplain mapping update.

The flood frequency analysis (FFA) was completed using the Canada Frequency Analysis program. This modeling approach employs stream flow gauge records into the hydrology results similar to the HEC-SSP (Statistical Software Package) software. The statistical software program was applied to calculate the AEP flows using a Three Parameter Log Normal distribution as recommended in previous discussions with ECCC. In Ontario, the Three Parameter Log Normal distribution can be applied with the maximum annual instantaneous peak data as the preferred data set.

For the purposes of obtaining AEP flood flows, the General Frequency Analysis (GFA) component can be employed and is a recommended method in the 2002 MNR guidelines. Parameters other than peak flows, such as stage or precipitation data, can also be calculated using a GFA.

In performing a GFA, data is provided to the program and the calculated results are output in graphical and tabular formats. Prior to providing input data, a variety of settings are defined by the user. Some notable settings and their descriptions are shown in Table 4-5.

From an assessment of the stream flow records, it is evident that the majority of the annual instantaneous peaks occur in the spring. For the 46-yr data record of annual instantaneous peaks at the WSOC Camden East flow gauge, only three (3) years had instantaneous peaks outside of December 30th to May 1st. This suggests a 93% probability that a future flood event would be the result of a snow-melt event, or a combination of a snow-melt and rainfall event. Therefore, the stream flow gauge records provide the best indication of the anticipated flow rates produced at the Camden East station in a snow-melt plus rain scenario.

The CFA program results for the respective AEPs are summarized in Table 4-6. For AEP flows at the downstream limit of the Napanee River Upper Lakes study area, a transposition of flows is required (see Figure 4-5). The transposed flows are useful for comparison purposes, but due to the large difference in areas subject to the transposition equation, further investigation is recommended. This further investigation is described in the following subsections (Sections 4.3 and 4.4). A transposition of flows to the Cameron Swamp outlet was also completed since the Cameron Swamp is the downstream limit of the hydrologic model used in the calibration simulations.

Transposition and interpolation of data from a stream gauge can be done based on the Modified Index Flood method as follows:

$$Q2 = Q1 [A2 / A1]^{0.75}$$

Where Q1 = known peak discharge

Q2 = unknown peak discharge

A1 = known basin area

A2 = known basin area of unknown peak discharge

Figure 4-5: Excerpt from MTO Online Drainage Manual

Table 4-5: HEC-SSP Settings and Descriptions

Setting	Description
Log Transform	<ul style="list-style-type: none"> ○ This setting can be selected to have the frequency analysis performed on the logs of the data ○ Log Transform needs to be used to allow for the LogNormal and LogPearson III distributions to be selected ○ If the Log Transform setting is not used, the Normal and Pearson III distributions can be selected
Confidence Limits	<ul style="list-style-type: none"> ○ Confidence limits measure the uncertainty of the computed value for a selected exceedance probability ○ Default settings calculate the 90% confidence interval, with confidence limits of 0.05 and 0.95
Distribution	<ul style="list-style-type: none"> ○ This setting provides the analytical distribution options used to perform the frequency analysis ○ Distribution choices are None, Normal, LogNormal, Pearson III, and LogPearson III
Generalized Skew	<ul style="list-style-type: none"> ○ Computes a skew value for the data ○ Three options that can be selected are Station Skew, Weighted Skew, and Regional Skew ○ The default option is Station Skew, where the skew of the computed curve is based solely on computing a skew from the provided data points

Table 4-6: Summary of Maximum Peak Flows from HEC-SSP General Frequency Analysis

AEP (%)	Peak Flow (m ³ /s)		
	Camden East (718 km ²)	Hardwood Dam (149 km ²)	Cameron Swamp (670 km ²)
50	43.2	13.2	41.0
10	61.5	18.9	58.4
1	80.7	24.7	76.6
0.5	86.0	26.4	81.7

4.3 Rainfall-Runoff Modeling

The SCS Curve Number (CN) method is commonly applied in hydrology models for precipitation-driven runoff modeling applications. It relies on the soils and land use data to establish the loss method with calculation of a CN. The modeling approach is supported by Visual OTTHYMO (VO) and HEC-HMS.

All modelling programs are simplifications of reality and are limited in their capabilities. While VO and HMS are both well-established and recommended software programs, they are limited by input parameters and the uncertainty associated in the data sets and calculations used to produce these inputs. Both modelling programs are acceptable for simulating peak flows to be used in the hydraulic model. The most recent software publications have been used for this project.

While VO was initially selected, HEC-HMS was ultimately preferred due to its use of timestamps in the model runs. The timestamp feature is useful in calibrating the hydrologic model since it allows the user to compare timing of the hydrograph peaks. The objective is to ensure the hydrograph peaks, start time, and lag times are similar in both the observed results and modeled results. Shape files for land cover and soil type prepared in GIS software were applied to develop the attribute tables used to calculate the weighted CN value for each sub-catchment. This replicates the GIS features available in Visual OTTHYMO. The shape files are included in the final deliverables package.

Notable input parameters for the HMS model include:

- Precipitation – intensity, duration and frequency as well as distribution.
- Catchment area.
- Percent imperviousness – runoff volume, time to peak and peak flow increase with percent imperviousness.
- Soil conditions – these determine how much and how quickly water will be removed from runoff through infiltration. This may be expressed as a curve number, or by a runoff coefficient or using an infiltration model such as Horton’s Infiltration.
- Slope – peak flows increase with slope.
- Initial abstraction – depth of precipitation input that is subtracted from the model and does not contribute to runoff.
- Manning’s n – frictional coefficient that affects the time to peak.
- Basin lag or time to peak.

The peak flows simulated in HMS for each storm event at each lake outlet and major road crossing are summarized in Subsection 4.3.8 that follows.

It is noted that the urbanization in the subject watershed is predominantly limited to cottages and the community of Verona. Future development is not expected to yield an appreciable impact to the hydrologic findings described herein.

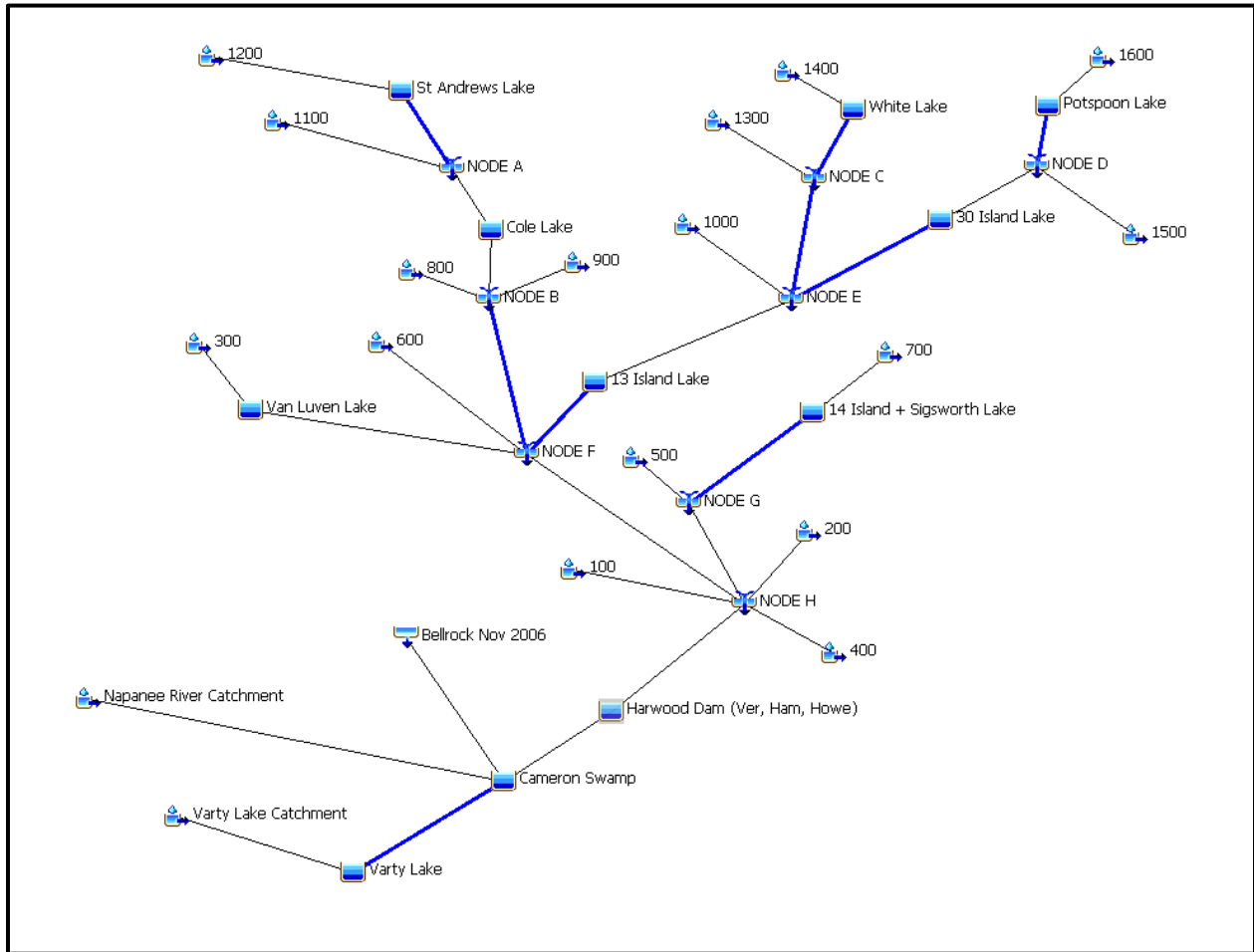


Figure 4-6: HEC-HMS Model Schematic for Initial Calibration

A consideration in the hydrology model is the starting water levels at or above the invert of each individual lake outlet. In the absence of a sufficient record of stage data for each lake, the best available data set is the stream flow record, which has 43 of its 46 annual instantaneous peaks occurring in the snowmelt period. The annual instantaneous peaks in the GFA include all data that contribute to the flows, including baseflows and flows that occurred prior to the main snowmelt event.

An alternative measure that was included in the HEC-HMS model is the near-zero initial abstraction value (0.1mm) that was applied in both the snowmelt and rainfall-runoff models. Whereas initial abstraction can range from approximately 2 – 15mm based on the default parameters in HMS, setting this value to 0.1mm essentially omits the initial precipitation removal and encourages immediate runoff

that simulates early depths of water above the invert prior to the peak that occurs later in the runoff event. While the ideal condition would be to have a long record of stage data for each lake, the above approach provides confidence that the amount of water level rise above the invert would error on the side of caution.

4.3.1 Calibration

The rainfall model was calibrated using the event that occurred in May of 2017. This event was selected as the calibration storm since it produced the largest annual instantaneous peak outside of the spring melt season. It also produced the seventh (7th) highest annual instantaneous peak out of the 46-year data record. The other six (6) were produced from spring melt events that are discussed further in Section 4.4.

The measured precipitation data (see Figure 4-2) was supplied as a meteorologic input to the HMS model. The objective is to obtain an outflow hydrograph from the HMS model that produces similar peak flow and lag time values to the measured flow gauge hydrographs.

The measured flow gauge hydrograph is shown in Figure 4-8. On May 9th of 2017, the stream flow gauge recorded its maximum peak flow of 58.9 m³/s.

It is important to note that in the spring of 2017, runoff was released from the snowpack by April 8th and majority of the runoff had subsided by April 30th based on a review of historic climate data and the local snowmelt model created by Dr. H. O. Schroeter (see further snowmelt discussion in Section 4.4). The May 2017 rainfall event began shortly after the spring melt season on the evening of May 4th. Therefore, antecedent moisture condition (AMC) III was selected for the model calibration to account for saturated ground conditions. The flow measurement preceding the rainstorm is considered baseflow for the purpose of the calibration, and is subtracted from the flow readings associated with the May 4-5th rainfall event; the baseflow is 24.4 m³/s. With the flow gauge peak of 58.9m³/s, subtracting the baseflow yields a *target peak flow* rate of 34.5 m³/s.

Per discussions with federal representatives on similar local FHIMP projects, the curve number (CN) is the primary input parameter subject to calibration. Lag time is also an important consideration, and the lag times of the calibrated model were adjusted to provide consistency with the *measured lag times*.

Ideally, a flow gauge would be present at the downstream limit of the study area near the Hardwood Dam. A unique component of the Napanee River Upper Lakes is that the study area terminates *prior* to, or upstream of, the available flow gauge location.

Given the flow gauge location downstream of the study area, an alternative calibration approach would be a regional frequency analysis. Since the lake outlet configurations for the Upper Napanee Lakes (several of which are man-made in the form of dams, weirs, or culvert crossings) are different than the controls (if any) located upstream of the stream flow gauges at adjacent watersheds, the regional approach was not selected. Rather, the local Upper Napanee River system and flow gauge results were applied in the calibration process (i.e. a single station approach).

In an effort to accommodate the “unknowns” between the Upper Lakes study area and the Camden East flow gauge, Jewell reviewed the watershed characteristics and flow controls for the downstream

catchment areas that contribute to the flow gauge location. These “external” catchments are listed below and shown in Figure 4-7.

1. **Catchment Ext-1** represents the 203 km² drainage area that contributes to the Depot Creek at Bellrock flow gauge located downstream of the Depot Lake outflow control structures. This runoff drains to the Cameron Swamp and ultimately to the flow gauge at Camden East. A benefit of the Bellrock flow gauge is that its data record provides known values into the calibration events for 28% (203/718) of the total catchment area that drains to the Camden East gauge. This increases the accuracy of the calibration for the Upper Napanee Lakes watershed by reducing uncertainties in the external catchments that contribute to the model calibration run. The flow records for the Bellrock gauge were input directly into the calibration model via an inflow hydrograph as a Source element in the HMS model.
2. **Catchment Ext-2** represents the 283 km² drainage area outside of Catchments Ext-1 and Ext-3 that drains to the Cameron Swamp. The storage-discharge relationship for the Cameron Swamp was obtained from the Crysler & Lathem study and included in a reservoir-routing simulation. There is significant flow attenuation from the Cameron Swamp due to the large storage volume within its reservoir. The CN for this catchment was calculated with the same methodology applied to the Upper Lakes sub-catchments. The lag time for Catchment Ext-2 however was calculated using the *measured* lag time. Lag time is defined as the time separation from the centroid of the rainfall to the centroid of the outflow hydrograph. Therefore, it was back-calculated from the precipitation and stream flow gauge records for the subject rainstorm.
3. **Catchment Ext-3** represents the 23.1 km² drainage area that contributes to Varty Lake and outlets to the Cameron Swamp. The storage-discharge relationship for the Varty Lake outlet was obtained from the Crysler & Lathem study and included in the reservoir-routing of Varty Lake.
4. **Catchment Ext-4** represents the drainage area between the outlet of the Cameron Swamp and the flow gauge at Camden East. This catchment is different than the others in that its control structures are not expected to provide any significant flow attenuation. The main control structure within Catchment Ext-4 is the Colebrook Dam. The inflows and outflows for large storm events at the Colebrook dam were presented in the 2022 Colebrook Dam Safety Study by Hatch Limited. The document showed that no appreciable flow attenuation is provided by the Colebrook Dam in a large flood event. Since Catchment Ext-4 represents a small (7%) portion of the catchment to the Camden East gauge and does not receive substantial flow attenuation, it was not included in the calibration model. Therefore, the downstream limit of the calibration model is up to and including the outlet from the Cameron Swamp.

The subject watershed for the total Upper Napanee Lakes is 148.5 km². The contributing area to the flow gauge at Camden East is 718 km². The objective of the “external” catchment review and calibration is to assess the suitability of standard hydrology inputs for the Upper Lakes sub-catchments. If the model produces lower or higher values than observed, then the input parameters can be adjusted accordingly. Since the external catchments are similar to the Upper Lakes watershed with significant lake/wetland coverage and significant outflow controls, adjustments made to the external catchments can reasonably be applied to the Upper Lakes sub-catchments. Although the Upper Lakes total watershed is only 21% of

the catchment area to the *Napanee River at Camden East* WSOC gauge, the known values at the Bellrock Creek flow gauge, combined with the storage-discharge relationships for Varty Lake and Cameron Swamp, allows for an accurate representation of the flows for the Upper Lakes system.

The HEC-HMS model compares well with the observed flow in the May 2017 calibration event (see Table 4-7). The *validation* component is described in the following subsection.

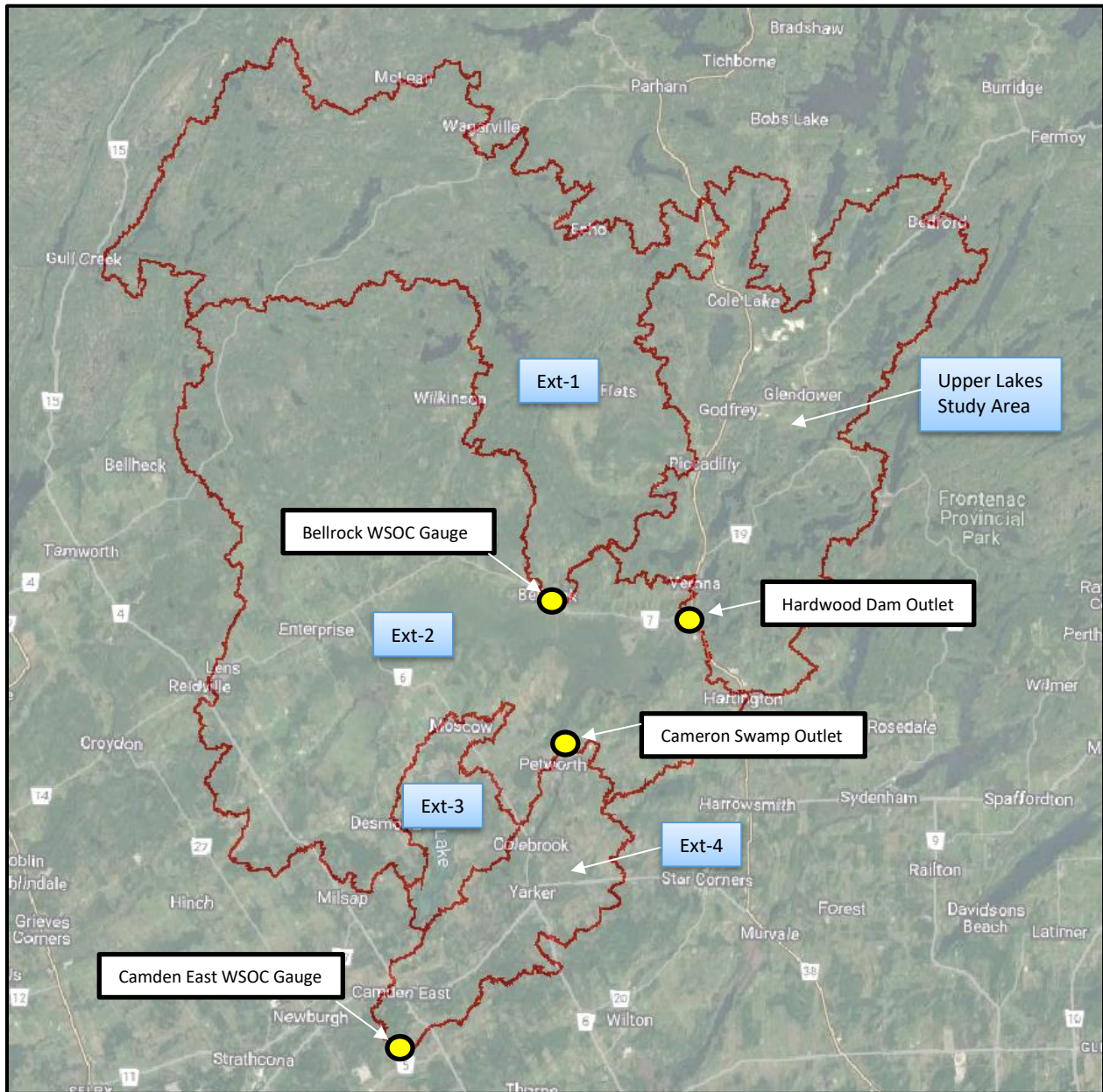


Figure 4-7: "External" Catchments Applied in Calibration Model

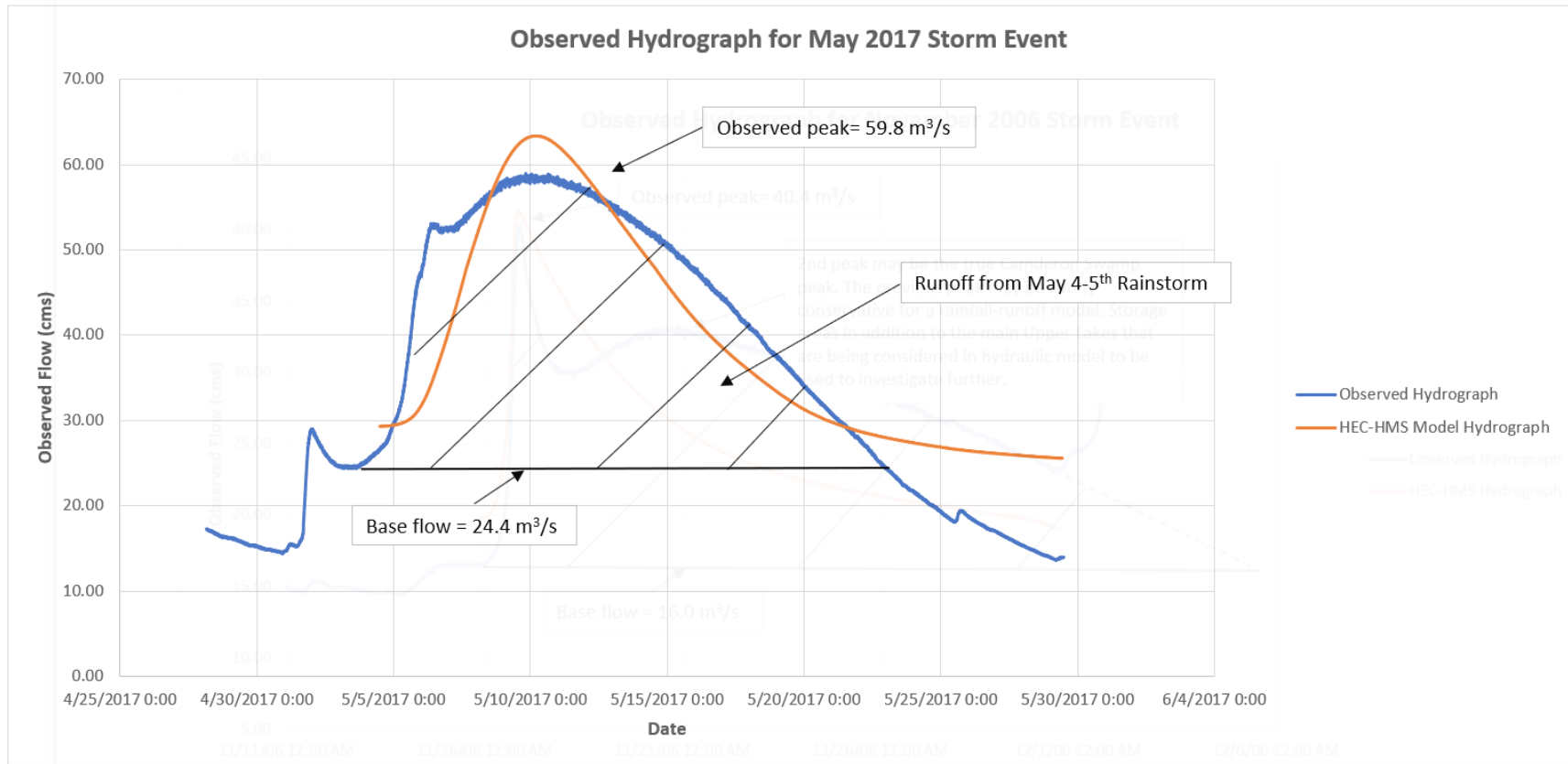


Figure 4-8: Observed Hydrograph for May 2017 Calibration Event

4.3.2 Validation

The precipitation event that primarily occurred on November 15th and 16th of 2006 was selected as the validation event since it is the next largest annual instantaneous peak flow outside of the May 2017 calibration storm. This approach is consistent with the 2002 MNR Technical Guide that recommends model testing be performed on the largest available storm events.

The flow gauge readings prior to the rain event are considered baseflow for the purpose of the calibration, and subtracted from the flow readings associated with the November 15-16th rainfall event; the baseflow is 16.0 m³/s. With the flow gauge peak of 40.4 m³/s, subtracting the baseflow yields a target peak flow rate of 24.4 m³/s.

The calibrated parameters were applied to the validation event; the only exception is a conversion of CN values from AMC III to AMC II since the validation event occurred in late fall and was not subject to the same saturated ground conditions as the calibration event that occurred in May of 2017. In the AMC II scenario, CNs were increased by 7% to better replicate the peak flows observed in the validation event.

A comparison of peak flow results between observed and modeled outputs for the calibration and validation storm events confirm the hydrologic model yields a realistic representation of the flows produced by a large rainfall-runoff event measured at the Cameron Swamp, and by extension, the Napanee River Upper Lakes study area. Therefore, the same rainfall-runoff model parameters, with AMC II + 7% CN values, were selected for the statistical AEP events.

Table 4-7: Measured vs. Modeled Peak Flows for Rainfall-Runoff Storm Events

	Calibration Storm		Validation Storm	
Moisture Condition:	AMC III		AMC II + 7%	
Rainfall Depth (mm):	66.7		45.0	
	Measured	HEC-HMS	Measured	HEC-HMS
Peak Flow (m ³ /s)	34.5	38.4	16.8	17.6
Hydrograph Volume (ha-m)	1,883	1,732	1,425	1,114
Time of Peak	5/9/2017 20:00	5/9/2017 15:45	11/20/2006 21:00	11/20/2006 22:00
Lag Time (hrs)	106	103	108	109

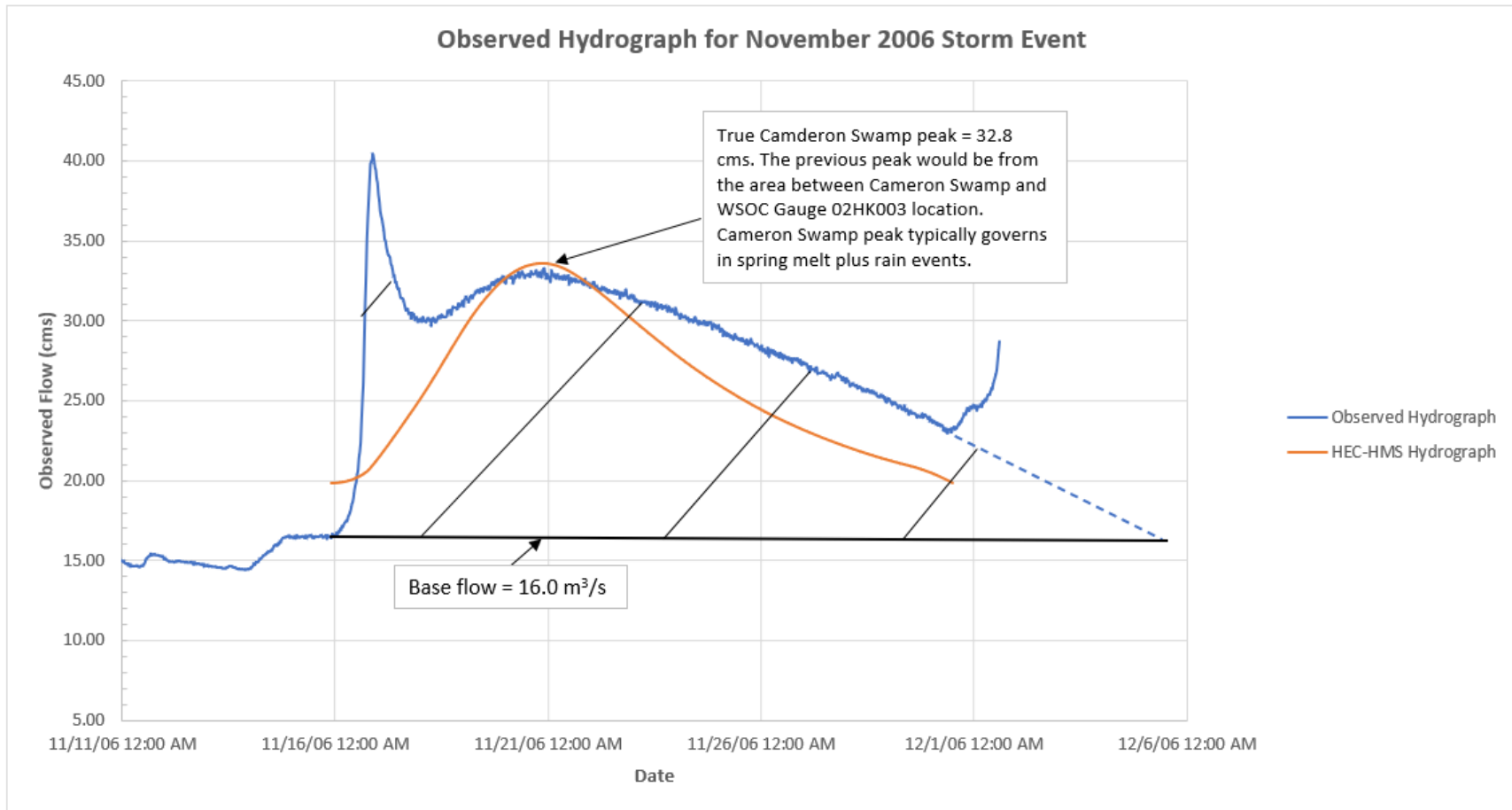


Figure 4-9: Observed Hydrograph for November 2006 Validation Event

4.3.3 Loss Method

The curve number (CN) loss method was selected since it accounts for both land cover and hydrologic soils group information. It was also selected because of the reputable sources available for this information. CNs were selected based on guidance from the CVC SWM guidelines in addition to MTO Design Charts. A look-up table was used to connect each land cover sub-area to its corresponding soil type. Attribute tables in GIS software applications were utilized to develop the detailed weighted CN applied to each sub-catchment.

AMC II, per Chapter 8 of the MTO Drainage Manual, was applied for antecedent moisture conditions (AMC) in the rainfall-runoff model. This represents 'average' soil conditions and AMC II values were increased by 7% based on the calibration/validation process. AMC II was used in the rainfall-driven AEP peak flow estimates as presented in Section 4.7. Saturated soil conditions (AMC III) were not selected for the rainfall-runoff event because this condition, combined with the statistical AEP rainfall events, would produce an event beyond the selected AEP frequency. Saturated conditions are applied separately in the snowmelt plus rain basin model that has been prepared to address spring melt conditions.

The individual basin inputs for the CN value were calculated with 28 combinations of HSG and land cover. For example, HSG A coverage could correspond with a woods, meadows, cultivated, lawns, impervious, or water land cover. The same process is repeated for HSGs B, C and D to develop the individual weightings for each sub-catchment. A brief description of each sub-area is provided in Appendix A-2.

4.3.4 Lag Time

Jewell applied the SCS Lag Time method to determine time of concentration and lag time values. This method was selected since it is derived from a study of watersheds that have drainage areas up to 24 km² with an upper limit of approximately 50 km². The sub-catchments within the Napanee River Upper Lakes are within this upper limit. The largest sub-catchment within the study area is 16 km². The SCS lag time method was also selected because it accounts for land cover and soil types by incorporating the CN value to estimate a retardance factor. The SCS lag time method is described in the *Hydrology National Engineering Handbook* published by the United States Department of Agriculture and the Natural Resources Conservation Service.

The individual lag time calculations were determined using the longest flow path and the mean overland slope specific to each sub-basin. The longest flow path was measured using LiDAR topography. The mean overland slope is higher than the channel slope as it includes direct runoff from the sub-basins before they reach the generally shallow sloping channels.

The lag times for the large external catchments in the calibration and verification models were adjusted based on observed precipitation and flow gauge recordings. The lag time is defined as the time differential between the centroid of the precipitation and the centroid of the runoff volume, which can be measured when these inputs are available.

4.3.5 Channel Routing

Channel routing was completed using the Muskingum-Cunge method for the initial model calibration. This method is applicable for reaches with relatively small slopes. The Napanee River Upper Lakes watershed has an average channel slope of 0.2%. This routing method allows the user to input a cross-section to represent the ground surface data for the channel and overbank areas. Cross-sections were obtained from the terrain data and then simplified into an eight-point cross-section.

The Muskingum-Cunge method was also selected since it incorporates Manning's n values to represent expected roughness for the channel and overbank areas. The applied Manning's n values are based on the design charts in the *MTO Drainage Manual*.

4.3.6 Reservoir Routing

Reservoir routing was included in the hydrologic model to account for the thirteen (13) lakes of interest. The storage-discharge relationship for each reservoir was established based on field survey, GIS applications, and a review of background documents. The field survey is used to identify the elevation and size of the outlet structures, the GIS applications are used to establish the footprint area and volume of the reservoir, and the background documents identified the storage-discharge relationship for reservoirs beyond the subject study limits for their use in the calibration model.

The storage-discharge for Cole Lake is presented in Figure 4-10. The governing control is a beaver dam downstream of the natural Cole Lake outlet. The beaver dam is approximately 15m wide and is simulated using broad-crested weir flow.

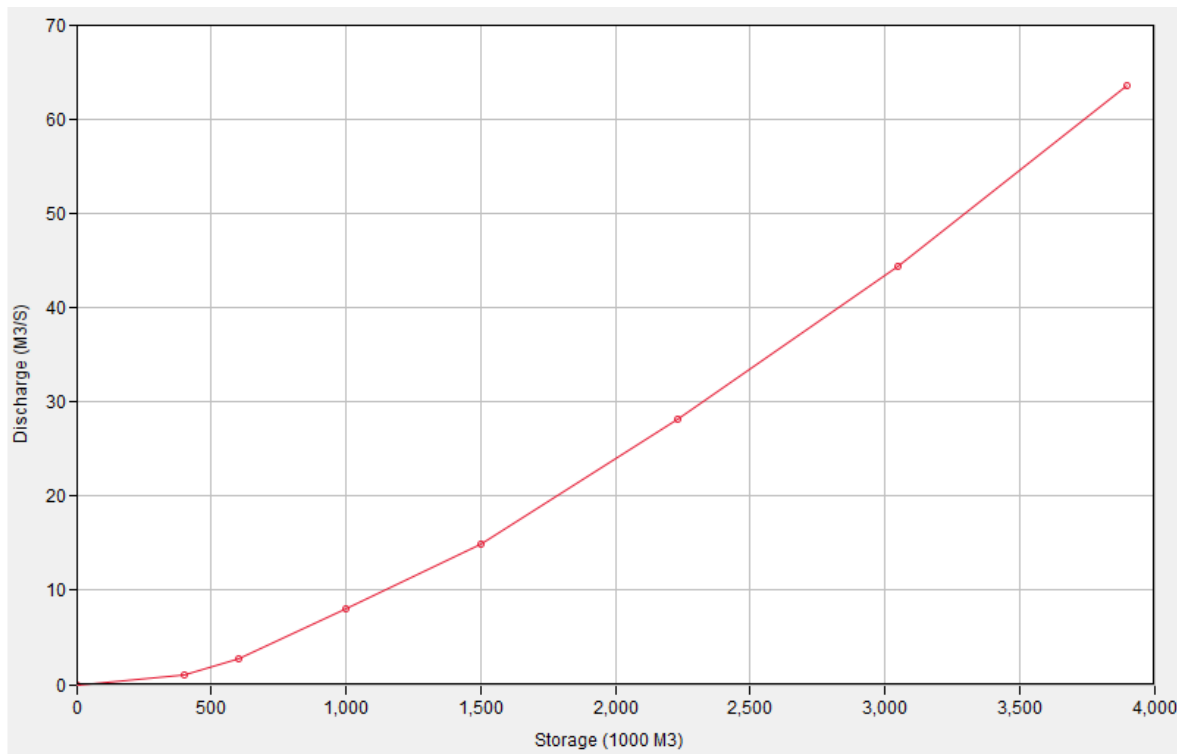


Figure 4-10: Cole Lake Storage-Discharge Relationship

The tailwater at the Hardwood Dam, imposed by the Cameron Swamp (see Section 5.1.3) controls the water levels in Verona, Howes, Van Luven and Hambly Lakes. The rating curves at the Cameron Swamp and Hardwood Dam outlets are provided in Figure 5-2 of the *Hydraulics* section of this report.

Potspoon Lake is controlled by a 6m weir and its storage-discharge relationship is shown below.

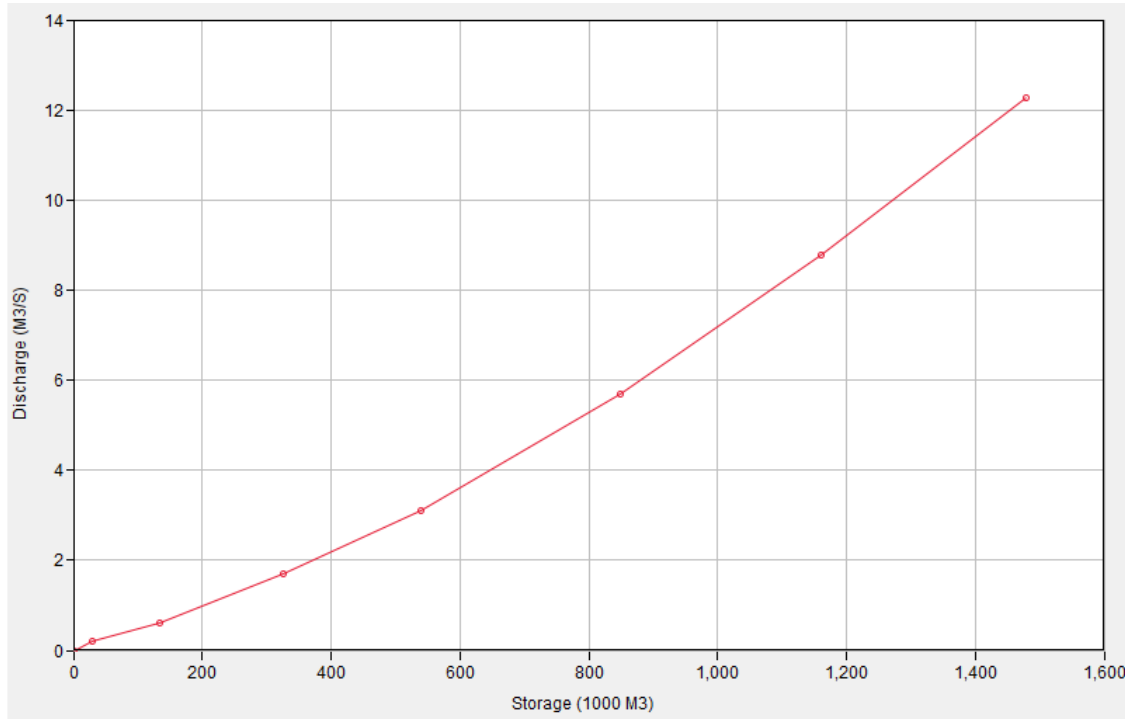


Figure 4-11: Potspoon Lake Storage-Discharge Relationship

The St. Andrew Lakes outflow is limited by the culvert crossing at St. Andrew Lakes Lane. Three (3) 750mm diameter pipes and a relatively flat roadway were analyzed to prepare the storage-discharge relationship presented in Figure 4-12.

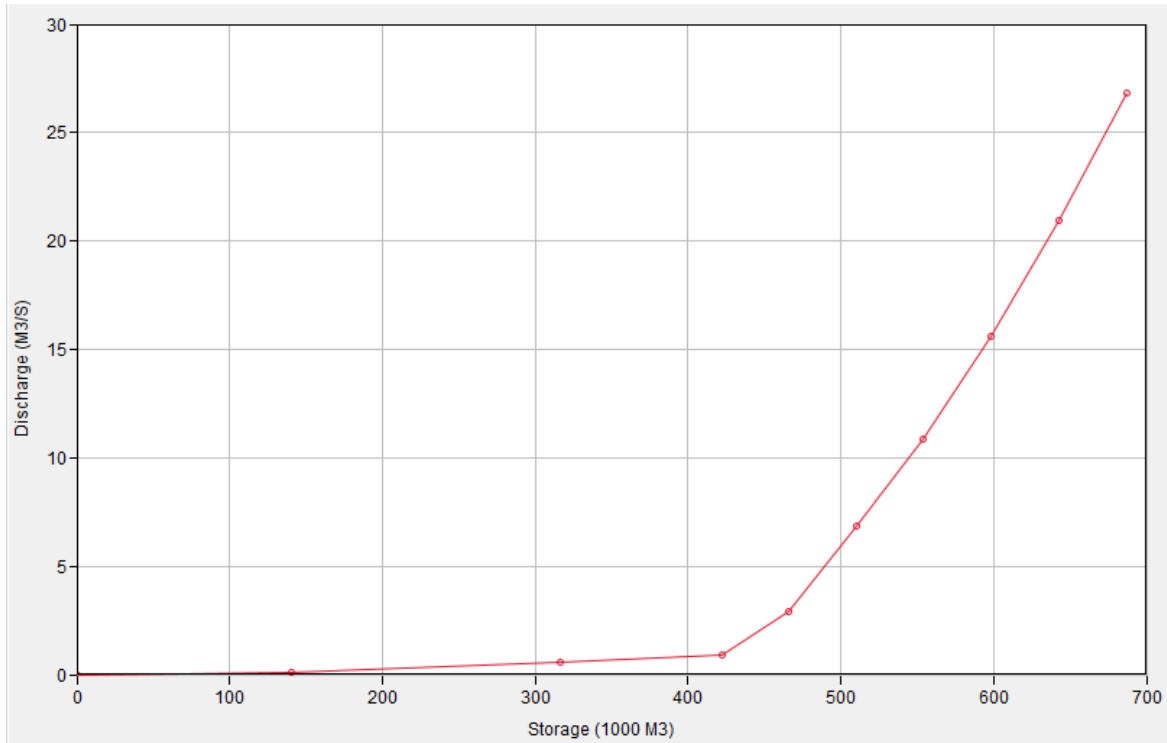


Figure 4-12: St Andrew Lakes Storage-Discharge Relationship

Van Luven Lake was assumed to be controlled by the Cameron Swamp & Hardwood Dam outlet in the 1981 study. However, a review of LiDAR data and 2023 field survey indicates that Van Luven Lake is controlled by the concrete box culvert crossing at County Road 38 before it drains into Howes Lake. The backwater impacts from the Cameron Swamp do influence the water levels in Van Luven Lake and subsequently the water levels for Van Luven and Howes Lake will be the same in large storm events. However, the headloss imposed by the box culvert could result in slightly higher water levels in Van Luven Lake relative to Howes Lake in smaller, more routine rainfall events or in standard summer water level conditions. The box culvert and road profile were included in the hydraulic calculations to prepare the storage-discharge relationship in Figure 4-13.

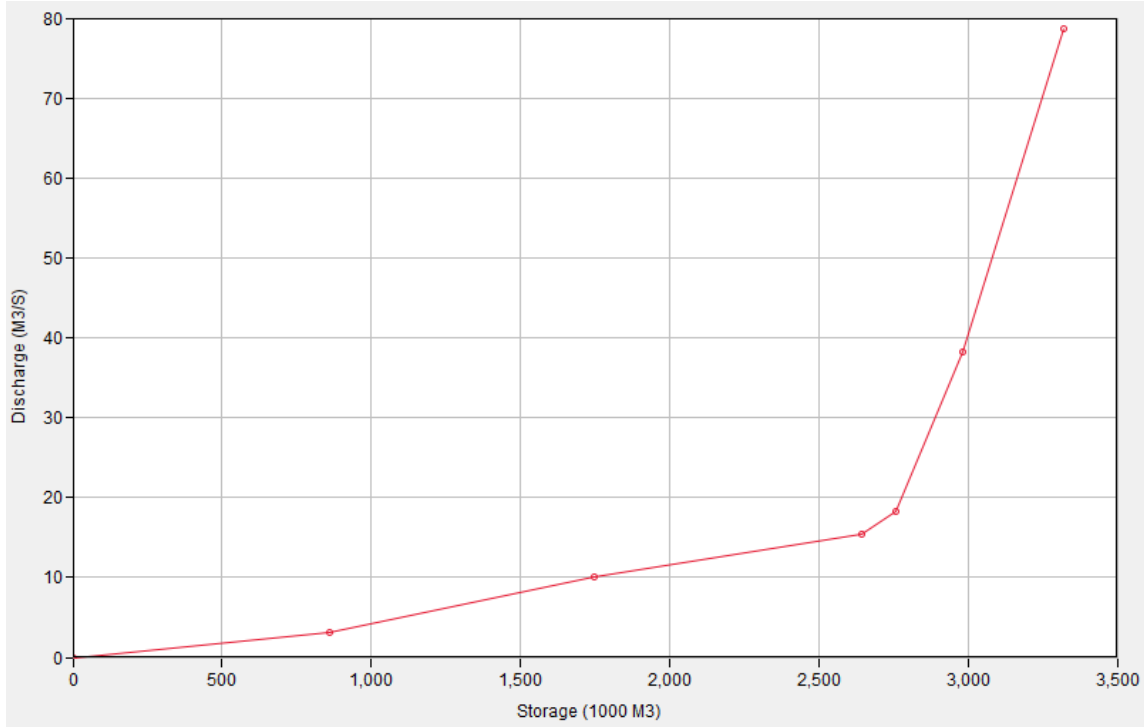


Figure 4-13: Van Luven Lake Storage-Discharge Relationship

Figure 4-14 below illustrates the storage-discharge relationship for Varty Lake. This relationship was obtained from the appendices of 1981 Chrysler & Lathem study for use in the calibration model.

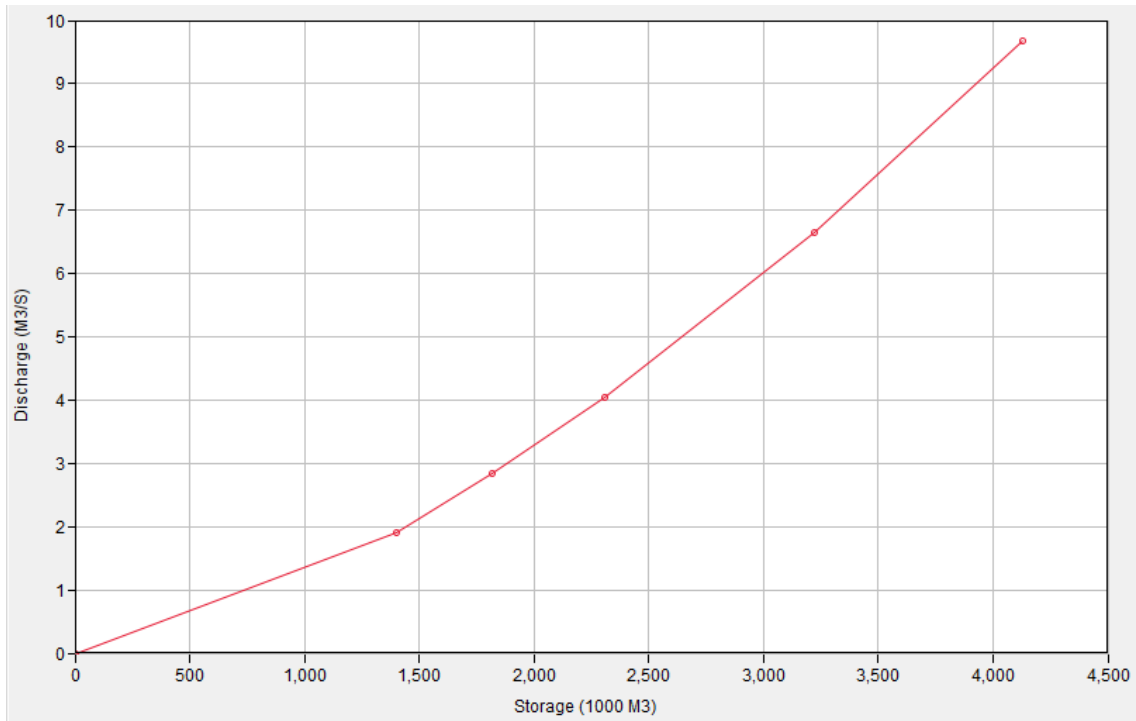


Figure 4-14: Varty Lake Storage-Discharge Relationship

The White Lake outlet was difficult to quantify in the field survey due to its remote location. A 4m length beaver dam is expected to be the current flow control based on satellite imagery and LiDAR and was simulated using broad-crested weir flow.

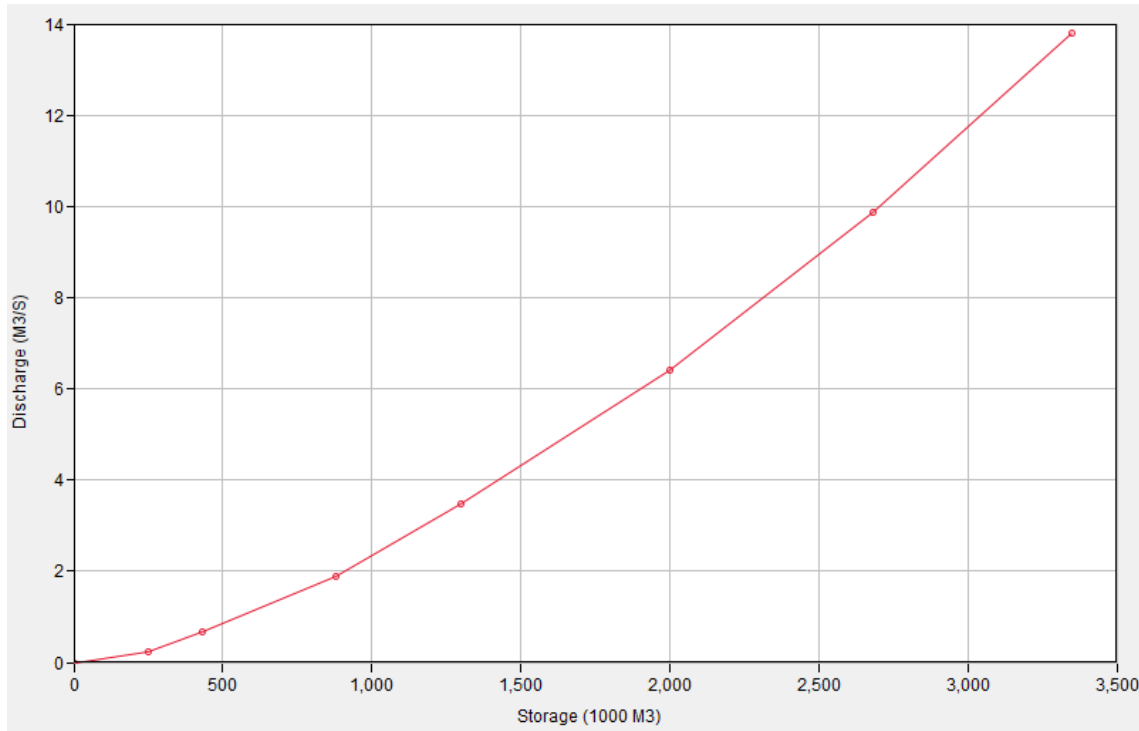


Figure 4-15: White Lake Storage-Discharge Relationship

Recent improvements to the Thirteen Island Lake Dam were completed as described in Section 2. The three (3) 1.8m x 0.9m concrete box culverts, stop logs at normal operating level, and road profile were used to establish the storage-discharge relationship shown below. For the winter setting, there are two logs in the west and middle log bays, and three logs in the east bay as directed by QC staff.

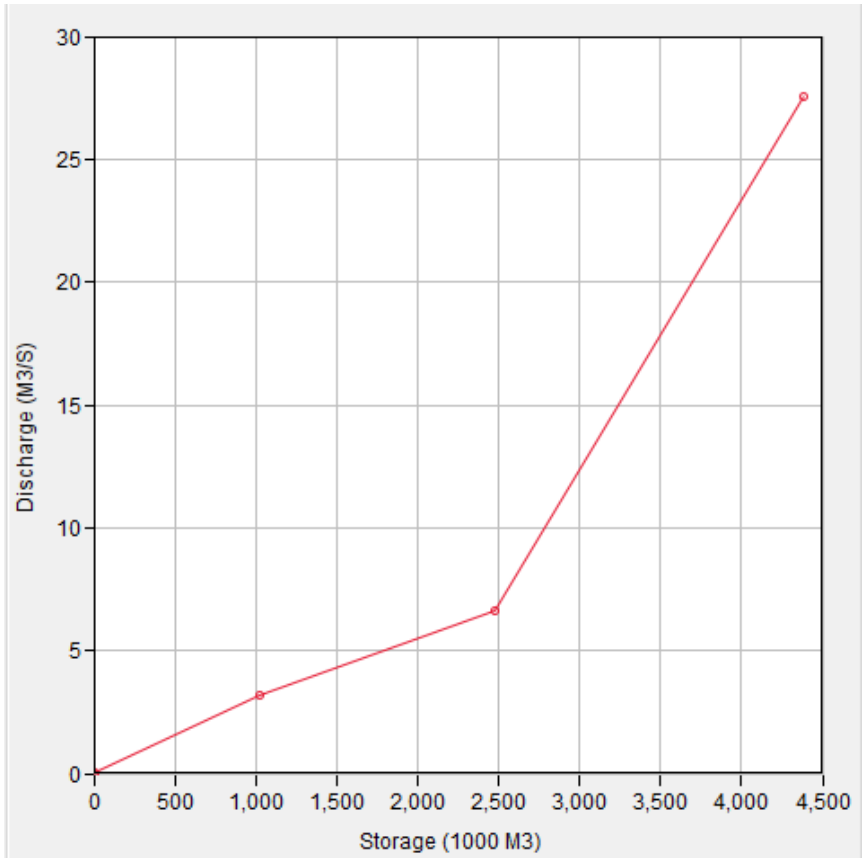


Figure 4-16: 13 Island Lake Storage-Discharge Relationship (Winter)

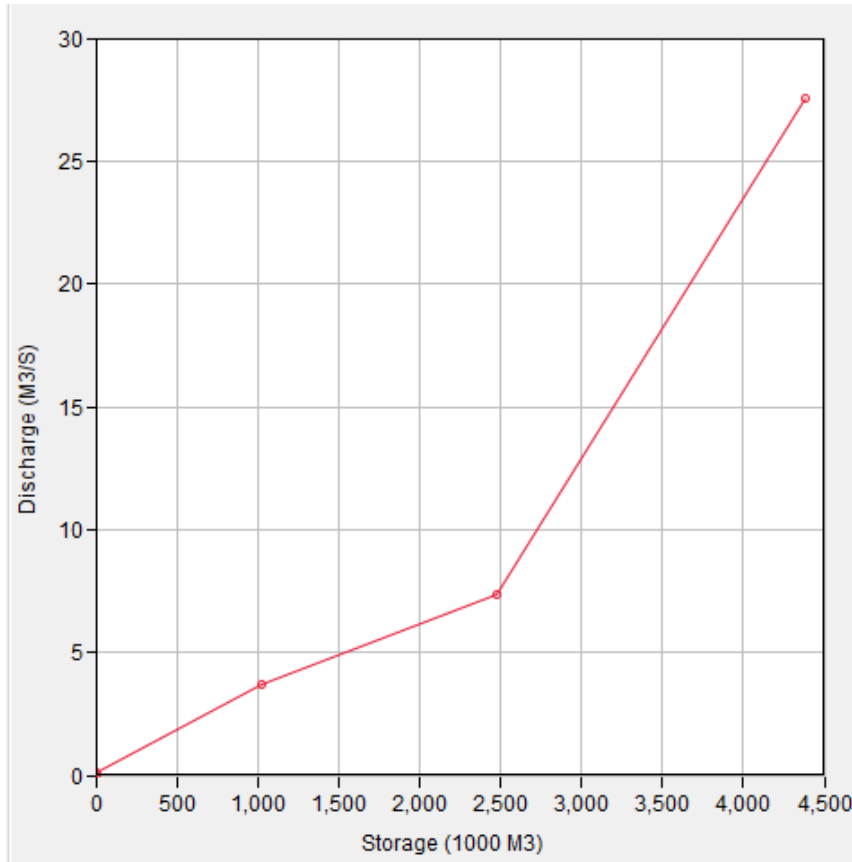


Figure 4-17: 13 Island Lake Storage-Discharge Relationship (Summer)

Fourteen Island Lake and Sigsworth Lake are controlled by the same outlet. The outlet includes a concrete two-stage weir with a 0.76m wide 1st stage and 9.2m wide 2nd stage. This weir yields the storage-discharge relationship shown below in Figure 4-18.

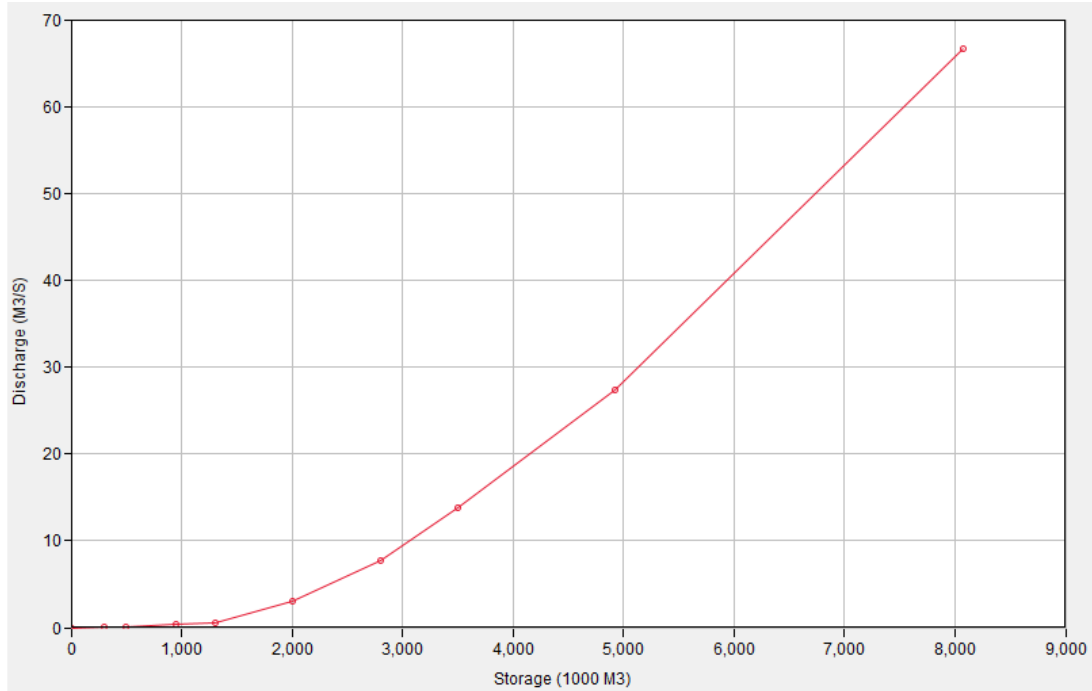


Figure 4-18: Fourteen Island + Sigworth Lake Storage-Discharge Relationship

Thirty Island Lake is controlled by a narrow open channel and a private crossing a short distance downstream. The entrance crossing includes two (2) 450mm diameter plastic pipes causing low outflows until overtopping of the driveway occurs. The result is very low outflows for majority of storm events as shown by the storage-discharge relationship presented in Figure 4-19.

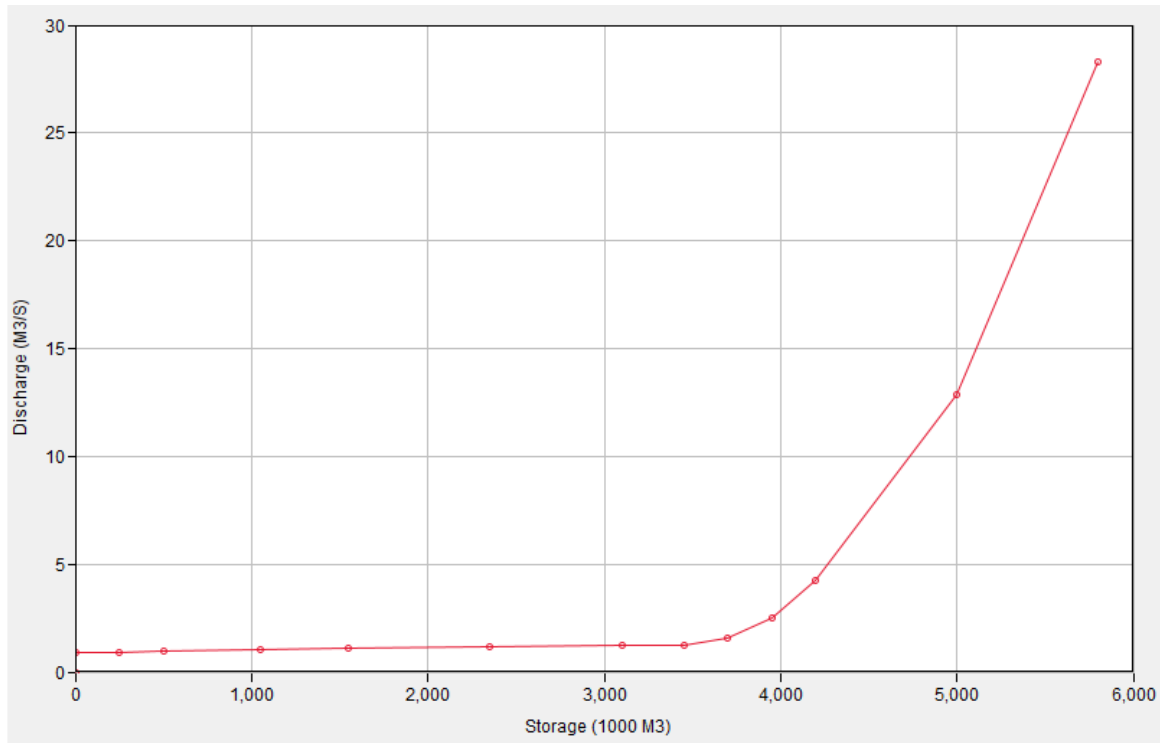


Figure 4-19: Thirty Island Lake Storage-Discharge Relationship

4.3.7 Hydrologic Input Summary

A hydrology input summary is provided below. This summarizes the area, curve number, and lag time (T_p) applied for each sub-catchment.

Table 4-8: Summary of Hydrology Inputs for HEC-HMS Model

Catchment ID	Area (km ²)	Mean Slope (%)	Watershed Length (km)	T_p (hr)	CN
100	2.56	5.5%	0.4	0.28	70.4
200	11.29	5.0%	4.9	2.05	74.3
300	10.07	4.8%	2.7	1.42	70.7
400	2.00	5.1%	2.3	1.25	69.1
500	6.86	6.5%	3.9	1.71	68.7
600	13.92	5.2%	4.7	2.07	71.2
700	16.39	5.7%	5.9	2.55	68.2
800	8.33	5.7%	1.2	0.48	72.8
900	3.64	5.4%	2.6	1.23	72.4
1,000	11.76	1.9%	2.3	1.97	70.8
1,100	14.63	1.3%	4.2	3.69	72.4
1,200	7.17	1.4%	3.4	2.93	73.7
1,300	7.65	3.4%	2.7	1.58	73.5
1,400	12.73	2.1%	2.2	1.91	67.1
1,500	14.63	4.5%	4.7	2.52	65.4
1,600	<u>4.91</u>	2.5%	2.2	1.74	67.9

4.3.8 Peak Flow Summary

A summary of annual exceedance probability peak flows for the rainfall-runoff scenario at each lake outlet and node of interest is shown in the table below. The rise in water levels associated with the 1% AEP regulatory event is presented in Section 4.7.

Table 4-9: Peak Flow Summary for HEC-HMS Rainfall-Runoff Model, m³/s

Outlet of Basin No.	Location	Return Period		
		10%	1%	*1% + CC
1600	Potspoon Lake Outlet	0.5	1.0	1.4
1500	30 Island Lake Outlet	0.9	1.0	1.0
1400	White Lake Outlet	0.5	1.0	1.6
1300	Westport Rd near Glendower	16.0	26.5	36.4
1200	St Andrews Lakes Outlet	0.2	0.7	1.7
1100	Cole Lake Outlet	0.8	3.1	5.2
1000	13 Island Lake Outlet	4.1	6.9	8.9
900	South Frontenac Rd at Godfrey	8.4	14.1	19.4
800	Cole Creek at Godfrey	28.2	47.3	65.4
700	14 Island Lake Outlet	0.1	0.3	0.4
600	Howes Lake Outlet	7.3	12.5	20.8
500	Spring Lake Outlet	0.5	1.2	1.5
400	Little Mud Lake Outlet	3.6	4.7	6.3
300	Van Luven Lake Outlet	0.5	1.5	7.1
200	Hambly Lake Outlet	2.0	2.8	3.8
100	Hardwood Dam Outlet	7.6	13.3	20.0

*Denotes 1% AEP plus climate change with scaled increases in rainfall intensities per Section 4.6.

4.4 Snowmelt + Rainfall Model

As described in the 2002 MNR Technical Guide, Canadian flow data records commonly show that annual peaks and runoff volumes occur in the spring. The nearby Napanee River at Camden East flow gauge is consistent with this national trend. The local gauge has 93% percent of its annual instantaneous peak occurring between December 30 and April 30 across its 46-yr data record.

A separate basin model was prepared in HEC-HMS for the snowmelt plus rainfall scenario, since spring melt events have different characteristics than rainfall only events. These characteristics generally include longer runoff durations, higher runoff volumes, saturated and/or frozen ground conditions, and increased lag times.

The same calibration process that was applied to the rainfall-runoff model was applied to the snowmelt plus rain model. The largest historical event on record that occurred in April of 2014 was selected as the calibration storm, and the Bellrock flow gauge in addition to the information on the Varty Lake and Cameron Swamp reservoirs were used to fill in the missing 'gaps' between the Upper Lakes watershed and the Camden East flow gauge. In the snowmelt plus rain calibration event, it was concluded that AMC III (i.e. saturated) conditions alone were not enough to produce the runoff rates observed in the 2014 event. Therefore, AMC III conditions in *combination* with a 7% increase in CN value (up to a maximum

CN value of 98) was applied. With this adjustment, the calibrated peak flow is within three (3) percent of the observed stream flow record (see Table 4-10); note that a minor transposition of flows was applied for the Cameron Swamp outlet per the same reasoning described in Section 4.3.

An essential component of the calibration for the HEC-HMS snowmelt basin model was the establishment of the distribution and runoff volume released from the snowpack in the 2014 event. Quinte Conservation provided Jewell with their snow melt model that was created by Dr. H. O. Schroeter in 2000 with revisions in September 2008 and February of 2010. The spreadsheet snowpack model is based on the GAWSER snowpack routine that has been through rigorous testing under separate projects prior to its use in the Napanee River Upper Lakes flood hazard mapping.

There are four key inputs into the Schroeter model: daily maximum temperature, daily minimum temperature, daily rainfall, and daily precipitation. For historical events, such as the 2014 flood, the historical climate inputs from the ECCC station at Hartington were supplied to the model. The historical climate inputs provide a reasonable estimate of the snow water content (SWC) and daily runoff that contribute to the corresponding stream flow gauge readings at the *Napanee River at Camden East*. Jewell converted the daily runoff output from the Schroeter model into hourly depths with the application of the sine function to represent the diurnal pattern of spring runoff that would occur due to daily temperature fluctuations (see Appendix C-2).

Table 4-10: Comparison of Observed vs. Modeled Peak Flows for the 2014 Flood Event

	Measured	HEC-HMS
Peak Flow (m ³ /s)	62.7	61.0
Time of Peak	4/15/2014 19:00	4/13/2014 19:00
Lag Time (hrs)	214.0	166.0

The adjustments to the CN values in the snowmelt basin model result in higher CN values and higher peak flow rates due to a greater amount of runoff volume rather than infiltration. This adjustment is consistent with the discussion in the 2002 Technical Guide that indicates runoff coefficients, directly related to the CN, may be 3 to 4 times higher in winter or spring melt events relative to other times of the year. The adjustments required to calibrate the snowmelt plus rainfall model are consistent with the provincial technical guidelines and thereby suitable for use in the statistical snowmelt events. The adjustments are also consistent with recent discussions with ECCC representatives in August of 2023 where ECCC confirmed that CN is the preferred parameter to be adjusted in the calibration process.

For the *statistical* snowmelt AEP events, the degree day method was applied with consideration of the two Meteorological Services of Canada (MSC) snowmelt equations for Ontario as indicated in the 2002 MNR Technical Guide. The MSC snowmelt equation for Model 5 was selected since it is intended for Southern Ontario and yields higher runoff volumes relative to the alternative MSC equation, Model 4, for Southern Ontario. The snowmelt plus rainfall frequency values published by MSC for the Hartington station were used to prepare the hyetographs supplied to the snowmelt simulations in HEC-HMS. The Hartington station has 45 years of record and located in close proximity to the Upper Lakes. The MSC frequency values range from 50 to 1% AEPs with daily volumes from 1 to 30 days (see Appendix C-1).

Local snowmelt characteristics were reviewed further by populating the Schroeter model with historic climate data from 1976 to 2023 to identify which durations produced the largest snowmelts. Out of the ten (10) largest annual instantaneous peaks that were produced by a snowmelt plus rainfall event at the

Camden East stream flow gauge, the average duration was 8.9 days. This suggests a 9-day snowmelt event is most likely to produce a 1% AEP storm event. However, since the historical record does not include all possible duration scenarios, Jewell tested the 1 to 5-day, 10 day, 15-day, 20-day, and 30-day durations to identify the critical duration.

Jewell concluded that the 3-day snowmelt event for the MSC snowmelt frequency values is the critical duration since it yields the largest peak flow at the downstream limit of the Upper Lakes study area.

A summary of AEP peak flows for the snowmelt plus rainfall scenario at each lake outlet and node of interest is shown in the table below. The maximum rise in water levels associated with the 1% AEP regulatory event is presented in Section 4.7.

Table 4-11: Peak Flow Summary for Snowmelt Plus Rainfall AEP Events (3-Day Duration), cms

Outlet of Basin No.	Location	AEP		
		10%	1%	0.5%
1600	Potspoon Lake Outlet	1.0	1.5	1.6
1500	30 Island Lake Outlet	1.0	1.1	1.1
1400	White Lake Outlet	1.5	2.4	2.7
1300	Westport Rd near Glendower	4.4	5.3	5.6
1200	St Andrews Lakes Outlet	1.5	1.9	2.1
1100	Cole Lake Outlet	4.9	6.5	9.4
1000	13 Island Lake Outlet	7.6	8.1	15.1
900	South Frontenac Rd at Godfrey	2.1	2.5	2.7
800	Cole Creek at Godfrey	4.9	5.9	6.2
700	14 Island Lake Outlet	0.4	1.5	1.9
600	Howes Lake Outlet	14.5	20.6	22.9
500	Spring Lake Outlet	1.3	2.2	2.5
400	Little Mud Lake Outlet	1.7	2.5	2.9
300	Van Luven Lake Outlet	1.6	2.0	2.2
200	Hambly Lake Outlet	1.6	1.8	2.1
100	Hardwood Dam Outlet	16.9	21.7	26.1

4.5 Index Flood Analysis

Jewell employed the Index Flood Analysis following the methodology established by the Ontario Ministry of Natural Resources to estimate design flows and assess the hydrology of the contributing drainage area. The Index Flood Analysis was included since its methodology relies on historical stream flow gauge data and based on experience it has shown to generally produce reliable flow estimates and often compares well with HEC-HMS modeling outputs. The Index Flood Analysis may be less suitable for the subject watershed since the Index Flood method relies in hydrologically similar watershed characteristics; the subject watershed has unique characteristics when considering its large lakes and

outflow control structures at Hardwood Dam, Thirteen Island Lake Dam, and Fourteen Island Lake Dam. However, the Index Flood method was included for comparison purposes.

The Index Flood method relates the annual peak instantaneous flow determined for 247 stream gauges across Ontario to drainage area based on the *User Guide for Ontario Flow Assessment Tool (OFAT)* published by the Ontario Ministry of Natural Resources. Twelve regions across the province were identified as having similar characteristics and a regression curve was developed for each region. See Figure 4-22. The Upper Napanee Lake watershed is located in Region 1. The 1% AEP peak flow estimated from the Index Flood Analysis, located at the Hardwood Dam, is 50.7m³/s. The peak flows from the Index Flood analysis are included in the presentation of peak flows in Section 4.7.

The 50% AEP flows are resolved directly from the equation using the constant and exponent from Table 4-12. Other AEP flows may be derived from the 50% AEP flow by multiplying with the factors provided in Table 4-13.

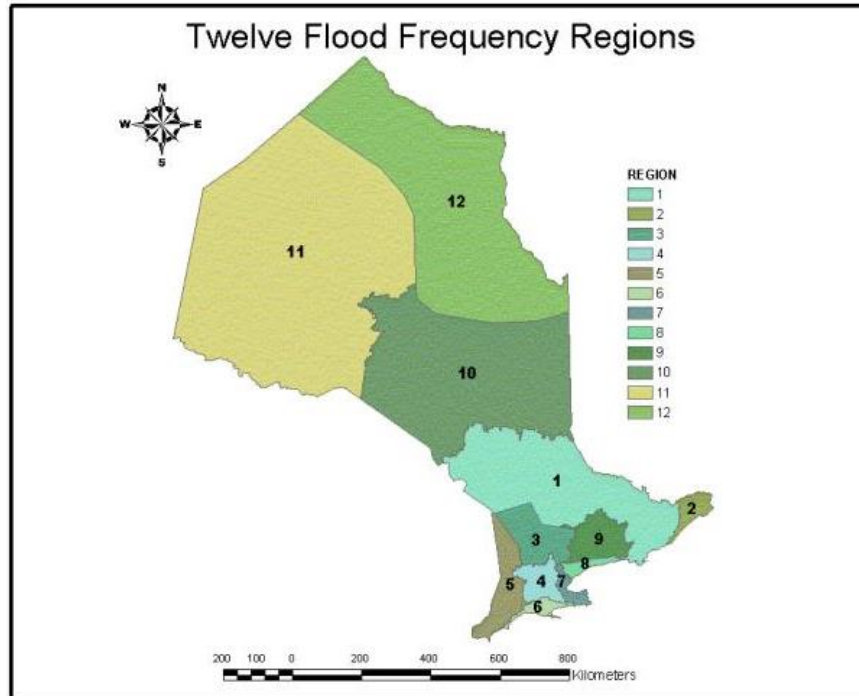


Figure 4-20: Index Flood Regions (Ministry of Natural Resources and Forestry, 2020)

Equation: Index Flood Method

$$Q_2 = CA^n$$

Where:

Q_2 = 50% AEP (3 parameter Log Normal) flood

A = Drainage Area (km²)

C = constant

n = exponent (slope of the line)

Table 4-12: Table of Constant (C) and Exponent (n) for use in the Modified Index Flood Equation (Ministry of Natural Resources and Forestry, 2020)

Region	Constant (C)	Exponent n
1(a)	0.22 (A < 60 km ²)	1.000
1 (b)	0.73 (A > 60 km ²)	0.707
2	0.51	0.896
3	0.20	0.957
4	0.71	0.842
5	0.45	0.775
6	0.41	0.806
7	1.13	0.696
8	0.73	0.785
9	0.40	0.810
10	0.28	0.849
11	0.38	0.706
12	0.59	0.765

Table 4-13: Ratio of Various Flood Frequencies to Q₂ (Ministry of Natural Resources and Forestry, 2020)

Region	Q _{1.25} /Q ₂	Q ₂ /Q ₂	Q ₅ /Q ₂	Q ₁₀ /Q ₂	Q ₂₀ /Q ₂	Q ₅₀ /Q ₂	Q ₁₀₀ /Q ₂	Q ₂₀₀ /Q ₂	Q ₅₀₀ /Q ₂
1	0.95	1.00	1.24	1.43	1.62	1.86	2.04	2.23	2.48
2	0.94	1.00	1.29	1.52	1.74	2.04	2.25	2.45	2.72
3	0.93	1.00	1.33	1.62	1.89	2.25	2.54	2.82	3.19
4	0.93	1.00	1.32	1.57	1.80	2.13	2.37	2.60	2.92
5	0.94	1.00	1.27	1.50	1.74	2.06	2.34	2.62	2.96
6	0.91	1.00	1.43	1.78	2.13	2.60	2.96	3.33	3.84
7	0.94	1.00	1.27	1.47	1.66	1.90	2.07	2.24	2.47
8	0.92	1.00	1.43	1.85	2.30	2.96	3.46	4.00	4.77
9	0.94	1.00	1.27	1.50	1.72	2.02	2.26	2.49	2.80
10	0.95	1.00	1.20	1.35	1.48	1.64	1.77	1.90	2.07
11	0.93	1.00	1.33	1.62	1.90	2.32	2.67	3.05	3.55
12	0.94	1.00	1.22	1.38	1.52	1.68	1.80	1.90	2.05

Table 4-14: Limitation of Application of Index Flood Method Based on Drainage Area (Ministry of Natural Resources and Forestry, 2020)

Region	Minimum (km ²)	Maximum (km ²)
1	0.11	9270
2	76.1	3816
3	86.0	3960
4	2.5	5910
5	14.2	4300
6	5.2	697
7	63.5	293
8	4.9	800
9	24.3	1520
10	18.6	11900
11	0.7	24200
12	4250	94300

4.6 Climate Change

In QC's meeting with the federal partners, it was noted that a preferred method to assess climate change scenarios is scaling rainfall intensity based on projected increase in degrees of warming. Jewell completed this preferred approach using the technical requirements outlined in the ECCC memorandum titled *Incorporating Climate Change in Floodplain Mapping under the Flood Hazard Identification and Mapping Program*. The 0.5% AEP was also assessed as a potential climate change scenario. The 0.5% AEP flows were included in the rainfall-runoff and snowmelt plus rain runoff models described in the previous subsections.

Recall that the Napanee River Upper Lakes are located within Zone 2 of the *Flood Hazard Criteria Zones of Ontario and Conservation Authorities*. The 1% AEP is the regulatory event.

The hourly rainfall that corresponds to the regulatory storm was adjusted using the mean annual temperature change obtained from the federal climate data portal for Verona, ON. Jewell followed the Ontario MNRFs recommendation of obtaining the value for the 50th percentile of the mean annual temperature change based on the CMIP5, RCP 4.5 scenario.

The year 2071 was selected since this is the furthest projected date in the Excel download from the federal climate data portal. The mean annual temperature change for the year 2071 is an increase of 3.3 degrees Celsius (see Appendix E). An excerpt from the technical memo defining the equation used to convert historic rainfall intensity and temperature change to the future estimated rainfall intensity is provided in Figure 4-23.

The increase in temperature results in a significant (25%) increase in precipitation volume (see Table 4-15).

The rainfall-runoff model is particularly sensitive to climate change since the increased runoff produces higher inflows to the lakes, and also depletes the storage within the reservoirs. The storage-discharge charts in Section 4.3.6 show that the outflow rates generally increase at a faster rate as the storage becomes depleted. The result is that the rainfall-runoff model is very sensitive to the increased rainfall projected in the climate change scenario.

Table 4-15: Future Estimated Rainfall Intensities for 1%, 24-Hr AEP

Time	Historic Intensity (R _c)	Percent of 24 Hour	Future Estimated Intensity (R _p)	% Increase in Intensity
Hour	mm/hr	mm	mm/hr	
1	1.1	1.1	1.3	25.0%
2	1.1	1.1	1.3	
3	1.3	1.3	1.6	
4	1.3	1.3	1.6	
5	1.6	1.6	1.9	
6	1.6	1.6	1.9	
7	1.9	2.0	2.4	
8	1.9	2.0	2.4	
9	2.6	2.7	3.3	
10	3.3	3.4	4.1	
11	5.2	5.4	6.6	
12	41.6	42.8	52.0	
13	10.6	10.9	13.2	
14	4.7	4.8	5.8	
15	2.9	3.0	3.6	
16	2.9	3.0	3.6	
17	1.7	1.8	2.2	
18	1.7	1.8	2.2	
19	1.7	1.8	2.2	
20	1.7	1.8	2.2	
21	1.2	1.2	1.5	
22	1.2	1.2	1.5	
23	1.2	1.2	1.5	
24	<u>1.2</u>	<u>1.2</u>	<u>1.5</u>	
Total	97.1	100	121.4	25.0%

Determine future estimated rainfall intensity value (R_p), according to the historic estimated rainfall intensity (R_c) and the long term (30-year mean) annual mean temperature change (ΔT) using equation (1):

$$R_p = R_c \times 1.07^{\Delta T} \quad (1)$$

Figure 4-21: Excerpt from Technical Memo with Equation for Future Estimated Rainfall Intensities (Environment and Climate Change Canada)

It should be noted that climate change impacts on peak flows are inherently difficult to quantify due to the reality of Earth's extremely complex global atmospheric and hydrologic systems. The climate change adjustment applied above relies on the relationship between temperature increase and rainfall depth. Therefore, the adjustment addresses a climate change scenario for a precipitation-driven flood event.

Based on calculations and an assessment of the data, climate change is expected to have a greater impact on rainfall-driven runoff events rather than a snowmelt driven runoff event.

The stream flow gauge data presented in Section 4 generally illustrates the expected AEP flows that would occur during a freeze-thaw/snowmelt condition. This is because most annual instantaneous peaks occur in the spring months. These events produce high peak flows due to a large volume of stored water content that is released when warmer temperatures occur.

With warmer seasonal temperatures generally expected due to climate change, it is reasonable to expect less stored water content during the winter months, since the period of below-freezing temperatures would be shortened with higher average temperatures. With less stored water content, it is possible that instantaneous peaks produced in a spring melt condition may not increase even with increased rainfall depths for single event conditions. Jewell investigated this further by testing the sensitivity of the Schroeter model to the projected temperature and precipitation increases. *Individual* increases for temperature and precipitation were also completed to better understand their individual effects on the model.

The Schroeter model predicts that an increase in precipitation by 25%, with no temperature adjustments, would *increase* the maximum snow water content (SWC) by 51%. The model predicts a mean temperature increase of 3 degrees Celsius, with no precipitation adjustments, would *decrease* the maximum SWC by 36%. It predicts that the *combination* of temperature and precipitation increases will produce net reduction of 9% in the maximum SWC.

A review of average monthly temperatures, with the mean daily maximum and minimums increased by 3 degrees Celsius, is presented in Figure 4-24. The 3-degree temperature increase would result in a daily maximum average temperature in the coldest month (January) that is near zero. This would encourage more frequent intermittent melting periods, and results in the net decrease in maximum SWC in the Schroeter model.

While there are many combinations of future temperature and precipitation that could produce higher or lower maximum SWC values, the key takeaway is that climate change is expected to have a greater impact on rainfall-driven flood events rather than spring melt events. Since higher future temperatures

will likely offset some of the future increases in precipitation, the extent of the net change to the maximum SWC and subsequent snowmelt plus rain events is largely unknown. It is recommended that potential impacts due to climate change be accommodated by utilizing the federal climate data adjustments for rainfall-driven events, and the 0.5% AEP for snowmelt-driven events.

Table 4-16: Temperature and Precipitation Impacts on Maximum SWC per the Schroeter Snowmelt Model

Adjustment	Average Maximum SWC (mm)	Percent Change in Maximum SWC
No adjustment	75	-
Temperature +3°C	48	-36%
Precipitation +25%	113	+51%
Temperature +3°C, Precipitation +25%	68	-9%

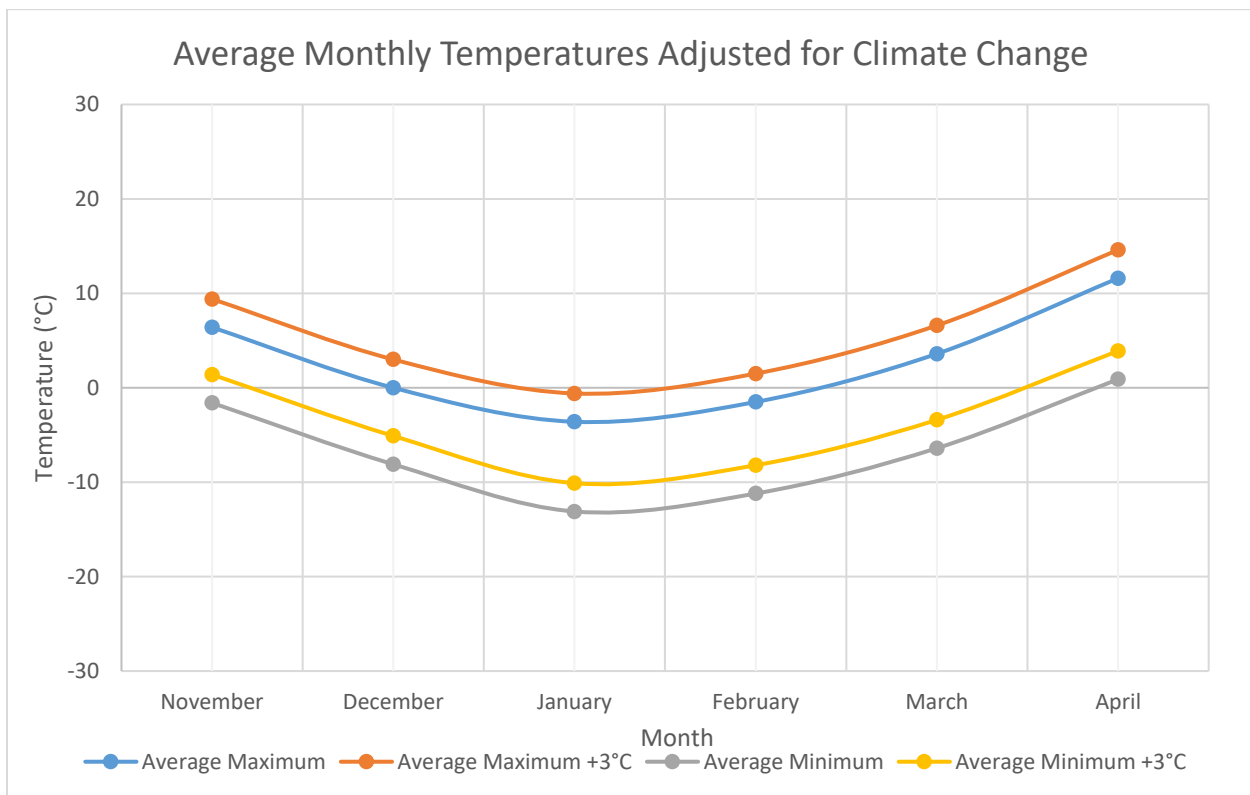


Figure 4-22: Projected Temperature Increases at Hartington ECCC Station Due to Climate Change

4.7 Presentation of Peak Flows

The peak flows for the Napanee River Upper Lakes are summarized below in Table 4-17. The peak flow rates in the table below are applied to identify the flood hazard limits. The climate change peak flows in Table 4-9 (see Section 4.3.8) will also be included in the hydraulic model computations.

Table 4-17: Peak Flow Summary for Napanee River Upper Lakes

Outlet of Basin No.	Location	100-Yr, 3-Day Snowmelt + Rain		100-Yr, 24-Hr Rainfall	
		Peak Flow (cms)	*Max. Water Level Rise (m)	Peak Flow (cms)	*Max. Water Level Rise (m)
1600	Potspoon Lake Outlet	1.5	0.35	1.0	0.26
1500	30 Island Lake Outlet	1.1	0.54	1.0	0.24
1400	White Lake Outlet	2.4	0.46	1.0	0.26
1300	Westport Rd near Glendower	5.3	-	26.5	-
1200	St Andrews Lakes Outlet	1.9	-	0.7	-
1100	Cole Lake Outlet	6.5	-	3.1	-
1000	13 Island Lake Outlet	8.1	1.1	6.9	0.8
900	South Frontenac Rd at Godfrey	2.5	-	14.1	-
800	Cole Creek at Godfrey	5.9	-	47.3	-
700	14 Island Lake Outlet	1.5	0.65	0.3	0.32
600	Howes Lake Outlet	20.6	1.78	12.5	1.33
500	Spring Lake Outlet	2.2	-	1.2	-
400	Little Mud Lake	2.5	-	4.7	-
300	Van Luven Lake	2.0	1.78	1.5	1.33
200	Hambly Lake Outlet	1.8	1.67	2.8	1.27
100	Hardwood Dam Outlet	21.7	1.67	13.3	1.27

**Maximum water level rise measured from outlet invert.*

Table 4-18 provides a comparison of peak flows for each of the hydrologic modeling methods. The 2023 regulatory peak flow of 21.7m³/s is below the peak flow from the transposed GFA result; however, the 21.7m³/s accounts for the high tailwater imposed by the Cameron Swamp at the outlet of the study area, whereas the GFA and Index Flood methods do not. In a free-flowing condition, the peak outflow from the study area would be 31.4 m³/s. The 2024 peak flow is less than the 1981 study, however the water level at the outlet and lower lakes is higher than the 1981 results. This is in large part due to the tailwater condition at the Cameron Swamp which is discussed further in Section 5.1.3. The high tailwater at Cameron Swamp increases the water level and reduces the peak flow in the 2024 assessment.

Table 4-18: Summary of Peak Flows from Alternative Methods for Napanee River Upper Lakes (m3/s)

Return Period	1981 Chrysler & Lathem		GFA (Transposed)		Index Flood	HEC-HMS	
	Rain Only	Spring Melt + Rain	Rain Only	Spring Melt + Rain		Rain Only	Spring Melt + Rain
100-Yr	25	50	16.4	24.7	50.7	12.7	21.7

*Denotes regulatory 1% AEP peak flow.

4.8 Combined HEC-HMS & HEC-RAS Models

The HEC-HMS model used in the model calibration (Section 4.3.1) was advanced further in Section 5 that follows to include the channel routing that occurs as part of the hydraulic simulations. Since the hydraulic model is predominantly a 2D, unsteady flow model, the travel time and roughness of the channel and overbank areas are reflected in the outflow hydrographs. The wetlands and smaller lakes are also included in the hydraulic model as they have been either modeled as a 2D flow area or as a storage area depending on their size and dimensions.

The annotated schematic below illustrates the sequence of the HEC-HMS and HEC-RAS model runs. In model testing, it was found that reservoir-routing in HMS and RAS produced similar results. For simplicity and faster run times, each lake that has a free-flowing outlet was modeled in HEC-HMS. For lakes with substantial backwater impacts, particularly at the bottom end of the study area (Verona, Howes, Hambly, Van Luven), they were modeled as a storage area in HEC-RAS. St. Andrews Lake was also modeled in RAS due to the strong backwater imposed by the K&P Trail crossing.

Figure 4-25 shows that there are six (6) HEC-RAS models applied to calculate the hydraulic outputs for the study area. Table 4-19 summarizes the corresponding Reach Number (see Section 5), route length, primary lake, and wetland/sub-lake associated with each hydraulic model.

Note that the peak flows presented in Section 4.7 were obtained using the modeling approach presented in Figure 4-25.

A seventh (7th) hydraulic model was prepared as a separate 1D, steady flow model to identify the shape of the rating curve for the Cameron Swamp outlet that is necessary to identify the tailwater conditions for the Napanee River Upper Lakes study area. This 7th model is 7.3 km downstream of the study area and upstream of Petworth Road; it is discussed in detail in Section 5.1.3.

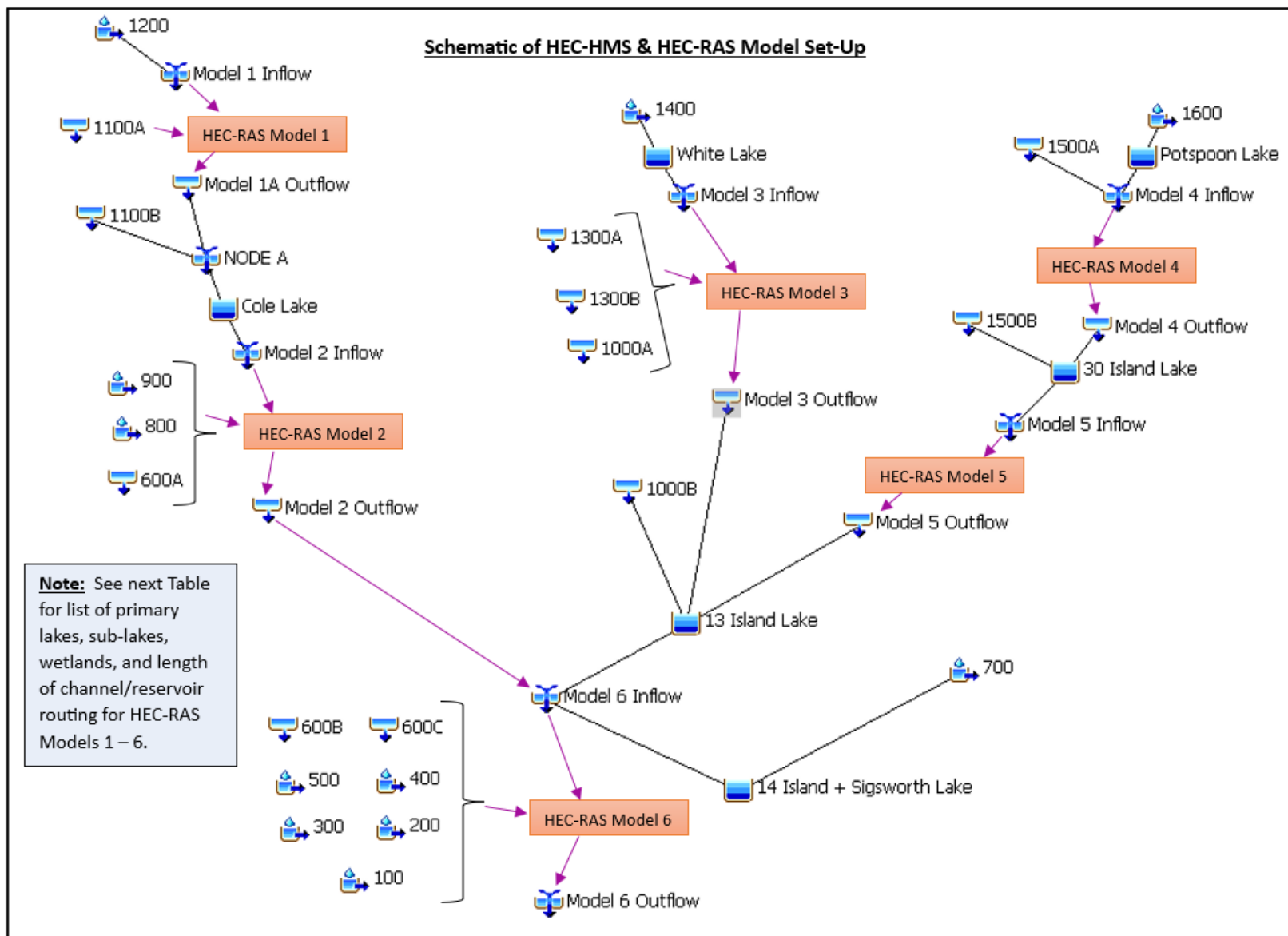


Figure 4-23: Schematic of Interactive HEC-HMS & HEC-RAS Models

Table 4-19: HEC-RAS Models with Corresponding Reach, Route Lengths, Primary Lake(s), Wetland(s) and Sub-Lake(s)

HEC-RAS Model	Reach No.	Route Length (km)	Primary Lake(s)	Wetland # / Sub-Lake
1	1	1.7	St. Andrews Lakes	1A, 1B, 1C
2	2	6.2	N/A	2A, 2B, 2C
3	6	5.3	N/A	6A, 6B
4	7	2.5	N/A	Connector Lake 7A, 7B
5	8	1.2	N/A	8A, 8B
6	3, 4, 5, 9, 10, 11	N/A - Lake Dominated	Verona, Howes, Hambly, Van Luven Lakes	Bass, Spring, Mud Lakes, Wetland 9

5 Hydraulics

The hydraulic analysis was prepared using HEC-RAS version 6.4.1. The HEC-HMS peak flows from Section 4 were applied in the hydraulic model to delineate the flood hazard limits for the Napanee River Upper Lakes drainage system. The HEC-RAS model simulates both the 1% AEP snowmelt plus rain and 1% AEP rainfall only events as separate scenarios to determine the governing condition for each portion of the watershed.

In addition to the thirteen main lakes of interest, the hydraulic system is defined by many wetlands and smaller lakes that act as hydraulic connections among the Napanee River Upper Lakes system. More traditional channels with moderate slopes and defined overbank areas are present to a much lesser extent. This subsection describes the modeling approach, bridge and culvert crossings, storage areas, and water levels.

5.1 Model Set-Up

HEC-RAS Version 6.4.1 and HEC-HMS Version 4.11 were used for the modeling of the Napanee River Upper Lakes (Brunner, 2023), (Bartles, et al., 2022). The HEC-RAS model applies the hydraulics software used to simulate the flood limits that result from a large surface runoff event. This section describes the model set-up including the field survey, computational mesh, hydraulic structures, storage areas and model run scenarios.

Due to the large size of the study area, its several branches, and many lakes and wetlands, the hydraulic model was discretized into eleven (11) reaches (see Figure 5-3). The model discretization for the eleven reaches is summarized in Section 5.2.

5.1.1 Field Survey

The LiDAR data described in Section 4.1 was supplemented by site-specific survey data from Jewell's survey crew using a GPS and a total station. The GPS was the main equipment used for the field survey. The GPS survey results were converted to the CGVD 2013 datum and imported into the terrain layer as an overlay to the LiDAR data. For local surveyors, a conversion of -0.33m can be used to convert from datum CGVD28 to CGVD 2013. This conversion is based on a comparison of elevations in the field measured at the same location in each datum. The projection settings in the model are NAD 1983 UTM Zone 18.

Historically, 1-dimensional hydraulic models have been used for floodplain mapping. This type of model requires cross-section data to be set up by the user to represent the geometry data applied in the hydraulic model calculations. With advancements in the HEC-RAS modelling software that is developed and distributed freely by the U.S. Army Corps of Engineers, 2-dimensional modelling presents an alternative that can provide added benefits depending on the river of interest.

A 2-dimensional model was selected for the Napanee River Upper Lakes for the following reasons:

- To improve upon the channel routing options in HEC-HMS; the Napanee River Upper Lakes do not have a uniform river cross-section throughout the study area due to the many wetlands and connector lakes. A 2D hydraulic model substitutes a 'representative' cross-section with a pre-determined number of grid cells set throughout the study area to allow flow to be routed using its proper geospatial geometry.
- To investigate further flow attenuation that would be present within the additional wetlands and smaller lakes using storage areas or 2D flow areas.
- To achieve more realistic modeling results in low-lying areas.

- To take advantage of detailed terrain and survey data; HEC-RAS software uses this data to produce output results for depth, velocity, and water surface elevations at any georeferenced location within the flood study area.

5.1.2 Computational Mesh

The terrain layer was used to develop a computational mesh that ultimately controls the movement of water through portions of the Napanee River Upper Lakes channels and surrounding overbank or wetland areas. For each computation cell, an elevation-volume relationship is calculated to produce a single water surface elevation.

The cell sizes for the 2D flow areas, including refined areas for creek sections and roads, are summarized by the attribute tables included in Appendix I.

5.1.3 Boundary Conditions

The Napanee River Upper Lakes hydraulic model includes three different types of boundary conditions (BCs). One is an inflow BC and the other two are outflow BCs.

The inflow BCs are represented by an inflow hydrograph. The 2D unsteady flow model received its flow data from an *inflow* hydrograph where the incoming flows change with time. The inflow hydrographs were obtained from the HEC-HMS model for individual sub-catchments. Each inflow BC corresponds to an inflow hydrograph. The two types of *outflow* BCs applied in the Napanee River Upper Lakes hydraulic model are a stage hydrograph and normal depth.

The stage hydrograph is used to reflect the potential backwater impacts from downstream lakes and ensures the water levels in the upstream channels are adjusted accordingly. The normal depth outflow BC is used in locations where there is no immediate lake downstream of a channel outflow. The normal depth was set to 1% to simulate free-flowing conditions.

Cameron Swamp

In spring melt or high flow conditions, the Hardwood Dam is submerged due to the backwater imposed by the Cameron Swamp. The Cameron Swamp is a large reservoir that receives majority of runoff that contributes to WSOC flow gauge *02HM007 Napanee River at Camden East*. This reservoir establishes the tailwater for the lower lakes of the study area (i.e. Verona, Howes, Hambly, Van Luven) and is critical to accurately model their water levels.

The Cameron Swamp does not have a hydraulic structure controlling its outflows. From site observations, review of LiDAR data, and a review of bridge elevations at the nearest hydraulic structure downstream of the swamp (Petworth Road bridge), it was confirmed that it is a natural channel outlet with a constriction point 3.5km upstream of Petworth Road that controls the water levels in the Cameron Swamp. This natural outlet channel was modeled in HEC-RAS to develop a rating curve that is representative of the Cameron Swamp outlet (see Figure 5-1).

The Cameron Swamp outlet rating curve is presented in Figure 5-2. Fortunately, the Cameron Swamp receives majority of the runoff that contributes to WSOC stream flow gauge *02HM007 Napanee River at Camden East*. Therefore, the flows selected to develop the rating curve are the same as those obtained from the General Frequency Analysis (GFA) described previously in Section 4.2 and presented in Table 4-6. This approach applies an assumption that the GFA results at the stream flow gauge are representative of the Cameron Swamp outflows (i.e. no transposition of flows). This is a reasonable assumption since the massive amount of storage in the Cameron Swamp would result in a peak outflow that occurs at a largely different timestamp relative to the peak outflows from the much smaller downstream

catchments of EXT-3 and EXT-4 from Figure 4-7. The application of a transposition of flow for the production of the rating curve would be an over-simplification that would not account for the timing of the hydrographs. Therefore, a transposition of flows was not utilized in the development of the Cameron Swamp rating curve.

The GFA peak flows for the 2-, 10-, 50-, 100-, 200-, and 500-yr return periods were applied in a 1D steady flow model of the outlet channel to develop the Cameron Swamp rating curve. Note that this 1D steady flow model is separate from the hydraulic model used for the Napanee River Upper Lakes study area; the sole purpose of the 1D steady flow model for this channel is to identify the water surface elevation vs. flow relationship at the Cameron Swamp outlet.

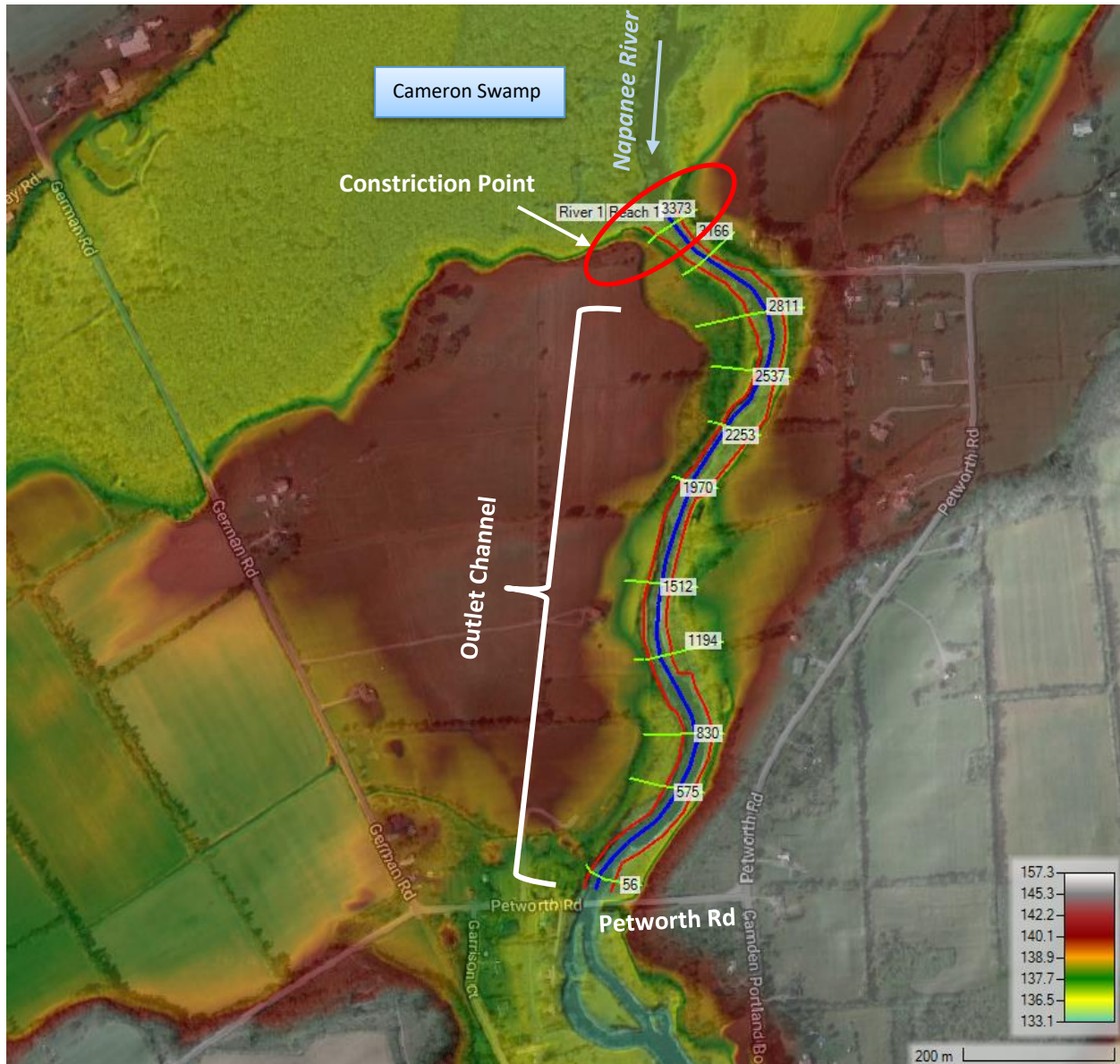


Figure 5-1: Cameron Swamp Constriction Point and Outlet Channel in HEC-RAS to Obtain Outlet Rating Curve

Once the Cameron Swamp outlet rating curve was established, it was verified by matching field measurements of water levels in the vicinity of the Hardwood Dam to the flows at the WSOC 02HM007

stream flow gauge during the time of measurement. The Jewell surveyors measured the water level and noted the time of measurement on November 1, 2023 and March 6, 2024. Then, the WSOC flow gauge was reviewed for its nearest timestamp to obtain the corresponding flow measurement. The measured water level and stream flow gauge data was used to identify the observed data points denoted as a red “X” in Figure 5-2.

Field measurements #1 (Nov. 1, 2023) and #2 (Mar. 6, 2024) were obtained from Jewell staff. Field measurement #3 was provided from Quinte Conservation for the peak water level on April 15, 2014.

Field measurement #1 represents a low flow condition where the tailwater of the study area is below the invert of the Hardwood Dam. Field measurement #2 represents a common spring-melt condition where the Harwood Dam is submerged. Field measurement #3 represents the high tailwater condition that would be similar to a 1% AEP event since the measurement was obtained at the same time as a 79.7 m³/s peak flow at the stream flow gauge that is similar to the 1% AEP flow of 80.8 m³/s from the GFA. Field Measurement #3 was obtained at the Desert Lake Road bridge; this measurement was conservatively assumed to be equal to the water level in the vicinity of the Hardwood Dam, although in reality the water level would likely have been slightly lower at Hardwood Dam.

The Cameron Swamp rating curve is used to provide the general shape of the graph of tailwater vs. flow that would occur near the Hardwood Dam. The massive size of the Cameron Swamp is evident in a measurement from the length of flow path a water particle would travel from the Hardwood Dam to the Cameron Swamp outlet; this distance is 7.3 km. At this size, even a minimal gradient creates a minor difference in water level between the Cameron Swamp outlet and the Hardwood Dam outlet. In low flow conditions, this gradient is 0.0055% (0.000055 m/m) and creates a 0.41m difference in elevation. As the flows become greater, the gradient becomes lower and the water level at the Cameron Swamp become closer to the Hardwood Dam outlet, although still slightly lower. This is evident in the two blue lines shown in Figure 5-2.

The dashed blue line in Figure 5-2 was derived from field measurements and was matched to the general shape of the Cameron Swamp rating curve. This dashed blue line was applied as the tailwater condition for each respective return period event in both *rainfall only* and *rainfall plus spring melt* conditions.

For rainfall only conditions, it would be unrealistic to apply the GFA return period results from Table 4-6 since these results are derived from a dataset that is dominated by annual instantaneous peak flows that occurred in a *spring melt plus rain* condition. ECCC provided Jewell with the flow records at WSOC Flow Gauge 02HM007 at 15 to 30-minute intervals for its full length of record (1974 – 2022). Jewell used this data to identify the maximum instantaneous peak flow that occurred between May 1st and December 31st for each year. This set of maximum instantaneous peaks was used to prepare the dataset to calculate a GFA on *rainfall only* events. As expected, the rainfall only events (i.e. storms between May 1st and December 31st) generally produce lower instantaneous peak flows at the stream flow gauge, meaning they would produce a lower water level in the Cameron Swamp, and subsequently a lower tailwater condition for the Napanee River Upper Lakes study area.

The 2nd column in Table 5-1 below is supplemental to the Flood Frequency Analysis described in Section 4.2 of this report; it represents the statistical return period flows for *rainfall only* events. The dashed blue line in Figure 5-2 was used to obtain the tailwater elevations for the study area under both spring melt plus rainfall conditions and rainfall only conditions (see Table 5-1).

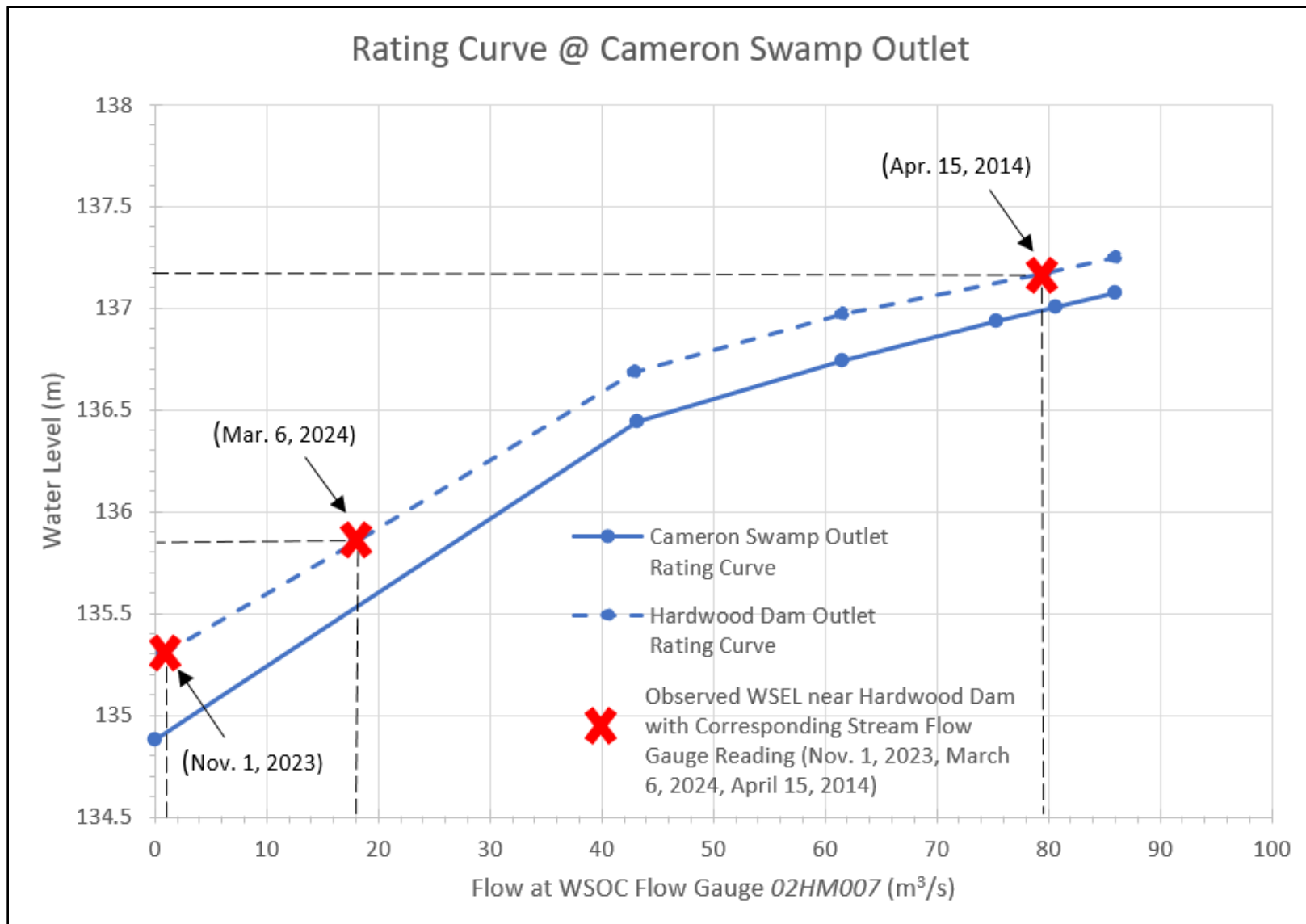


Figure 5-2: Rating Curve at Cameron Swamp Outlet with Comparison to Observed Water Levels near Hardwood Dam

Table 5-1: Tailwater Elevations for Napanee River Upper Lakes Study Area

AEP (%)	Rain Only		Snowmelt Plus Rain	
	GFA Q _{peak} (m ³ /s)	Tailwater (m)	GFA Q _{peak} (m ³ /s)	Tailwater (m)
50	23.0	136.02	43.2	136.69
10	37.5	136.51	61.5	136.97
2	48.9	136.78	75.3	137.12
1	53.5	136.85	80.7	137.18
0.5	58.0	136.92	86.0	137.24

**Tailwater represents water level at Hardwood Dam.*

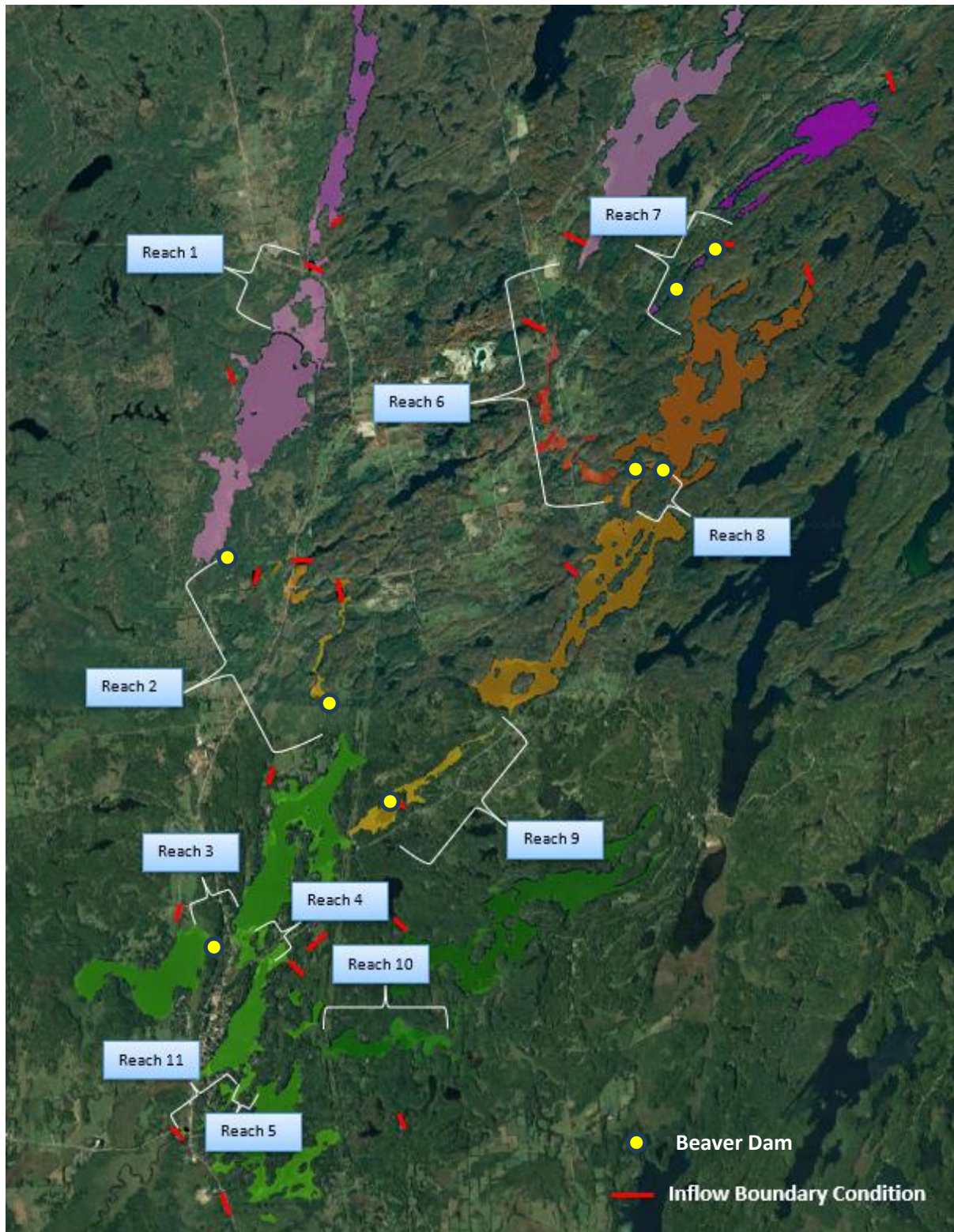


Figure 5-3: Reach and Inflow Boundary Condition Location Summary

5.1.4 Storage Areas

For areas with significant level pools (i.e. wetlands, lakes, and swamps), storage areas were connected within the hydraulic model using a boundary condition or hydraulic connection. The storage areas within the HEC-RAS model simulate the reservoir routing for prominent lakes, sub-lakes, and wetlands throughout the study area. The storage areas are routed using their stage-storage relationship combined with their outlet configuration using an SA/2D connection. The SA/2D connection feature is used to connect lakes and wetlands, and is also used to connect lakes and wetlands to road crossings and/or the lake outlet control structures.

5.1.5 Model Run Scenarios

Several model runs were completed to cover different runoff scenarios. These scenarios are listed below.

- 10% AEP rainfall event.
- 10% AEP snowmelt plus rain event.
- 1% AEP rainfall event.
- 1% AEP snowmelt plus rain event.
- 0.5% AEP snowmelt plus rain event (i.e. spring melt climate change scenario).
- 1% AEP rainfall event with increased rainfall and sub-catchment flows based on ECCC methodology for climate change as applied in the *Section 4.6*.
- Simulation with beaver dams in place for the 1% snowmelt plus rain event.
- Simulation with high Manning's n values to assess the hydraulic model sensitivity to this parameter (see Section 7).
- 2-, 5-, and 50% AEPs were also modeled for rainfall only and rainfall plus snowmelt conditions.

The model run scenarios above were applied in the model set-up to obtain the results presented in Section 6.

5.2 Napanee River Upper Lakes Model Discretization

The eleven (11) reaches that comprise the Napanee River Upper Lakes study area are individually summarized below.

5.2.1 Reach #1: St. Andrews Lake to Cole Lake

Reach #1 extends from the outlet of St. Andrews Lake to the inlet of Cole Lake (see Figure 5-4). Reach #1 is defined by a series of wetlands, and includes Crossings #1-4. The culvert crossings are summarized in Section 5.3.

Per Section 4.3, St. Andrews Lake is controlled by three 750mm diameter culverts at St. Andrews Lake Lane. It can also be controlled by the backwater imposed by the crossing at the K&P Trail. The wetland upstream of the K&P Trail acts as the tailwater for the St. Andrews Lake Lane crossing and subsequently can affect the St. Andrews Lake water level. Given the series of wetlands that affect the tailwater immediately downstream of St. Andrews Lake, this lake was included as part of the HEC-RAS hydraulic model for Reach #1.

Downstream of the K&P trail there is a wetland at a lower elevation, followed by another wetland on the downstream side of Highway 38. The series of wetlands were modelled using a 2D flow area. The flood limit and water surface elevations (WSELs) depicted in Figure 5-4 are governed by the 1% AEP rainfall plus snowmelt event.

Stage and flow hydrographs for the two crossings that can control the water levels in St. Andrews Lake are provided in Figures 5-5 and 5-6. The stage hydrographs represent the headwater (HW) and tailwater (TW) on each side of the crossing. The flow hydrograph represents the outflow from the culvert crossings.

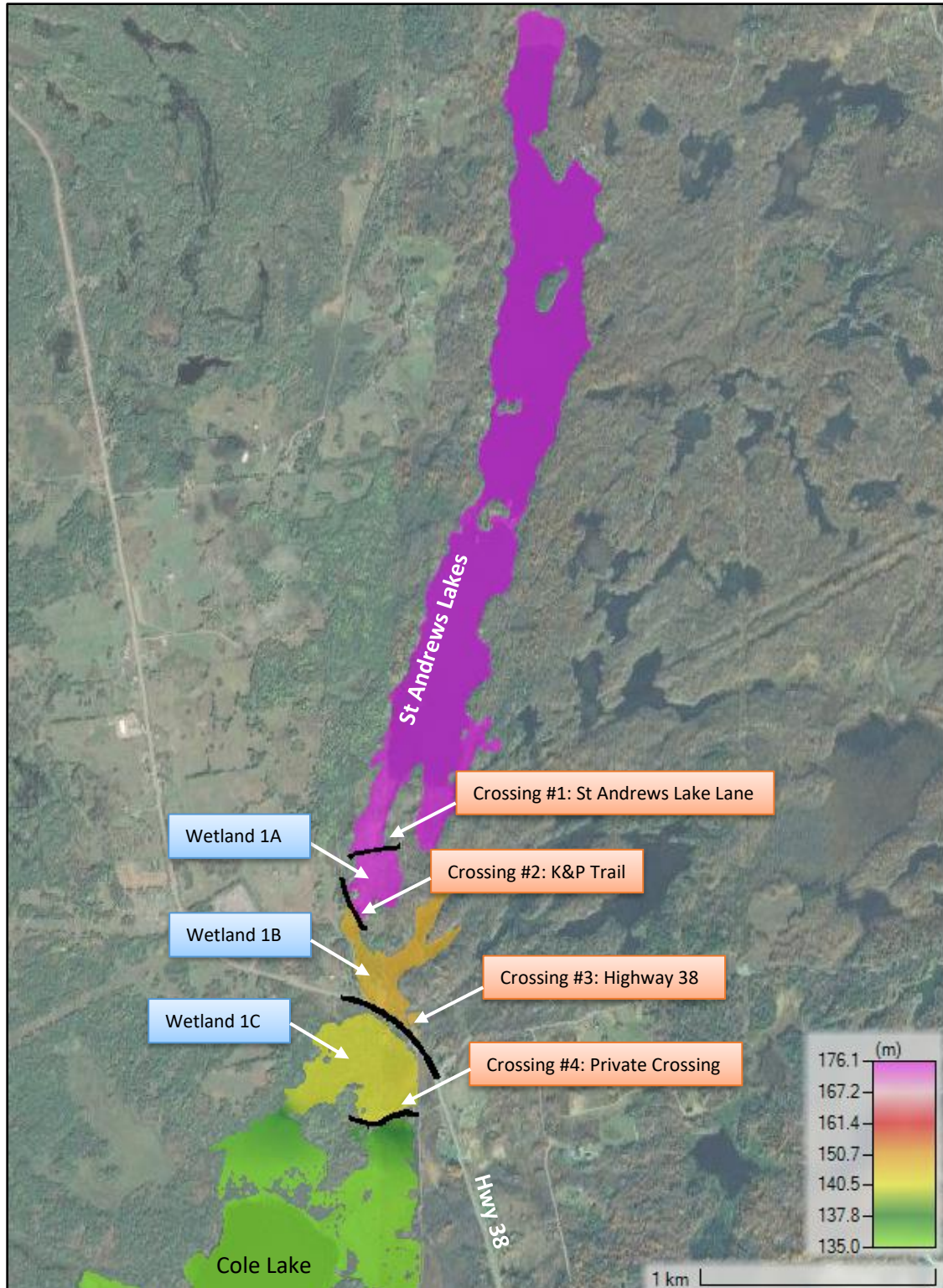


Figure 5-4: Reach#1 - St. Andrews Lake to Cole Lake

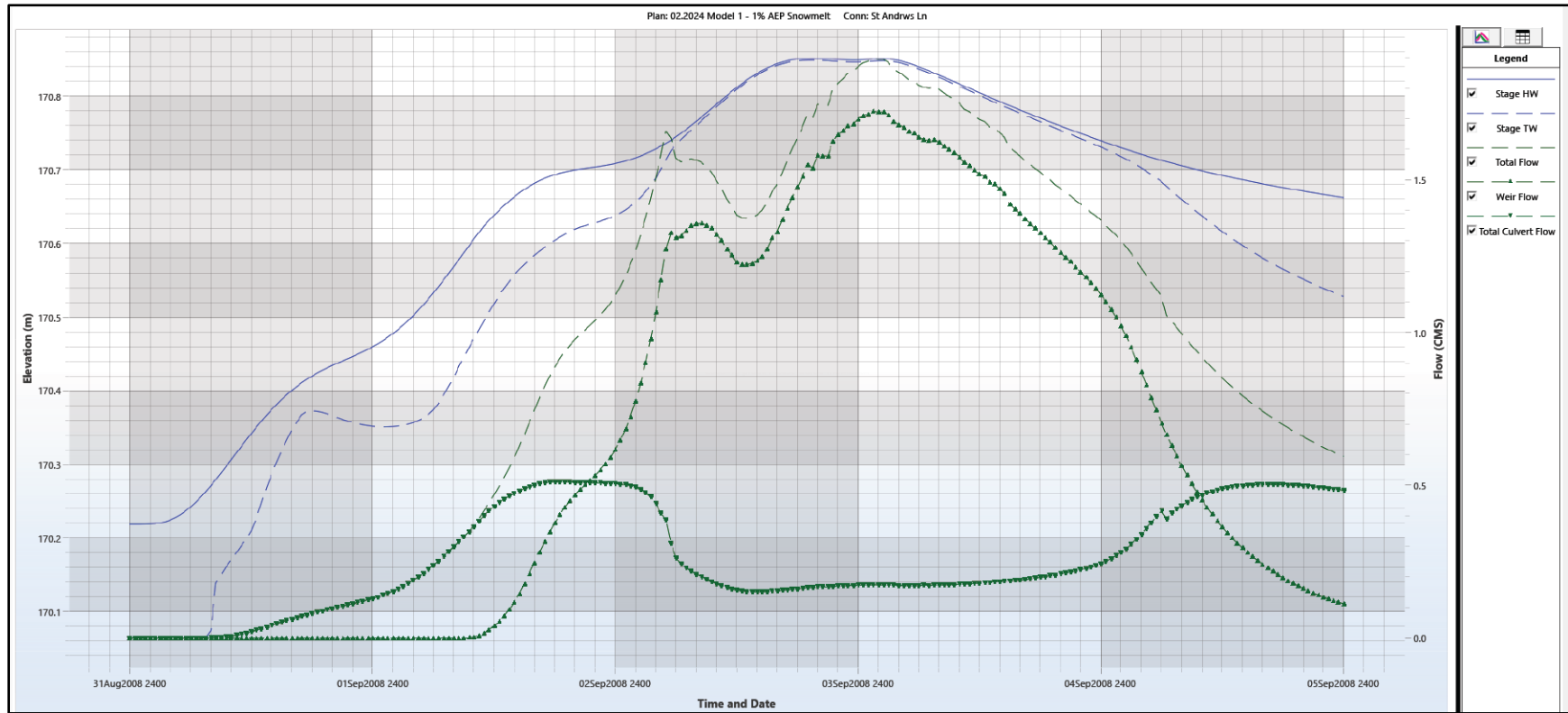


Figure 5-5: St. Andrews Lake Lane Stage and Flow Hydrograph; 1% AEP Rainfall Plus Snowmelt

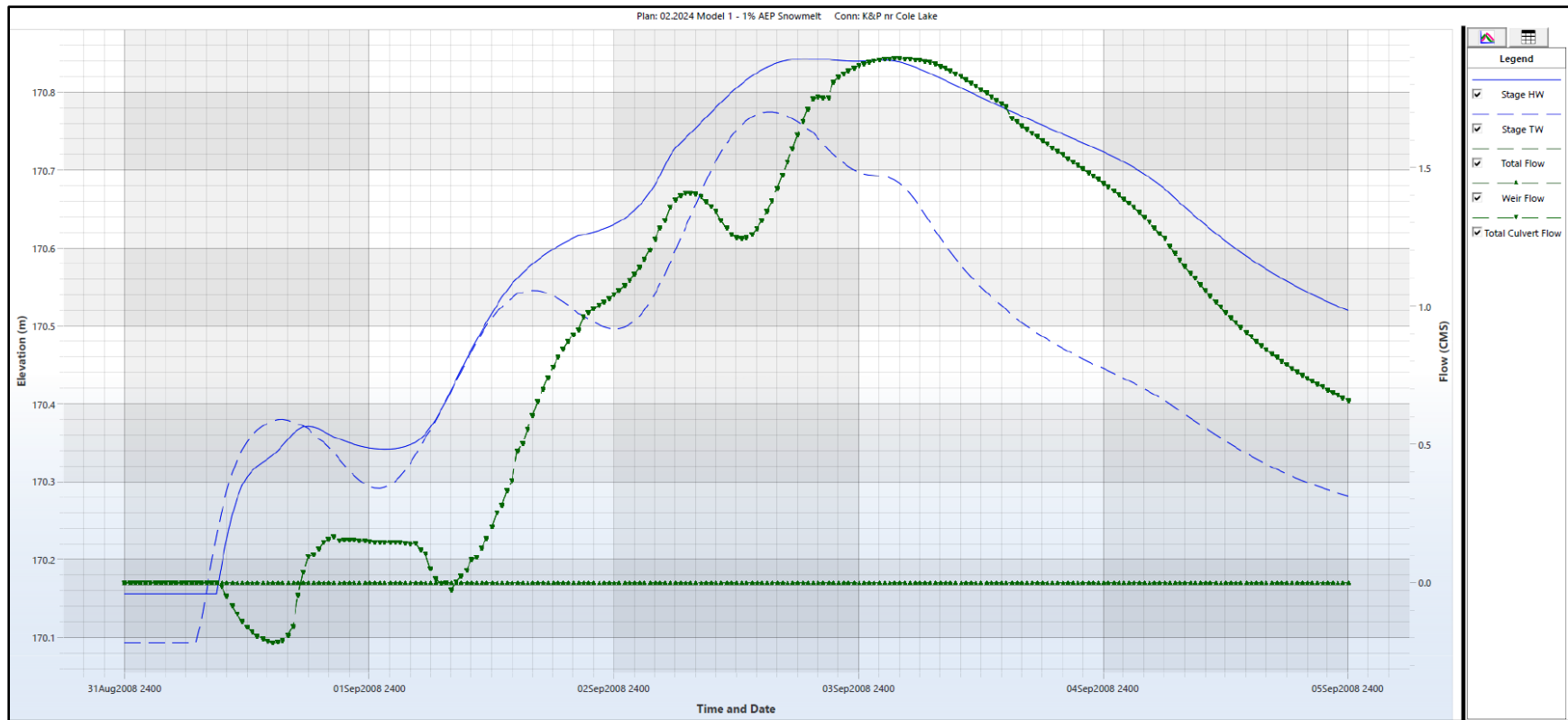


Figure 5-6: K&P Trail Outlet Stage and Flow Hydrographs; 1% AEP Rainfall Plus Snowmelt

5.2.2 Reach #2: Cole Lake to Howes Lake

Reach #2 extends from the outlet of Cole Lake to the inlet of Howes Lake (see Figure 5-7). Reach #2 is defined by a combination of channels and wetlands, and includes Crossings #5-9. The culvert crossings are summarized in Section 5.3.

A beaver dam currently establishes the lake level for Cole Lake. Since beaver dams will not be relied upon as a flood control structure, the beaver dam is removed in the model and a free-flowing channel is used for the outlet. Since there is no reservoir immediately downstream of the Cole Lake outlet, Cole Lake was not modelled in HEC-RAS since the HMS reservoir-routing would be sufficient.

The abrupt change in colour in Figure 5-7 illustrates the presence of the crossings and the backwater they impose near Crossings #5, #6, and #9. Crossing #7 is immediately upstream of a wetland and has lesser backwater impacts upstream.

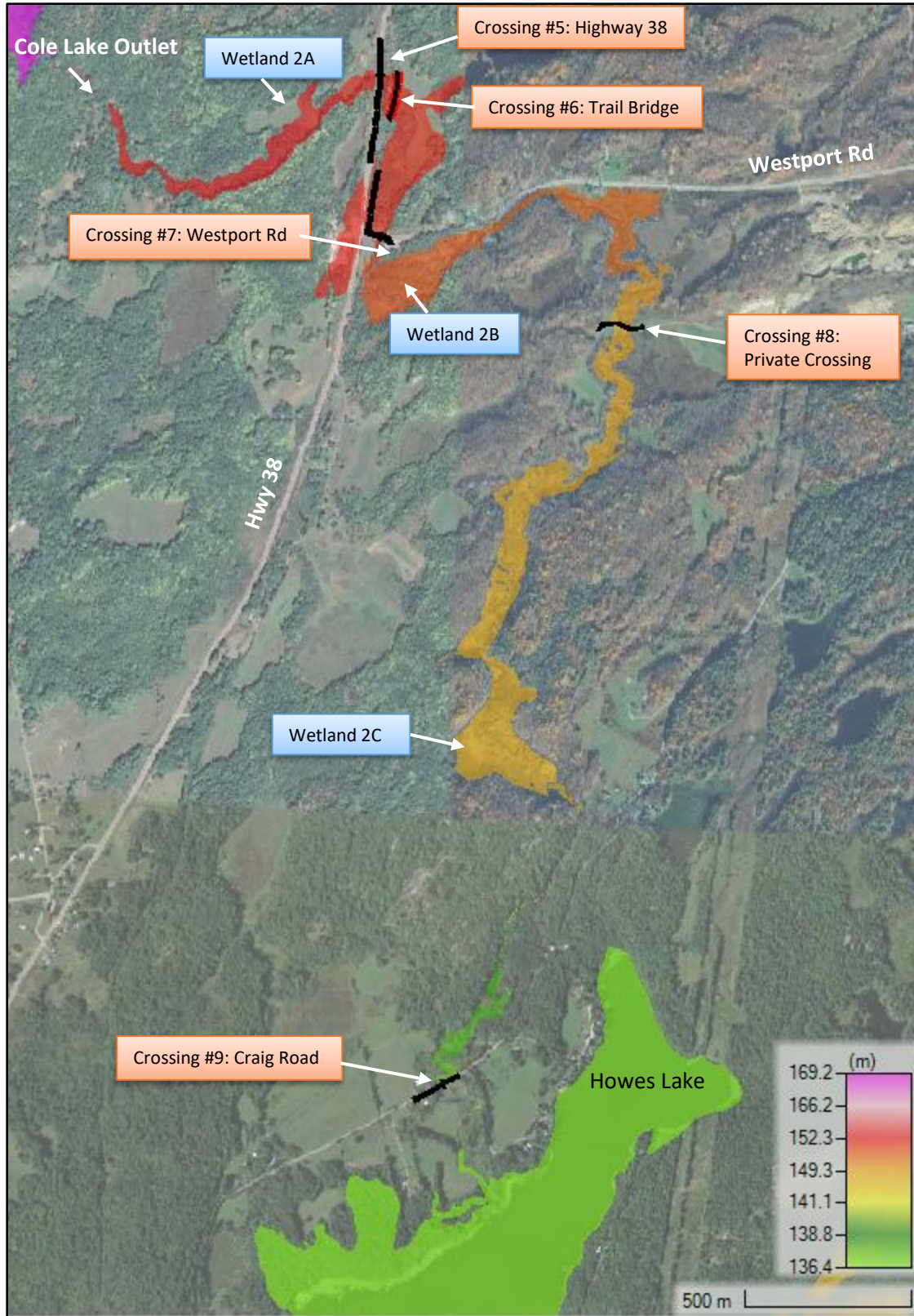


Figure 5-7: Reach #2 – Cole Lake to Howes Lake

5.2.3 Reach #3: Van Luven Lake to Howes Lake

Reach #3 connects Van Luven Lake to Howes Lake (see Figure 5-8). Reach #3 is a relatively short reach and its purpose is to investigate the backwater impacts from Howes Lake on the Van Luven water levels. Crossing #10 is a concrete box culvert across Highway 38 that acts as the control for Van Luven Lake. The rainfall plus snowmelt condition governs the 1% AEP regulatory flood limit. A plot of the stage and flow hydrographs for Van Luven outlet control crossing is provided in Figure 5-9.

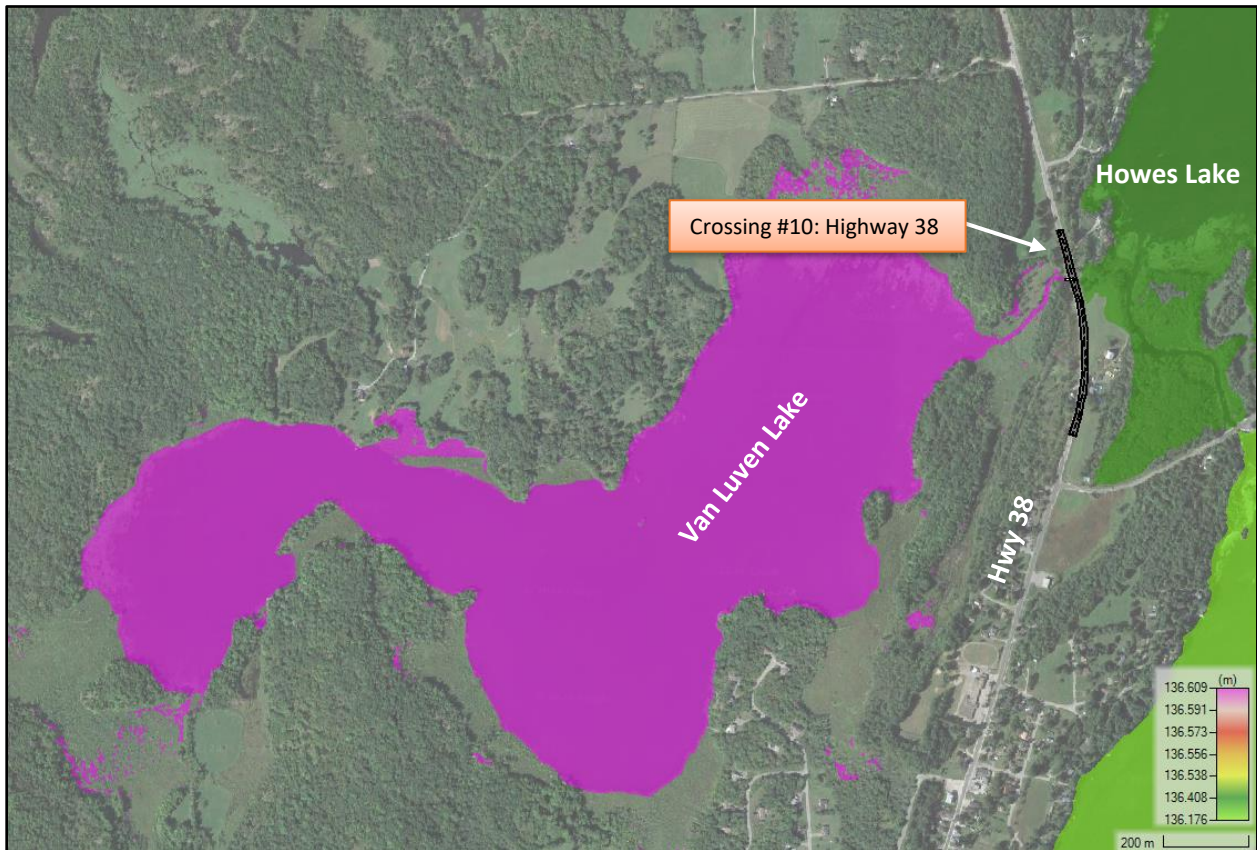


Figure 5-8: Reach #3 Plus Van Luven Lake

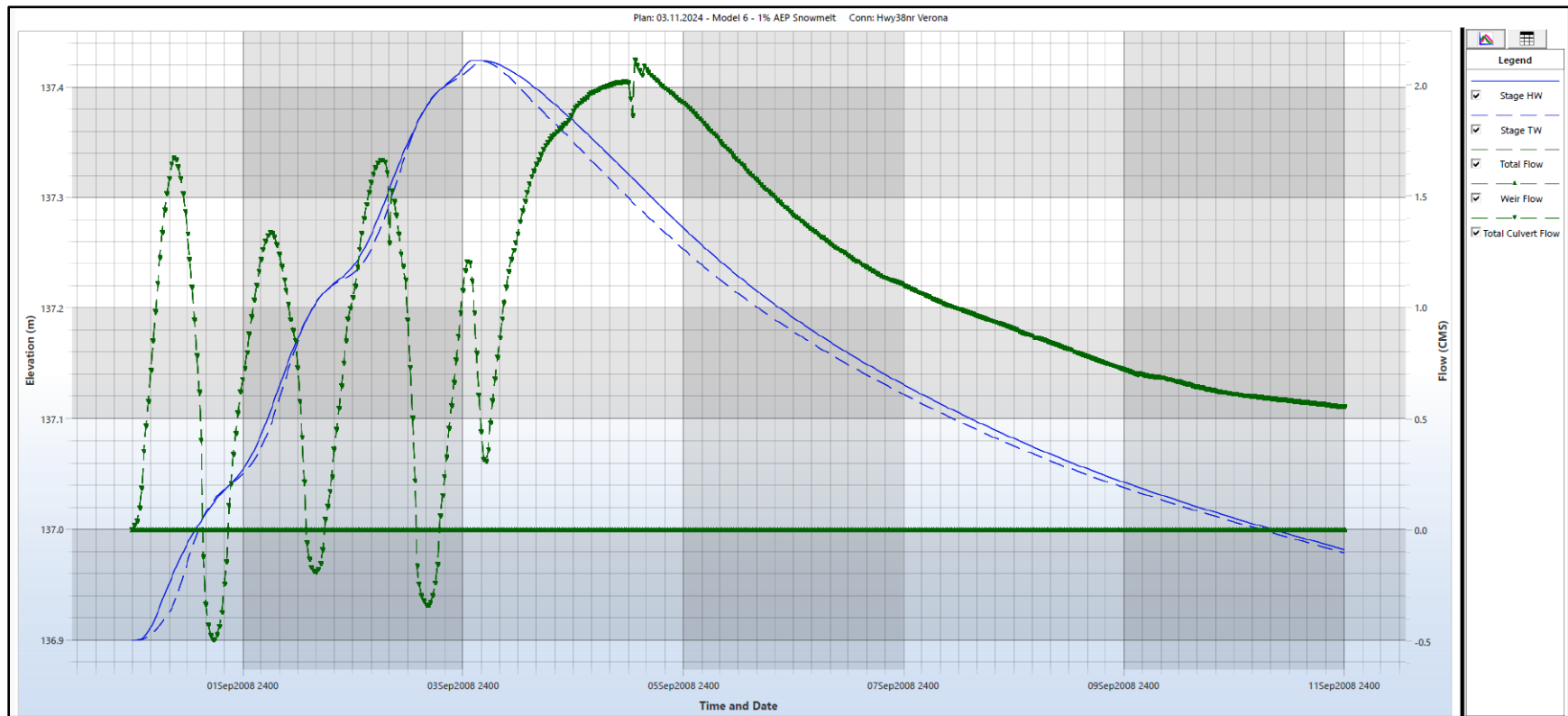


Figure 5-9: Reach #3 - Van Luven Lake Outlet (Crossing #10) Stage and Flow Hydrograph

5.2.4 Reach #4: Howes Lake to Verona Lake

Reach #4 connects Howes Lake to Verona Lake with the bridge at Crossing #11: Desert Lake Road (see Figure 5-10). Howes Lake receives significant inflow peaks and runoff volumes since it is the confluence for the majority of the Napanee River Upper Lakes system. The Desert Lake Road bridge has a large span and the contraction for the outlet channel provides some flow attenuation before runoff enters Verona Lake. Figure 5-11 shows the flow and stage hydrographs for the Desert Lake Road crossing that represents the outlet of the connector channel from Howes Lake to Verona Lake.

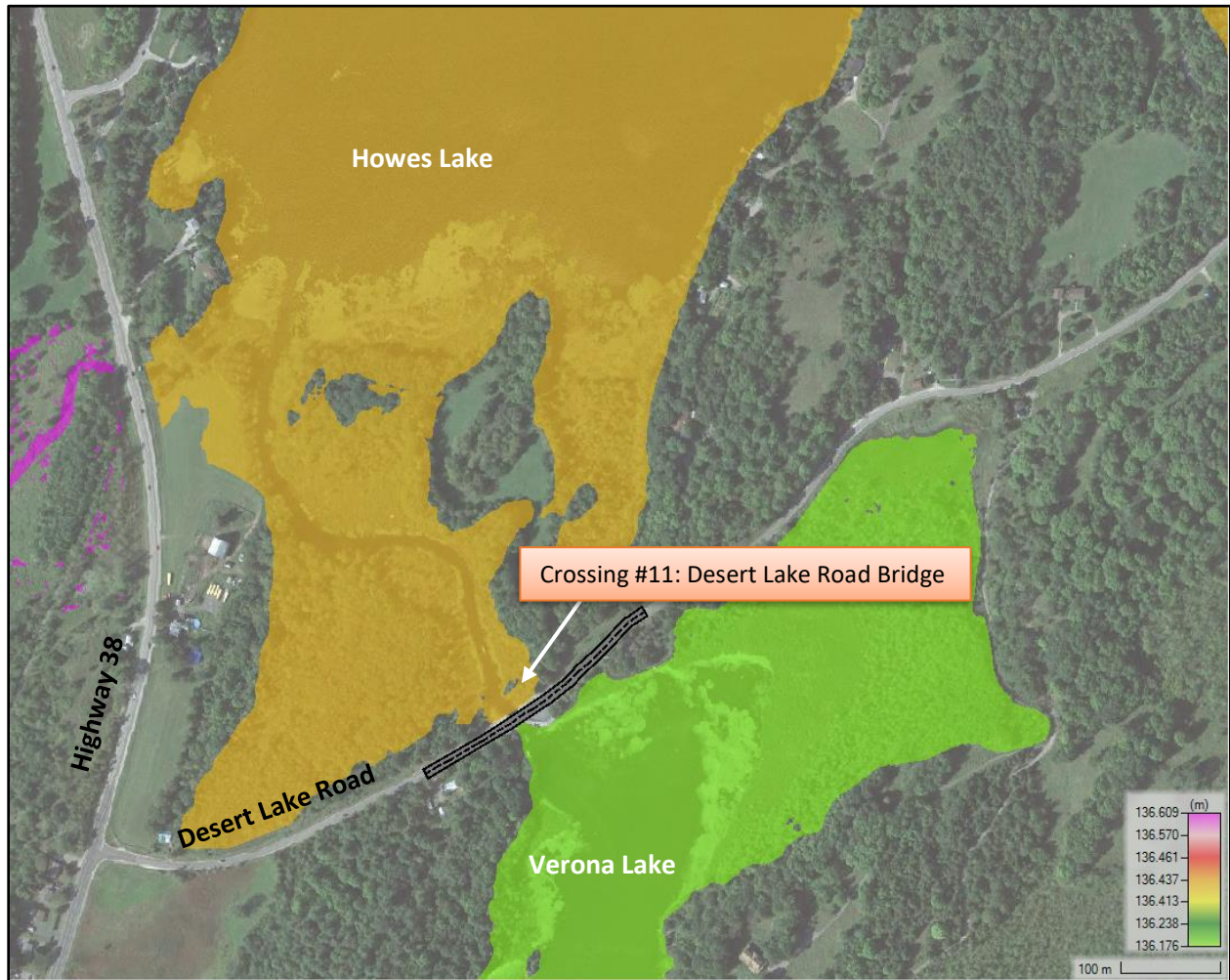


Figure 5-10: Reach #4 – Howes Lake to Verona Lake

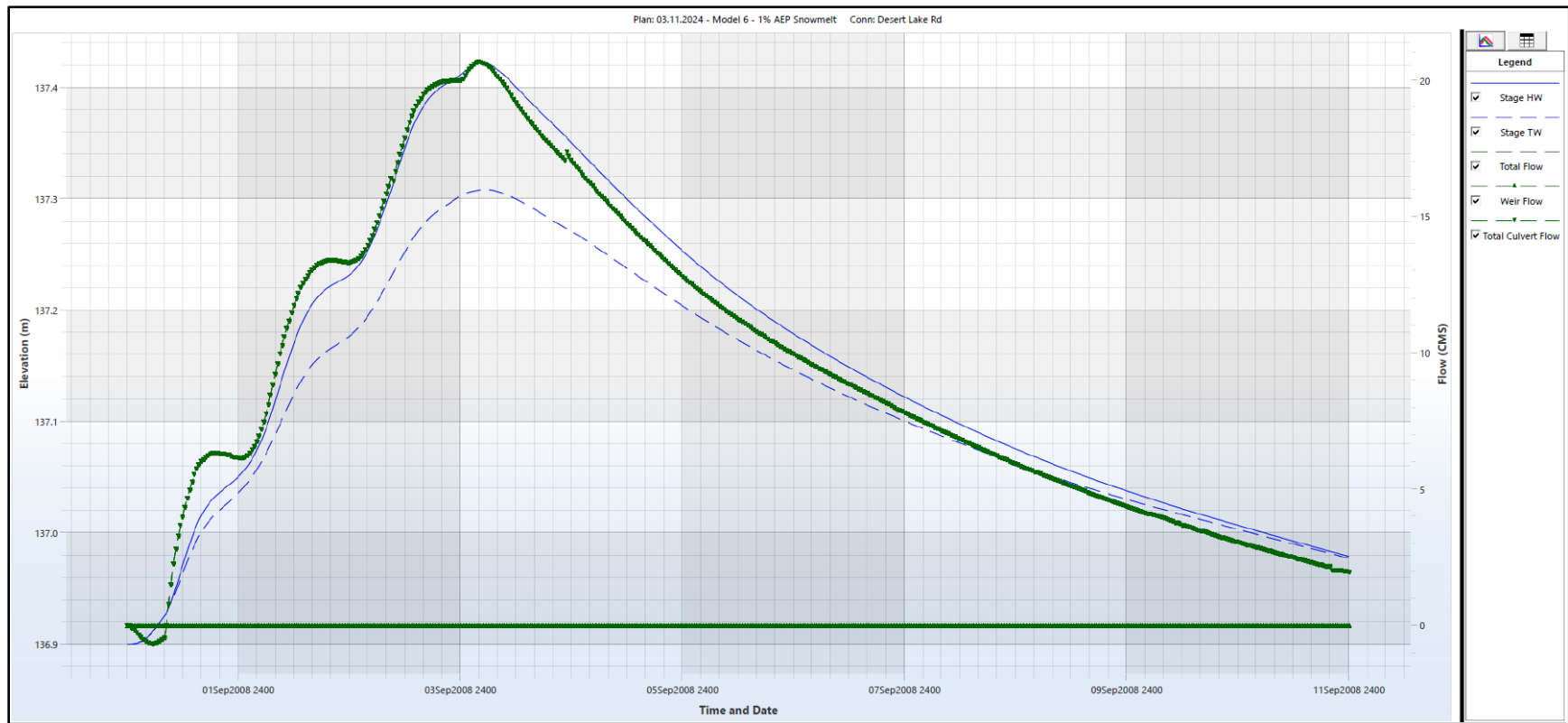


Figure 5-11: Stage and Flow Hydrograph for Reach #4 Outlet at Desert Lake Road for 1% AEP Snowmelt Plus Rain Event

5.2.5 Reach #5: Hambly Lake to Verona Outlet Channel

Reach #5 is the outlet channel from Hambly to Verona Lake. The Cedarwoods Drive crossing (Crossing #12) is within this reach. The Cameron Swamp controls the regulatory water level for Hambly Lake similar to the Cameron Swamp controls for Verona Lake (see Figure 5-12).

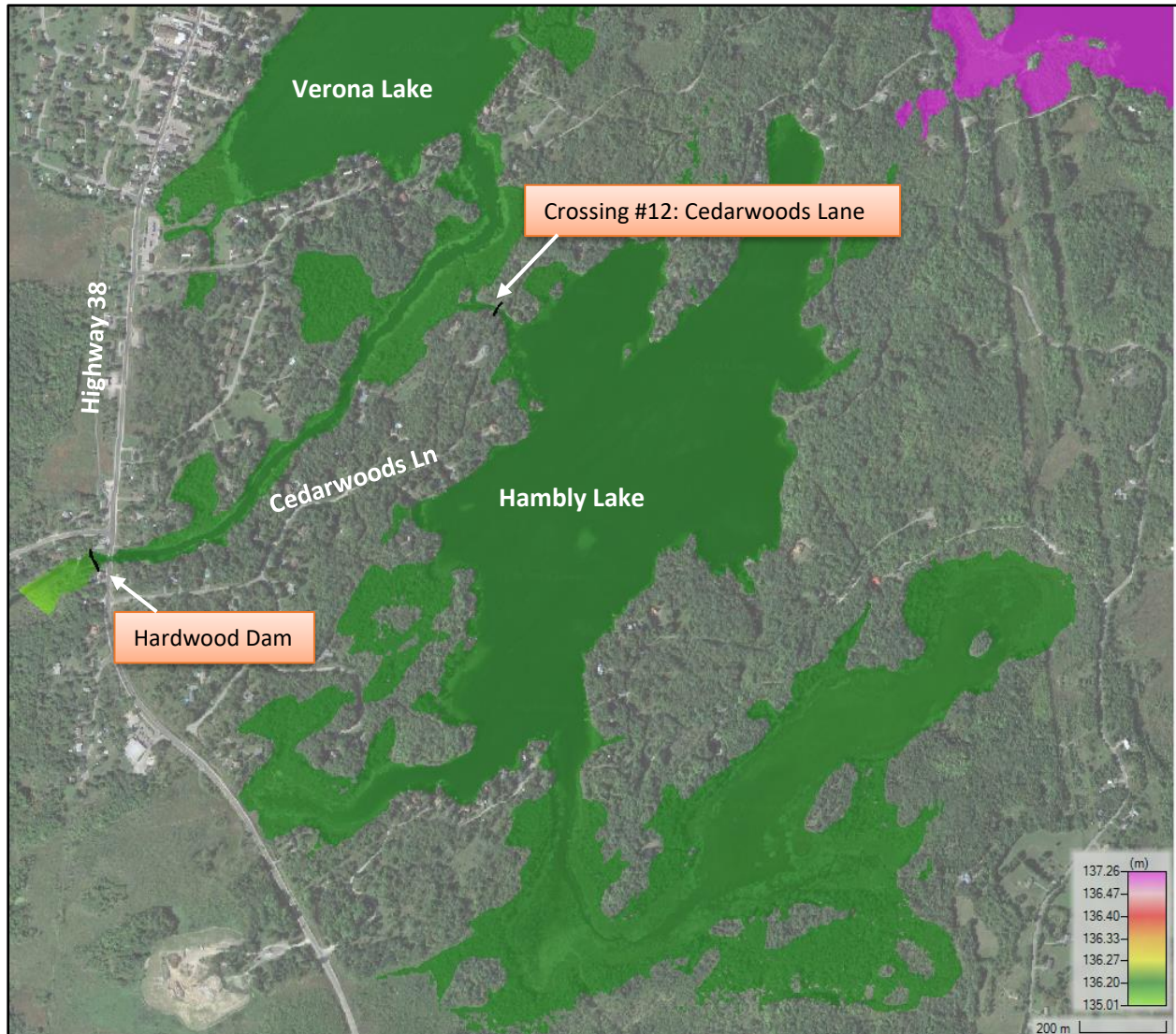


Figure 5-12: Reach #5 and Hambly Lake

5.2.6 Reach #6: White Lake to 13 Island Lake

Reach #6 extends from the outlet of White Lake to the inlet of Thirteen Island Lake (see Figure 5-13). Reach #6 is defined by a combination of channels and wetlands, and includes Crossings #13-17.

The outlet of White Lake is currently a beaver dam; the beaver dam was removed in the model to allow the outlet control to be the natural channel outlet. The White Lake channel outlet and the entirety of Reach #6 is modeled with a 2D flow area. The natural channel outlet for White Lake outlets to a wetland upstream of Buck Bay Road. Buck Bay Road is also bound by a wetland on its downstream side. With the

2D flow area and topographic data in the terrain model, the TW condition is accounted for in the hydraulic calculations.

The remaining crossings within Reach #6 have similar characteristics to Buck Bay Road; meaning they are bound by wetland areas. Bunker Hill Road A (Crossing #17) is the most downstream crossing within this reach and its TW condition is set by the stage hydrograph applied at Thirteen Island Lake.

Section 3 showed the 1% AEP rainfall event produces the largest peak flows from individual sub-catchments, whereas the 1% AEP rainfall plus snowmelt event produces the larger lake elevations and lake outflows due to the greater runoff volume in snowmelt events that overwhelms the lake storage capacities. Reach #6 is governed by the peak flows from the 1% AEP rainfall event rather than the snowmelt scenario since Sub-Catchment 1300 drains directly to the river reach without flow attenuation.

5.2.7 Reach #7: Potspoon Lake to 30 Island Lake

Reach #7 connects Potspoon Lake to Thirty Island Lake (see Figure 5-14). Potspoon Lake outlets to a channel that crosses Sperling Road. The hydrograph at Sperling Road is also representative of the outflows from Potspoon Lake given its proximity to the Potspoon Lake outlet (see Figure 5-15).

Downstream of Sperling Road, there are two smaller lakes that are modelled as storage areas. These two smaller lakes have different elevations, and the connection between the two is currently a beaver dam. The beaver dam is removed in the hydraulic model to allow the connection between the two lakes to be the natural constriction point at the downstream end of the upper reservoir (Connector Lake 7A). The lower reservoir (Connector Lake 7B) is connected to the 2D flow area that includes the creek channel that crosses McColl Lane and outlets to Thirty Island Lake. The stage hydrograph for Thirty Island Lake was applied as the downstream boundary condition for Reach #7.

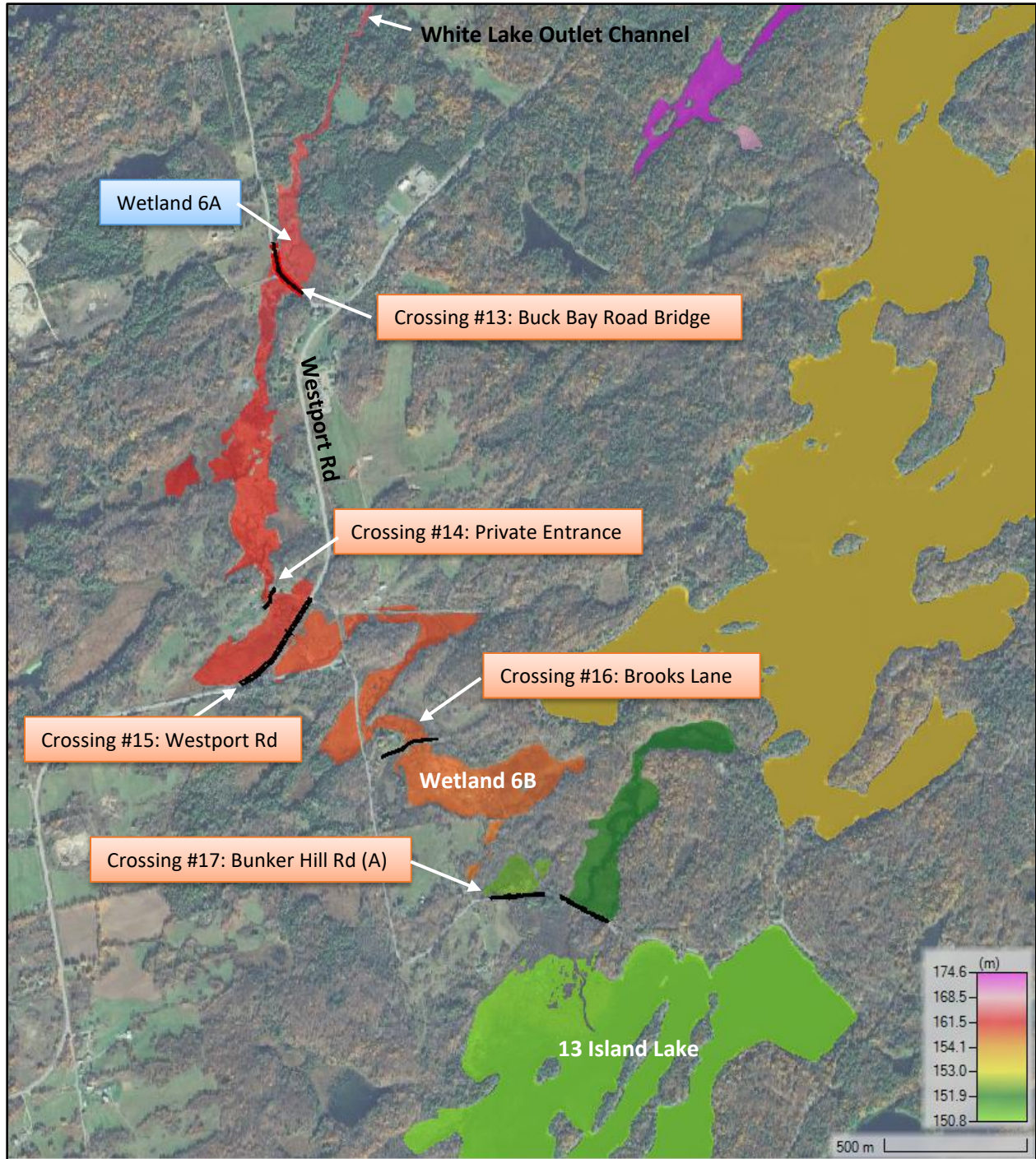


Figure 5-13: Reach #6 - White Lake to 13 Island Lake

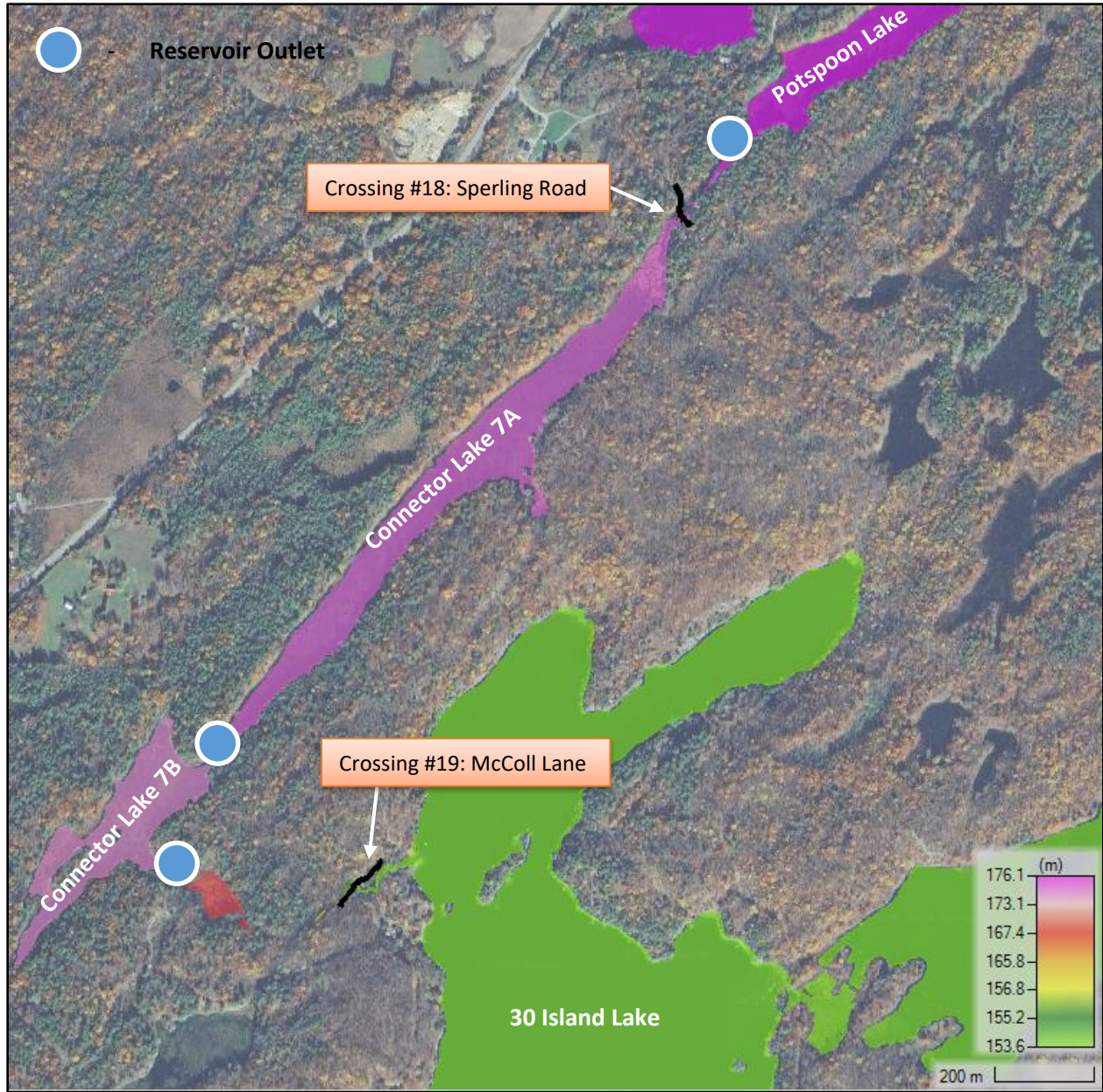


Figure 5-14: Reach #7

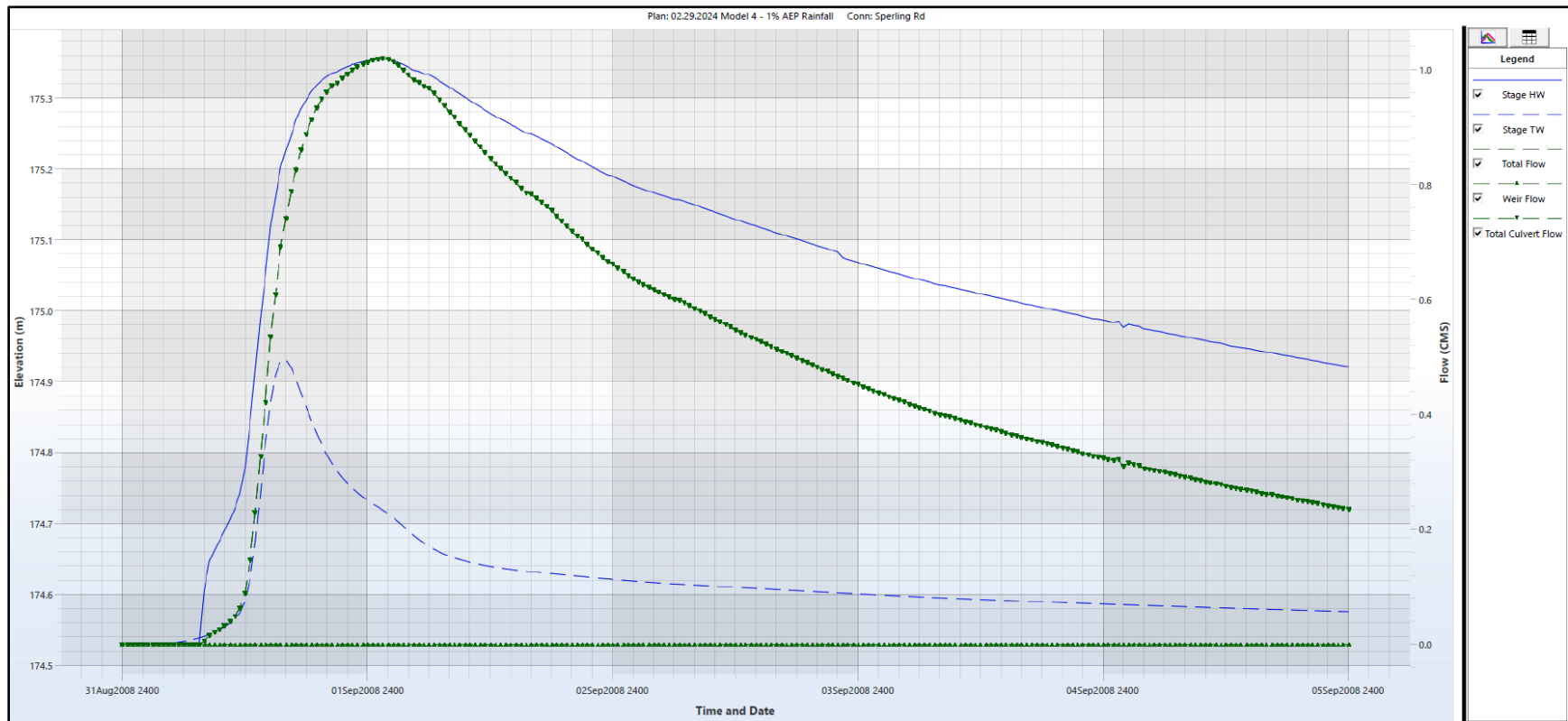


Figure 5-15: Stage and Flow Hydrograph for Sperling Road near Potspoorn Lake Outlet; 1% AEP Snowmelt + Rain Event

5.2.8 Reach #8: 30 Island Lake to 13 Island Lake

Reach #8 represents the channel and two wetlands between Thirty Island Lake and Thirteen Island Lake.

The outlet from Thirty Island Lake is currently a beaver dam that was removed in the hydraulic model. There is a natural channel that is very narrow; within this narrow channel there is a private crossing that outlets to a well-defined wetland area. This wetland area had a shallow pool of water at the time of the field visit due to beaver activity between Wetlands 8A and 8B (see Figure 5-16). A local resident also noted that there is a steady flow of water from underneath the bedrock between Thirty Island Lake and Wetland 8A. The water daylights downstream of the private crossing, and may contribute to lower lake levels in 30 Island Lake during dry periods. The downstream connection for Wetland 8B is Bunker Hill Road B (Crossing #21).

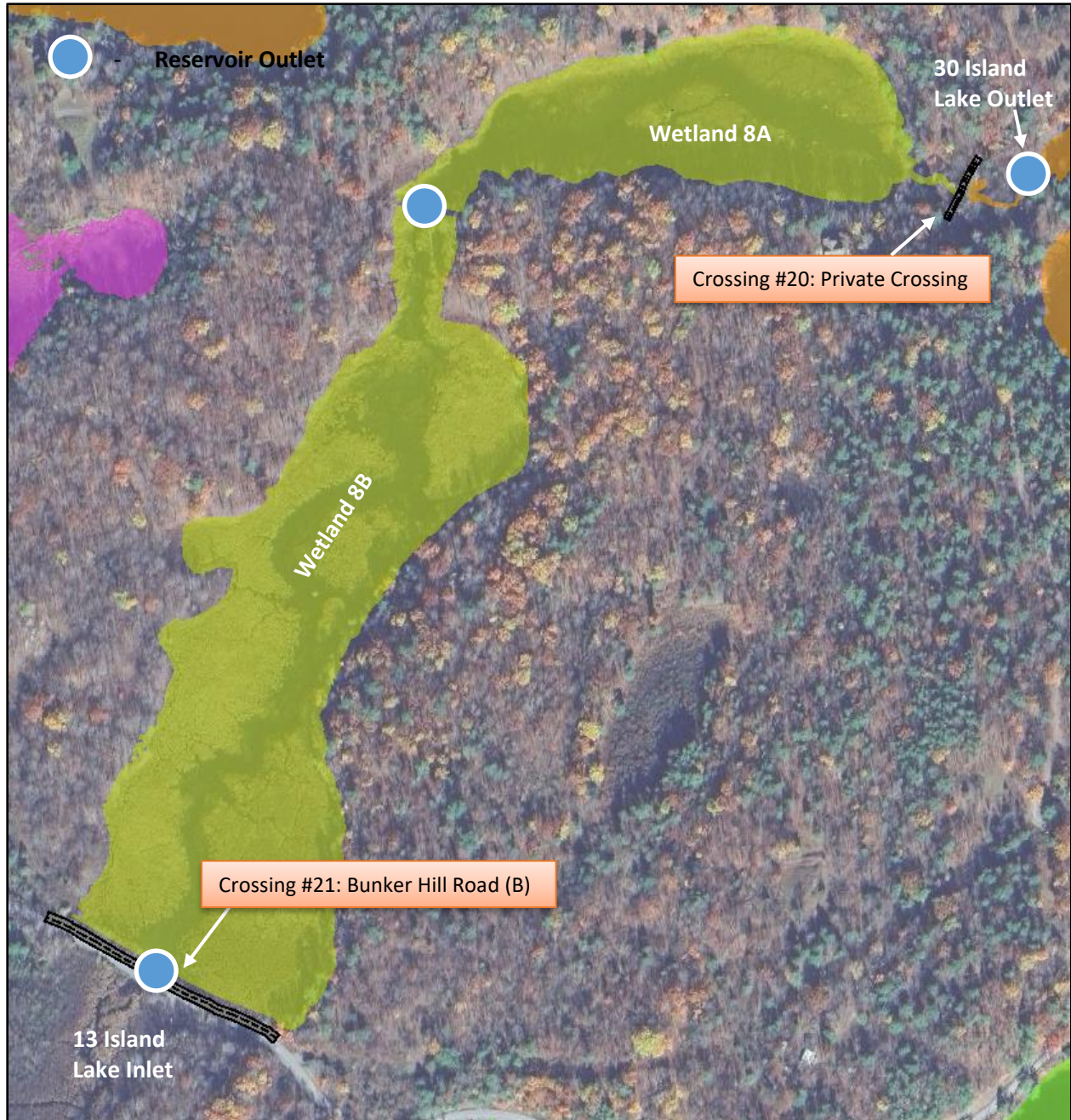


Figure 5-16: Reach #8

5.2.9 Reach #9: 13 Island Lake to Howes Lake

Reach #9 is predominantly a series of two smaller lakes that connect 13 Island Lake to Howes Lake (see Figure 5-17). This includes a connector lake with a longitudinal shape that outlets to Bass Lake. The two lakes were modelled as storage areas due to their size and subsequent storage capacities that would contribute to flow attenuation. They are connected by a narrow channel that was simulated as a storage area connection in the hydraulic model.

The outlet for Bass Lake is currently a beaver dam, however the beaver dam was removed in the hydraulic model and the culvert at Hinchinbrooke Road North is the outlet control for Bass Lake. On the downstream side of Bass Lake there is a steep drop off to Howes Lake. With the sudden drop in elevation, the water levels within Reach #9 are unaffected by the water levels in Howes Lake.

The stage and flow hydrograph for the Hinchinbrooke Road culvert control of Bass Lake is provided in Figure 5-18).

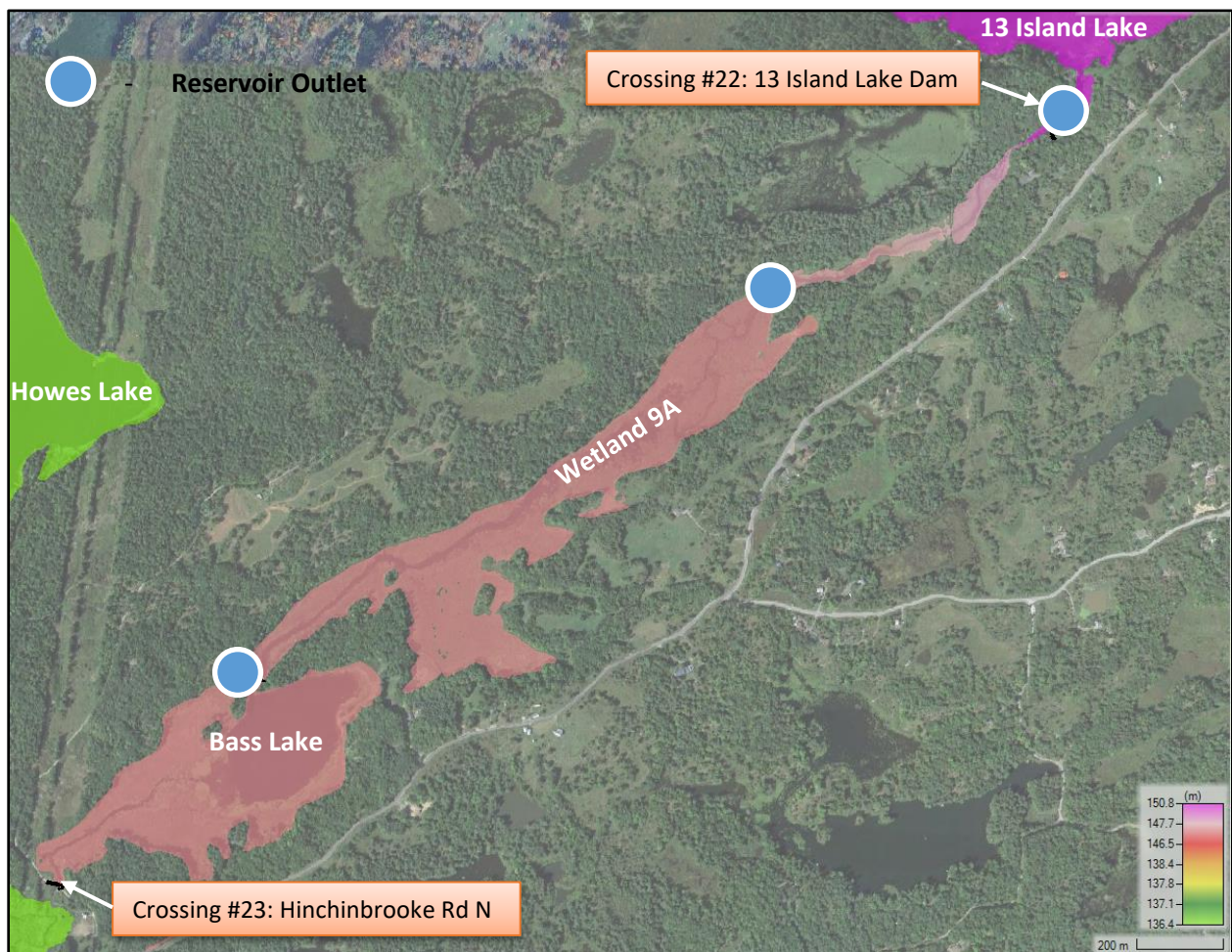


Figure 5-17: Reach #9

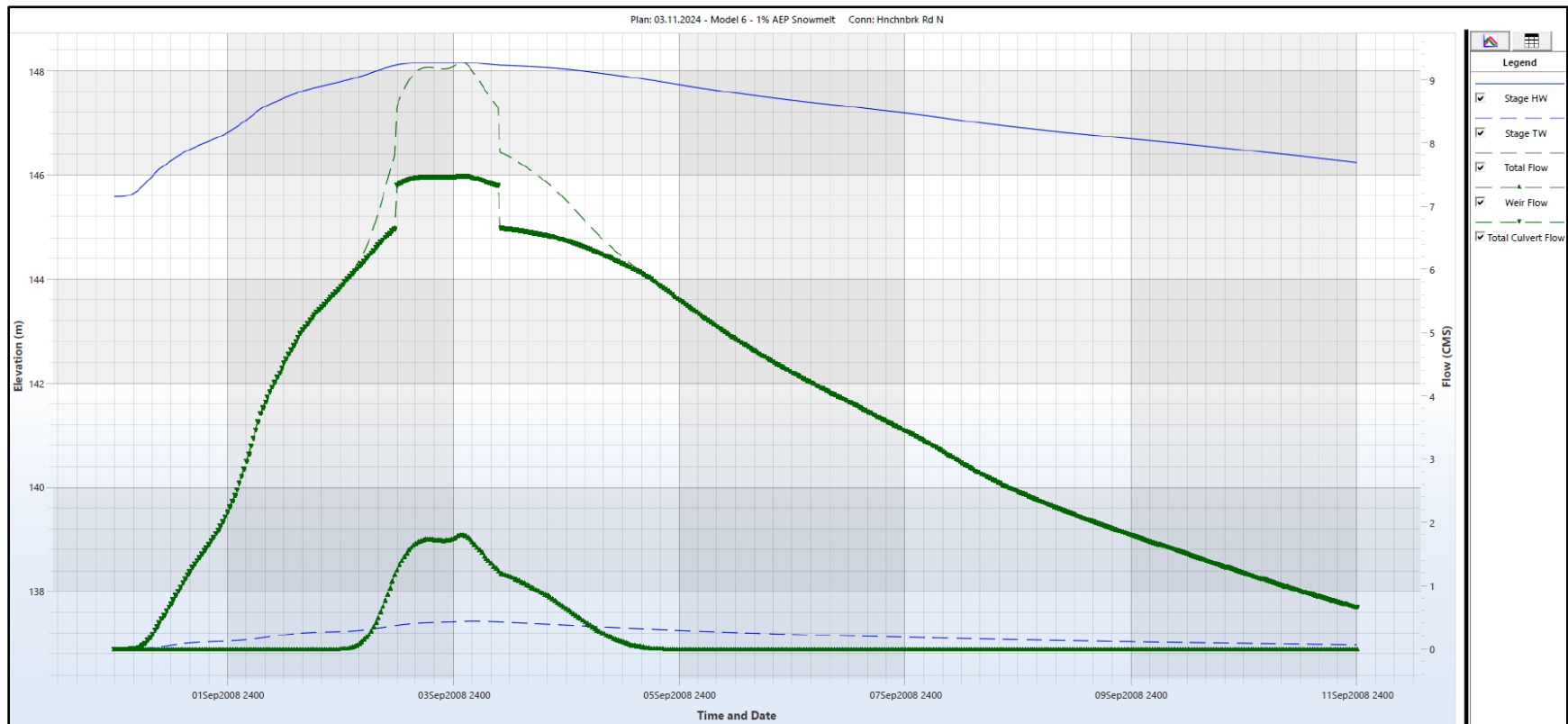


Figure 5-18: Hinchinbrooke Road North Stage and Flow Hydrograph Control of Bass Lake; 1% AEP Rainfall Plus Snowmelt Event

5.2.10 Reach #10: Fourteen Island Lake to Verona Lake

The upstream limit of Reach #10 is the Fourteen Island Lake dam outlet; the downstream limit of Reach #10 is the channel outlet to Verona Lake (see Figure 5-19). The Fourteen Island Lake Dam outlets to a short stretch of channel and wetland area before entering Spring Lake. Spring Lake is controlled by the culvert crossing at Hinchinbrooke Road and drains to Little Mud Lake. Little Mud Lake outlets at a constriction point into a short channel that inlets to Verona Lake. A stage hydrograph for Verona Lake is the downstream boundary condition for Reach #10.

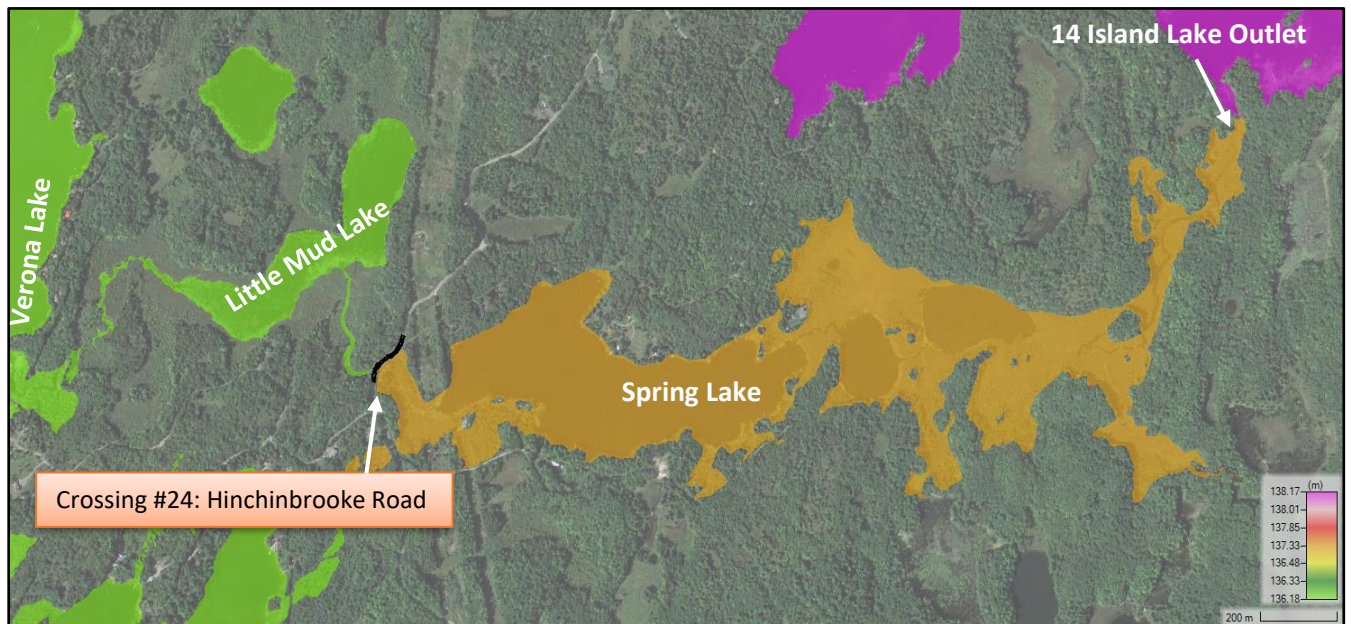


Figure 5-19: Reach #10; Depth Overlay

5.2.11 Reach #11: Verona Lake Outlet Channel to Hardwood Dam

Reach #11 is represented by the Verona Lake outlet channel that is controlled by the Hardwood Dam in summer conditions (see Figure 5-20). This is an important location in the hydraulic model because the reach inflows account for all upstream lake, reservoir, and channel routing. Section 4 concluded the flow attenuation in the lakes is a critical component of the Napanee River Upper Lakes system due to the amount of storage held within each of the main lakes. Since the lakes and their natural channel outlets can reduce peak inflow rates by factors up to 5 to 10 times (depending on the lake and outlet configuration), the outflow hydrographs at Hardwood Dam in the hydraulic model are of interest given that it accounts for backwater impacts, storage areas, and intermittent natural outlet controls throughout the eleven (11) river reaches described herein.

Figure 5-20 presents the outflow hydrograph at the Hardwood Dam for the 1% AEP spring melt event.

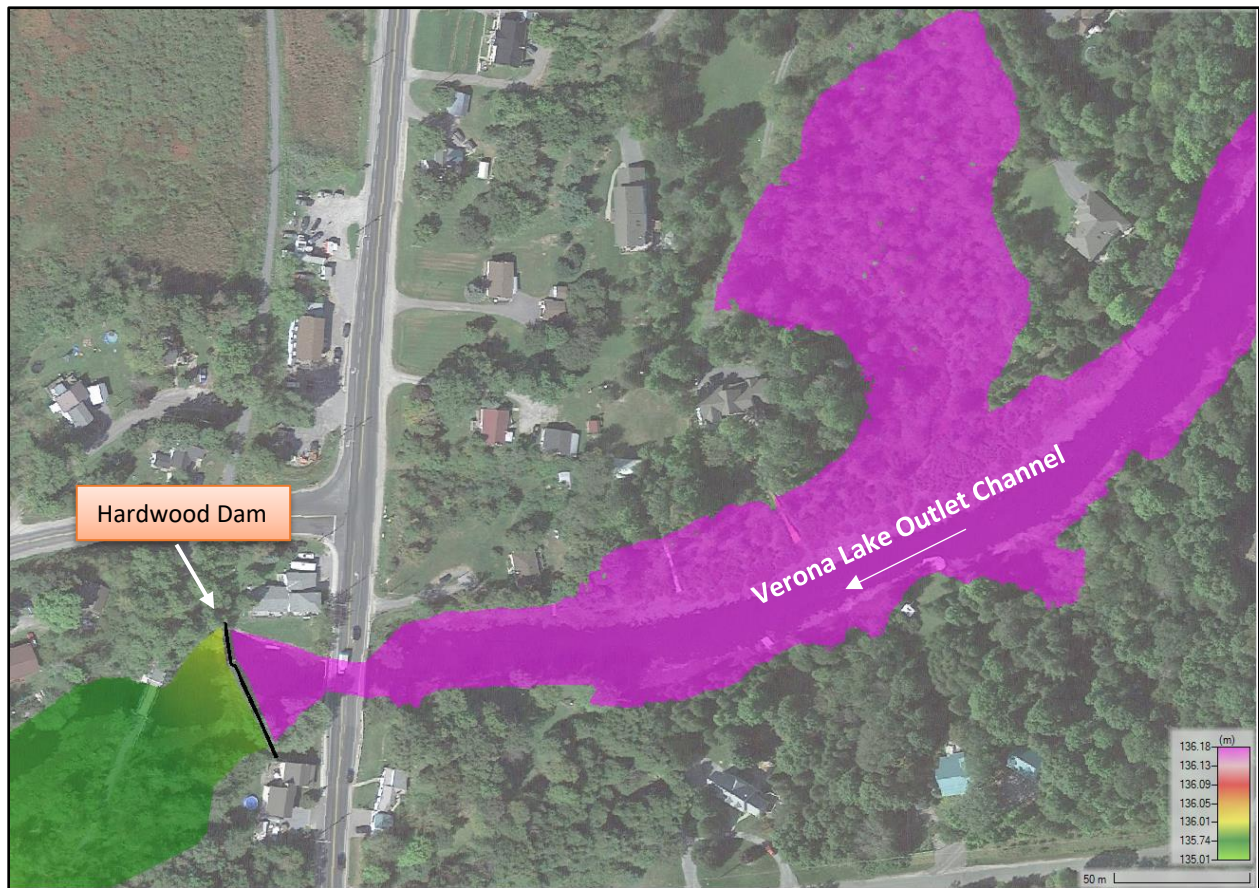


Figure 5-20: Reach #11

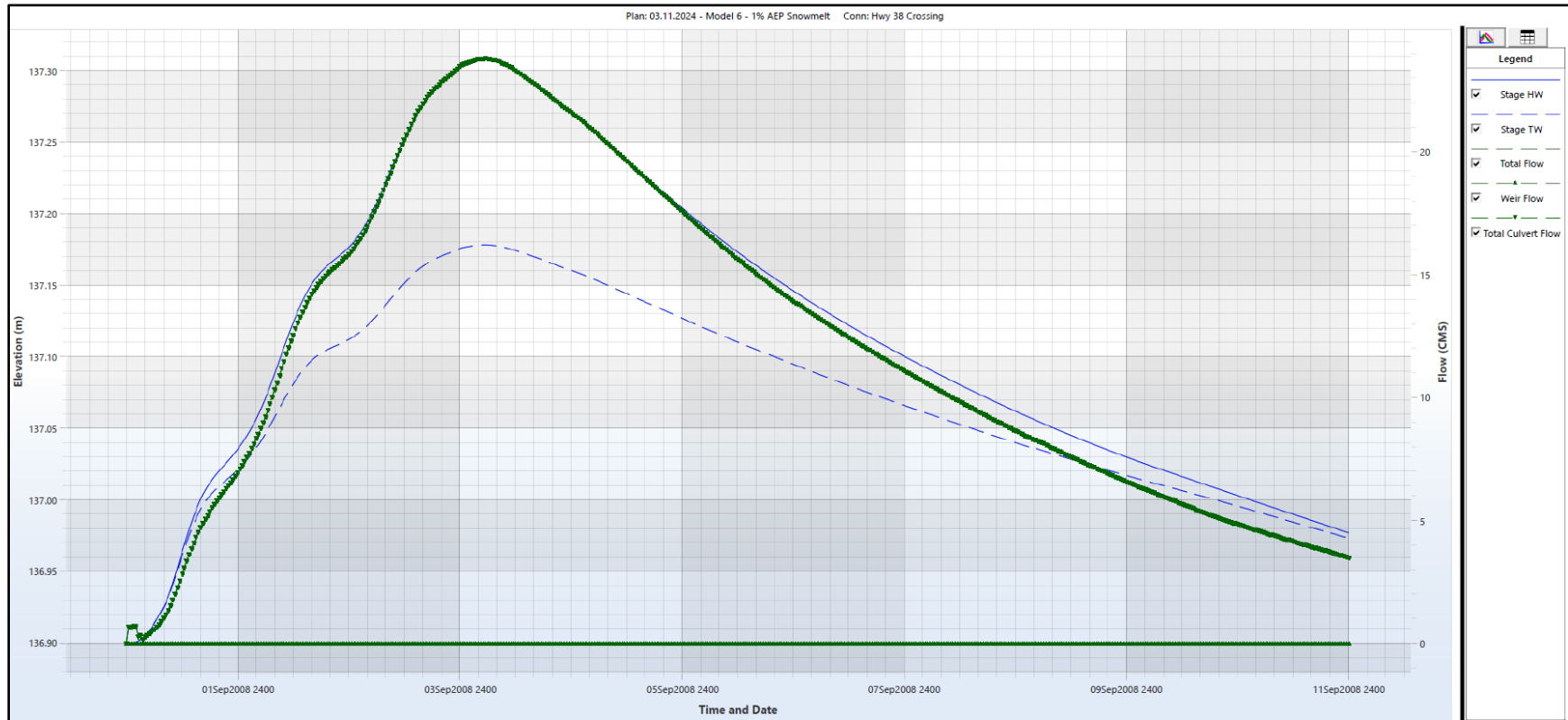


Figure 5-21: Highway 38 Stage and Flow Hydrograph; 1% AEP Rainfall Plus Snowmelt Event

5.3 Culvert & Bridge Crossings

There are twenty-six (26) crossings within the study area as previously depicted throughout Subsection 5.2. Tables 5-2 and 5-3 summarize the type of each crossing, its governing 1% AEP peak flow, and its corresponding headwater (HW) and tailwater (TW) elevations. The maximum relief flow depths are also included.

The governing peak flow and relief flow depth are based on the greater peak flow received from the 1% AEP rainfall or spring melt scenario since some crossings are less 'protected' by upstream flow attenuation than others. Recall that the rainfall-only events are more susceptible to flow attenuation by the lakes and intermittent wetlands relative to the snowmelt plus rain events. Therefore, crossings without upstream flow attenuation tend to receive greater peak flows from the 1% AEP rainfall only event, whereas the outflows for the main lakes and their immediate downstream crossings have greater peaks in the 1% AEP spring melt condition. The governing condition is included as a column in the tables below.

Some stage and flow hydrographs were presented within the discussion in Section 5.2; Appendix I provides the stage and flow hydrographs for each crossing. Culvert/bridge data sheets are provided in Appendix J.

Table 5-2: Summary of Crossings #1-12

Crossing #	Name	Type	Low Point of Road (m)	Governing 1% AEP Event	Governing 1% AEP Q_{peak}	HW_{peak}	TW_{peak}	ΔH_{peak}	Max. Relief Flow Depth
				m	m^3/s	m			
Reach 1: St Andrew Lakes Ln to Howes Lake									
1	St. Andrews Lakes Lane	Three (3) 750mm Φ HDPE culverts	170.59	Spring melt	1.9	170.85	170.85	0.00	0.26
2	K&P Trail	3m span x 1.5m rise concrete box culvert	171.13	Spring melt	1.9	170.84	170.77	0.07	0.00
3	Highway 38 near Cole Lake	3m span x 2.1m rise concrete box culvert	170.99	Spring melt	3.6	170.75	170.38	0.37	0.00
4	Private Crossing: 8985 B Hwy 38	Assumed 400mm diameter driveway culvert	169.47	Spring melt	2.0	169.66	169.35	0.31	0.19
Reach 2: Cole Lake to Howes Lake									
5	Hwy 38 near Godfrey	4m span x 2m rise concrete box culvert	153.52	Rainfall	11.7	154.65	152.94	1.71	1.13
6	K&P Trail near Godfrey	4m span bridge	154.28	Rainfall	11.7	152.74	152.66	0.08	0.00
7	Westport Road near Godfrey	3.2m span x 2.4m rise CSP arch culvert	151.66	Rainfall	20.6	152.47	152.11	0.36	0.81
8	Private Crossing: 7297 Hinchinbrooke Rd N	Assumed 400mm diameter driveway culvert.	148.94	Rainfall	34.2	150.30	150.26	0.04	1.36
9	Craig Road	4.4m span x 2.9m rise CSP arch culvert	139.31	Rainfall	27.6	139.12	138.26	0.86	0.00
Reach 3: Van Luven Lake to Howes Lake									
10	Highway 38 near Howes Lake	3m span x 1.5m rise concrete box culvert	138.22	Spring melt	21.9	137.29	137.18	0.11	0.00
Reach 4: Howes Lake to Verona Lake									
11	Desert Lake Road Bridge	10.0m span bridge	139.16	Spring melt	17.8	137.38	137.29	0.09	0.00
Reach 5: Hambly Lake to Verona Outlet Channel									
12	Cedarwoods Lane	2.4m span concrete box culvert	137.20	Spring melt	2.2	137.31	137.29	0.02	0.11

Table 5-3: Summary of Crossings #13-26

Reach 6: White Lake to 13 Island Lake									
13	Buck Bay Road Bridge	9.1m span bridge	162.74	Rainfall	12.2	163.26	163.17	0.09	0.52
14	Private Crossing	Assumed 400mm diameter driveway culvert	161.00	Rainfall	20.2	161.44	161.24	0.20	0.44
15	Westport Road near Glendower	1800mm Φ CSP culvert	160.68	Rainfall	21.9	161.14	160.13	1.01	0.46
16	Brooks Lane	1.4m x 1.2m CSP arch culvert	158.13	Rainfall	16.0	158.62	158.40	0.22	0.49
17	Bunker Hill Road A	1800mm Φ CSP culvert	152.22	Rainfall	14.4	152.68	152.14	0.54	0.46
Reach 7: Potspoon Lake to 30 Island Lake									
18	Sperling Road	1600mm Φ CSP culvert	175.41	Rainfall	1.0	175.35	174.93	0.42	0.00
19	McColl Lane	Two 600mm Φ CSP culverts	154.59	Rainfall	3.7	155.17	155.16	0.01	0.58
Reach 8: 30 Island Lake to 13 Island Lake									
20	Private Crossing	Assumed 400mm diameter driveway culvert	155.13	Spring melt	7.1	155.58	152.49	3.09	0.45
21	Bunker Hill Road B	1200mm Φ CSP culvert	151.84	Rainfall	0.1	151.56	151.56	0.00	0.00
Reach 9: 13 Island Lake to Howes Lake									
22	13 Island Lake Dam	Dam with three (3) stoplog bays	151.40	Spring melt	8.1	151.50	150.43	1.07	0.10
23	Hinchinbrooke Road North	1800mm Φ CSP culvert	147.76	Spring melt	9.3	148.16	137.42	10.74	0.40
Reach 10: 14 Island Lake to Verona Lake									
24	Hinchinbrooke Road	2400mm Φ CSP culvert	137.81	Spring melt	2.2	137.40	137.31	0.09	0.00
Reach 11: Verona Lake Outlet Channel to Hardwood Dam									
26	Highway 38 near Verona	11.3m span bridge	138.29	Spring melt	2.1	137.42	137.42	0.00	0.00

6 Water Level Summary

The water levels are discussed in this section for the 0.5%, 1%, and 10% AEP events. The 0.5% AEP event is included as the climate change condition for the spring melt plus rain scenario. A separate scenario for climate change based on forecast temperature and rainfall increases is also included per Section 4.6.

The water levels summarized in this section primarily focus on the core Napanee River Upper Lakes; however, results for the governing 1% AEP event are also summarized for the sub-lakes and wetlands distributed throughout the study area. Recall that both the 1% AEP snowmelt plus rain, and rainfall only scenarios, were applied to determine the governing regulatory flood hazard limit for each reach.

6.1 Napanee River Upper Lakes Water Level Summary

The AEP Lake levels for both the rainfall and spring melt scenarios are presented in Table 6-1. Further to the 1% AEP regulatory flood limits, the 10% AEP and 0.5% events were simulated for the snowmelt plus rain scenario. The 1% AEP plus climate change scenario was also simulated as an individual model run.

Table 6-1 concludes that the Napanee River Upper Lakes water levels are governed by the spring melt condition as described in the *Section 3*. For Reaches 2 and 6, their channel and wetland areas are governed by the rainfall only condition.

There are several prominent beaver dams throughout the study area (see Figure 5-1). A model simulation was included to assess the potential impact of the beaver dams on the lake levels. The height of the beaver dams was based on LiDAR imagery and local survey data where available. A comparison of water levels for the 1% AEP snowmelt plus rain event with and without beaver dams is presented in Table 6-2. The beaver dams provide an increase in storage volume prior to their intended outlet controls becoming utilized. As a result, there is some reduction in water levels in the lower reaches if the beaver dams were to maintain structural stability in a large runoff event. On the other hand, the beaver dams result in an increase the water levels at their respective upstream lake or wetland area.

6.2 Comparison to 1981 Regulatory Water Levels

A comparison of lake level rise (i.e. maximum depth) between the 1981 study and the 2024 results is shown in Table 6-3. The primary differences between the previous and current mapping include 1) decades of 'new' flow gauge data at the Napanee River at Camden East, 2) the addition of the Schroeter model to investigate spring melt events, 3) a more detailed investigation of sub-lakes and wetlands, and 4) improved modeling software and data capabilities relative to historical modeling software.

An important observation was the water level measurement taken by Quinte Conservation on April 15 of 2014 at the Desert Lake Road bridge that connects Howes and Verona Lakes. This measurement occurred during a nearly 1% AEP peak flow at the Napanee Camden East gauge and provided excellent benchmark information to establish the tailwater condition and subsequent lake levels in the lower lakes (i.e. Hambly, Howes, Verona, Van Luven).

6.3 Sub-Lakes and Wetlands Water Level Summary

The sub-lakes and wetlands are the dominant characteristic among the river reaches between the thirteen main lakes of interest. The larger sub-lakes and wetlands were modeled using storage areas. Smaller sub-lakes and wetlands were modeled using 2D flow areas. A water level summary for the most prominent sub-lakes and wetlands is presented in Table 6-4. The names of each sub-lake and wetland in the table below correspond to the labels in the figures shown in Section 5.2 of this report.

Table 6-1: Water Level Summary for 10% AEP, 1% AEP, 0.5% AEP, and Climate Change Scenarios

Lake	Outlet Inv. (m)	10% AEP *Water Level (m)		1% AEP *Water Level (m)		Climate Change *Water Level (m)	
		Rainfall	Rainfall + Snowmelt	Rainfall	Rainfall + Snowmelt	Rainfall (ECCC Method)	Rainfall + Snowmelt (0.5% AEP)
St. Andrews	170.20	170.51	170.73	170.68	170.85	170.84	170.90
Cole	168.90	168.98	169.14	169.11	169.25	169.19	169.27
Van Luven	136.20	136.63	137.11	137.03	137.37	137.43	137.45
Howes	135.50	136.60	137.10	137.03	137.36	137.17	137.44
White	168.25	168.40	168.59	168.51	168.71	168.60	168.75
Potspoon	175.91	176.09	176.19	176.18	176.27	176.26	176.30
30 Island	153.79	153.93	154.13	154.04	154.34	154.15	154.41
13 Island	150.43	150.34	150.70	150.48	150.83	150.62	150.89
Sigsworth	137.57	137.76	138.05	137.90	138.23	138.03	138.28
14 Island	137.57	137.76	138.05	137.90	138.23	138.03	138.28
Hambly	135.48	136.56	137.05	136.98	137.29	137.03	137.36
Verona	135.47	136.55	137.04	136.97	137.27	137.03	137.34
Little John	146.30	146.54	146.70	146.73	146.78	146.90	146.80

*Water levels in datum CGVD 2013

Table 6-2: Water Level Summary for 2% AEP, 5% AEP, and 50% AEP Scenarios

Lake	Outlet Inv. (m)	2% AEP *Water Level (m)		5% AEP *Water Level (m)		50% AEP *Water Level (m)	
		Rainfall	Rainfall + Snowmelt	Rainfall	Rainfall + Snowmelt	Rainfall	Rainfall + Snowmelt
St. Andrews	170.20	170.63	170.81	170.56	170.76	170.38	170.63
Cole	168.90	169.08	169.22	169.04	169.19	168.96	169.08
Van Luven	136.20	136.90	137.33	136.74	137.21	136.34	136.82
Howes	135.50	136.90	137.31	136.73	137.20	136.09	136.81
White	168.25	168.48	168.67	168.44	168.62	168.32	168.49
Potspoon	175.91	176.15	176.25	176.12	176.21	176.04	176.12
30 Island	153.79	154.01	154.28	153.89	154.20	153.86	154.01
13 Island	150.43	150.44	150.77	150.38	150.69	150.26	150.49
Sigsworth	137.57	137.86	138.18	137.80	138.10	137.66	137.89
14 Island	137.57	137.86	138.18	137.80	138.10	137.66	137.89
Hambly	135.48	136.85	137.25	136.69	137.15	136.12	136.77
Verona	135.47	136.85	137.23	136.69	137.13	136.04	136.77
Little John	146.30	146.67	146.76	146.59	146.72	146.39	146.62

*Water levels in datum CGVD 2013

Table 6-3: Comparison of 1% AEP Lake Levels With and Without Beaver Dams

Lake	Without Beaver Dams	With Beaver Dams
St. Andrews	170.85	170.85
Cole	169.25	169.42
Van Luven	137.37	137.38
Howes	137.36	137.35
White	168.71	168.91
Potspoon	176.27	176.27
30 Island	154.34	154.41
13 Island	150.83	150.81
Sigsworth	138.23	138.23
14 Island	138.23	138.23
Hambly	137.29	137.29
Verona	137.27	137.27
Little John	146.78	146.78

Table 6-4: Comparison of Maximum Lake Flood Depths (m) Between 1981 and 2024 Output Results

Lake	Starting Elevation (m)	1% AEP Rainfall			1% AEP Spring Melt		
		1981	2024	Diff.	1981	2024	Diff.
*Howes	135.64	0.76	1.40	0.64	1.31	1.73	0.42
*Van Luven	136.20	0.76	1.01	0.25	1.31	1.35	0.04
*Hambly	135.64	0.76	1.35	0.59	1.31	1.66	0.35
*Verona	135.64	0.76	1.34	0.58	1.31	1.64	0.33
Fourteen Island	137.58	0.31	0.35	0.04	0.63	0.68	0.05
Thirteen Island	150.43	0.31	0.35	0.04	1.27	0.70	(0.57)
Thirty Island	153.80	0.22	1.10	0.88	0.76	1.40	0.64
Potspoon	175.92	0.16	0.22	0.06	0.48	0.31	(0.17)
White	167.99	0.12	0.16	0.04	0.28	0.36	0.08

*Depths measured above 2024 Hardwood Dam outlet for consistency with 1981 comparison.

Table 6-5: Sub-Lakes and Wetlands 1% AEP Water Level Summary

Sub-Lake / Wetland #	Governing 1% AEP Event	Starting Elevation	1% AEP Elevation	Max. Depth
	m	m		
Reach 1: St Andrew Lakes Ln to Howes Lake				
1A	Spring melt	170.38	170.85	0.47
1B	Spring melt	169.78	170.76	0.98
1C	Spring melt	169.48	169.70	0.22
Reach 2: Cole Lake to Howes Lake				
2A	Rainfall	152.20	154.65	2.45
2B	Rainfall	150.80	151.87	1.07
2C	Rainfall	147.20	148.53	1.33
Reach 3: Van Luven Lake to Howes Lake				
N/A				
Reach 4: Howes Lake to Verona Lake				
N/A				
Reach 5: Hambly Lake to Verona Outlet Channel				
N/A				
Reach 6: White Lake to 13 Island Lake				
6A	Rainfall	162.60	163.30	0.70
6B	Rainfall	157.68	158.12	0.44
Reach 7: Potspoon Lake to 30 Island Lake				
7A	Rainfall	174.53	174.93	0.40
7B	Rainfall	173.85	174.46	0.61
Reach 8: 30 Island Lake to 13 Island Lake				
8A	Springmelt	151.53	151.95	0.42
8B	Spring melt	150.90	151.95	0.05
Reach 9: 13 Island Lake to Howes Lake				
9A	Spring melt	146.92	147.56	0.64
9B	Spring melt	145.57	147.55	1.98
Reach 10: 14 Island Lake to Verona Lake				
10A	Spring melt	136.10	137.64	1.54
10B	Spring melt	136.28	137.29	1.01
Reach 11: Verona Lake Outlet Channel to Hardwood Dam				
N/A				

7 Sensitivity Analysis

Flood hazard limits are derived from the runoff rates supplied to the hydraulic model. It is important to assess the sensitivity of the selected peak flows to their input parameters to understand the potential variance in peak flows due to uncertainties in the modeling input. Uncertainties are inherent in all scientific modeling programs, and individual models can be used responsibly when the user understands the limitations and potential factors that can influence the model output.

Figures 7-1 to 7-3 illustrate the parameters with the greatest sensitivity impact on the HEC-HMS flows. These parameters include CN, lag time, and rainfall depth. The CN affects the losses, and unsurprisingly has a significant influence on the peak runoff. The lag time has the least impact of the three, but still has sensitivity impacts on the model results. The lag time has less influence than usual since the reservoir routing governs in the timing of the peak outflows throughout the Napanee River Upper Lakes drainage system. The rainfall volume is of particular interest. It is evident that there is an appreciable increase in peak flows with an increase in the rainfall volume. Since climate change considerations include increasing the rainfall depth by 25%, the increase in the peak runoff rate is significant (see further discussion in Section 4.6).

Given the sensitivity of the HEC-HMS results to the rainfall depths, it is important to consider the data being supplied to the input hyetographs. As discussed in Section 4.2, the Kingston station has a long historical data record (recall that the Kingston station has a greater depth than the Hartington 1%, 1-day AEP rainfall depth and is expected to be a conservative input into the hydrologic model).

With the hydraulic model, there is a sensitivity check for the Manning's roughness values. Since the majority of the study area is comprised of lakes, wetlands, and low velocity areas, the hydraulic model has minimal sensitivity to the Manning's n values. The low, medium, and high Manning's n values used in the sensitivity analysis are summarized in Table 7-1. A model run was completed to compare the water levels with the high and low Manning's values. The water levels showed minimal sensitivity, with several lakes showing no change and the maximum change in elevation being 2cm. Evidently, the Napanee River Upper Lakes and their sub-reaches have minimal sensitivity to the Manning's roughness value considering that the vast majority of the study area is dominated by storage areas.

8 Public Information Center

A public information center (PIC) was held from 5:30pm to 7:00pm on February 6, 2024 at the Sydenham Public Library. The PIC included display boards for each of the draft floodplain maps, and a PowerPoint presentation was available. The PIC was hosted by Quinte Conservation staff along with the project engineers from Jewell Engineering. No major concerns were brought forward by the public and the project proceeded to finalization following the PIC.

Table 8-1: Manning's n Values Applied in Hydraulic Model Sensitivity Tests

Land Cover	Low	Medium	High
Swamp	0.035	0.045	0.06
Clear open water	0.028	0.032	0.035
Community infrastructure	0.035	0.05	0.12
Tree upland	0.05	0.07	0.09
Marsh	0.035	0.045	0.06
Deciduous treed	0.05	0.07	0.09
Mixed treed	0.05	0.07	0.09
Coniferous treed	0.05	0.07	0.09
Agriculture and undifferentiated rural	0.035	0.05	0.07
Plantations - treed cultivated	0.035	0.05	0.07
Hedge rows	0.04	0.05	0.07
Sand gravel mine tailings extraction	0.017	0.025	0.033

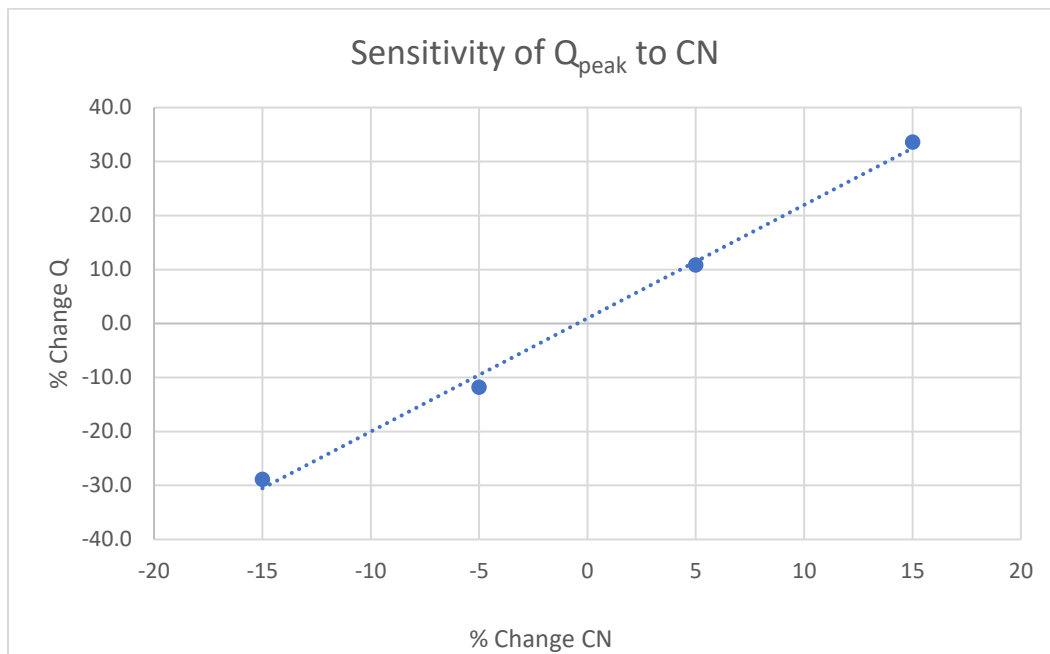


Figure 8-1: Sensitivity of Peak Runoff Rates to CN

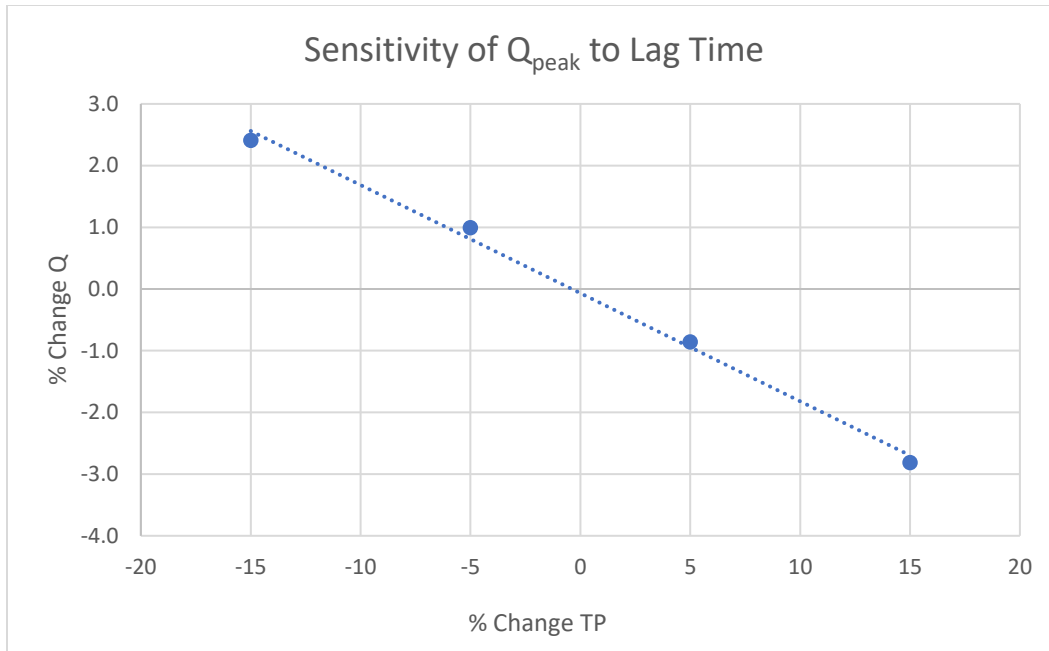


Figure 8-2: Sensitivity of Peak Runoff Rates to Lag Time

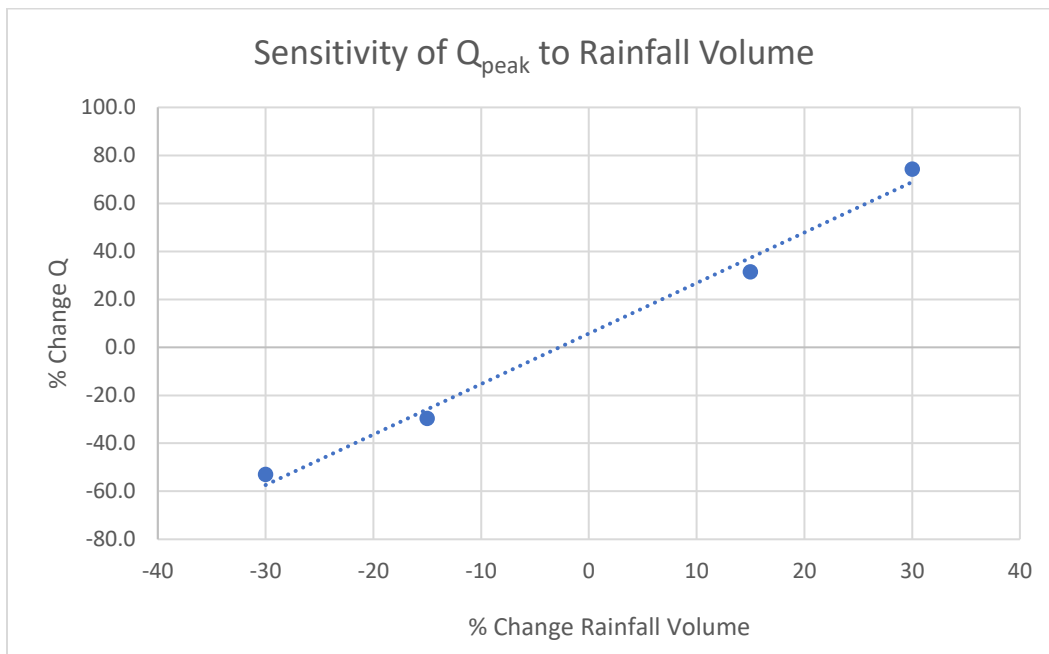


Figure 8-3: Sensitivity of Peak Runoff Rates to Rainfall Volume

Prepared by:



Elliott Fledderus, P.Eng.
Jewell Engineering Inc.



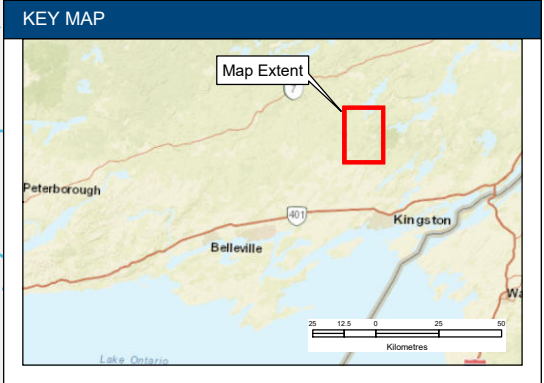
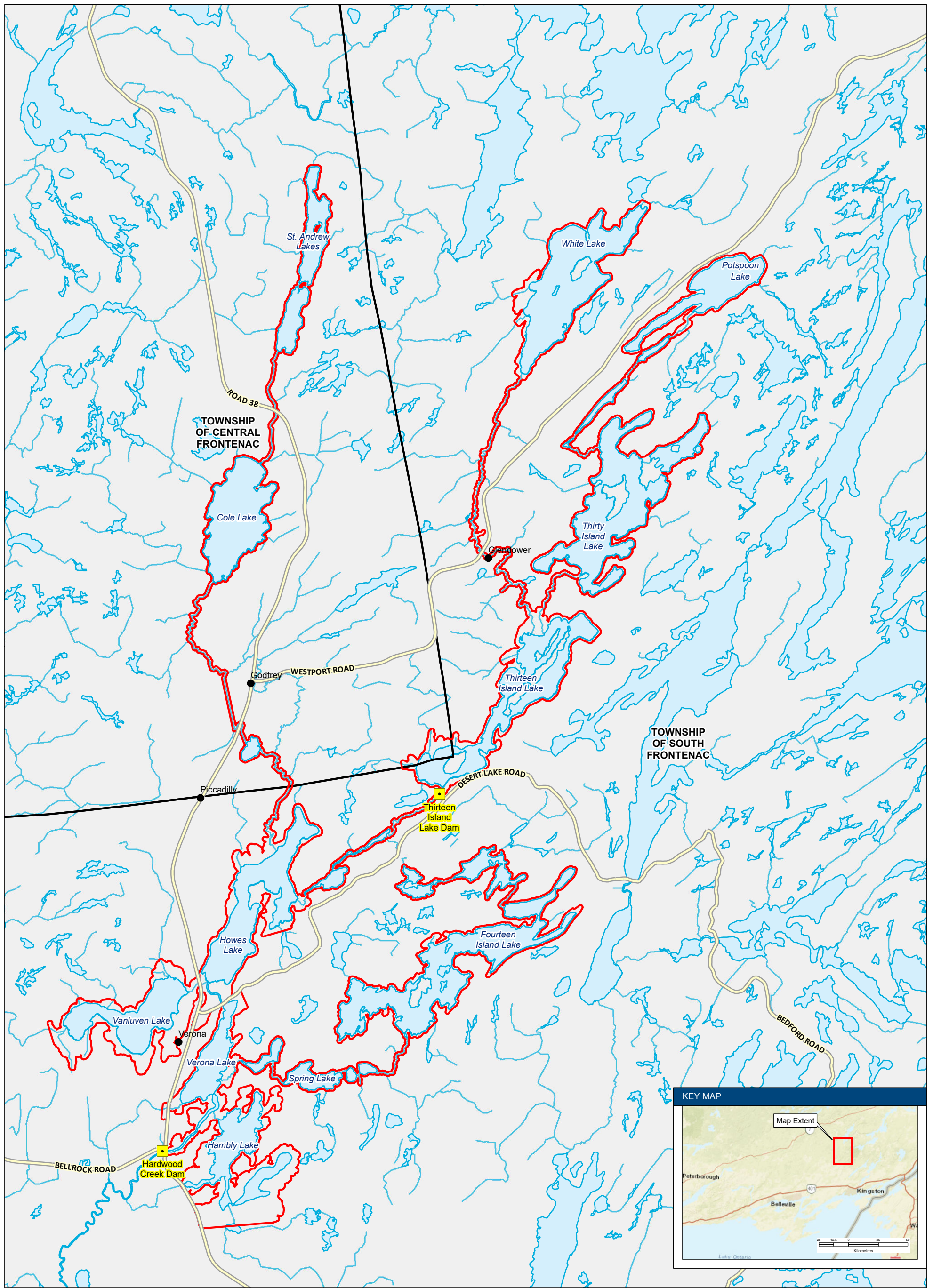
Bryon Keene, P.Eng.
Jewell Engineering Inc.

9 References

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Appendix A-1:
Quinte Conservation – Napanee River Upper Lakes FHIMP Study Area





DEPARTMENT:	Water Resources		
TITLE:	FHIMP Application Area Upper Napanee River		
SCALE:	1:60,000	03	Area extension
SHEET:	1 of 1	02	Area extension
DRAWING:	C.D.	01	Draft for comment / review
CHECK:	C.P.	No.	REVISION

03	Area extension	25/04/23
02	Area extension	23/02/23
01	Draft for comment / review	02/09/22
No.	REVISION	DD/MM/YY

LEGEND / NOTES

- Application Area
- Lower/Single-Tier Municipality
- Population Centre
- Waterbody
- Watercourse
- Water Control Structure

NORTH
 Metres

Appendix B-2: Sub-Catchment Areas and Descriptions



UPPER NAPANEE RIVER MAJOR LAKES



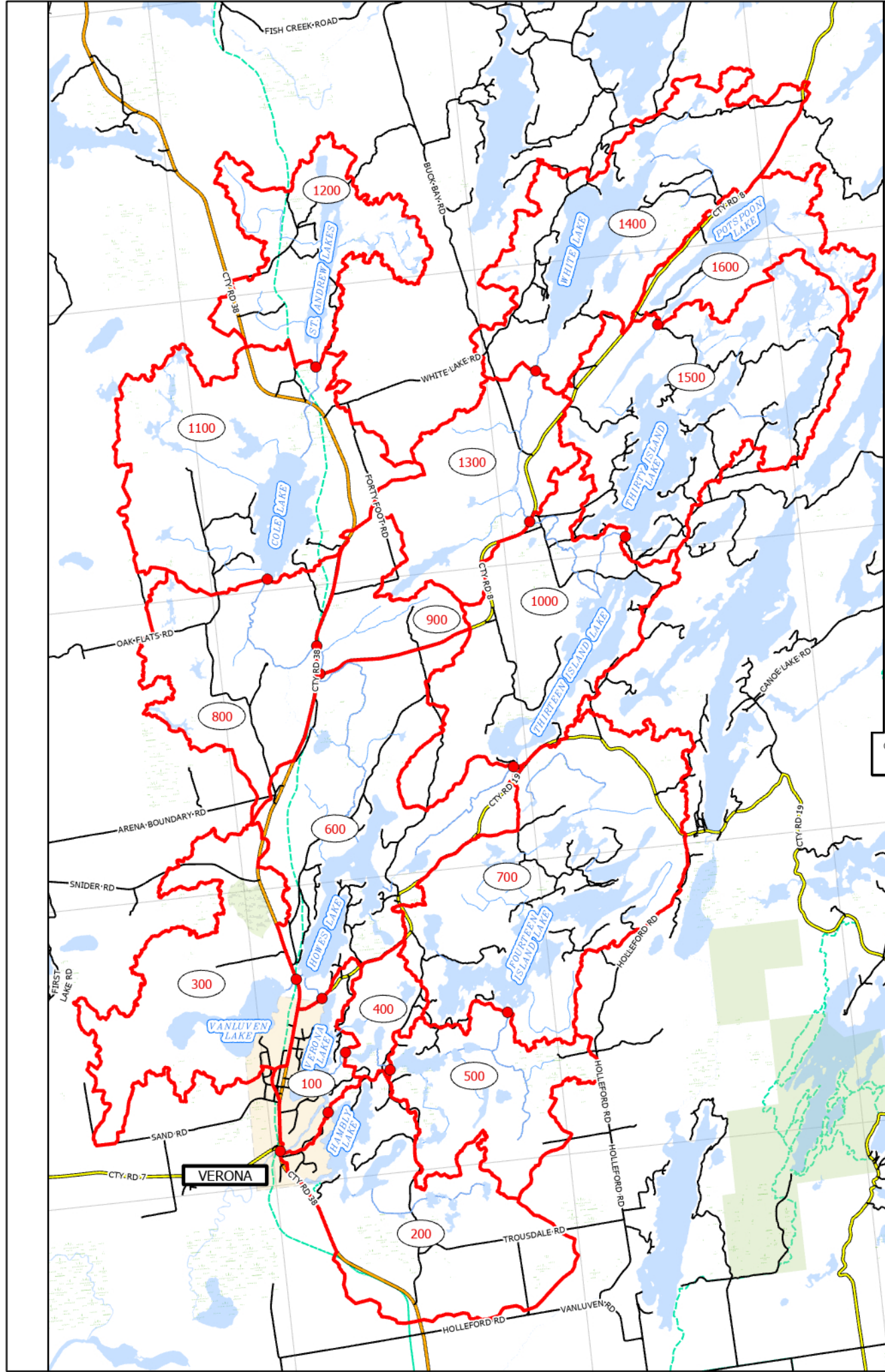
LIDAR PROJECT CONTROL DATUM:
 HORIZONTAL DATUM: NAD 83 CSRS
 (EPOCH 2010.0)
 VERTICAL DATUM: GVD 2013



LEGEND

- HYDROLOGIC NODE
- ONT TRAIL NETWORK
- RAIL ROAD TRACKS
- RIVER NETWORK
 - PRIMARY TRIBUTARY
 - SECONDARY TRIBUTARY
- ROADS
 - MAJOR HIGHWAY
 - MINOR HIGHWAY
 - COUNTY ROAD
 - ROAD
- UTM 5 Km GRID LINE
- WETLANDS
- UPPER NAPANEE RIVER SUB CATCHMENTS

SCALE: 1:120,000



UPPER NAPANEE RIVER ELEVATION MAP

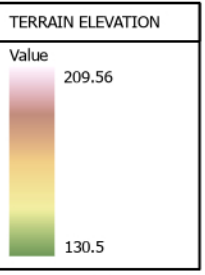


LIDAR PROJECT CONTROL DATUM:
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 VERTICAL DATUM: CGVD 2013

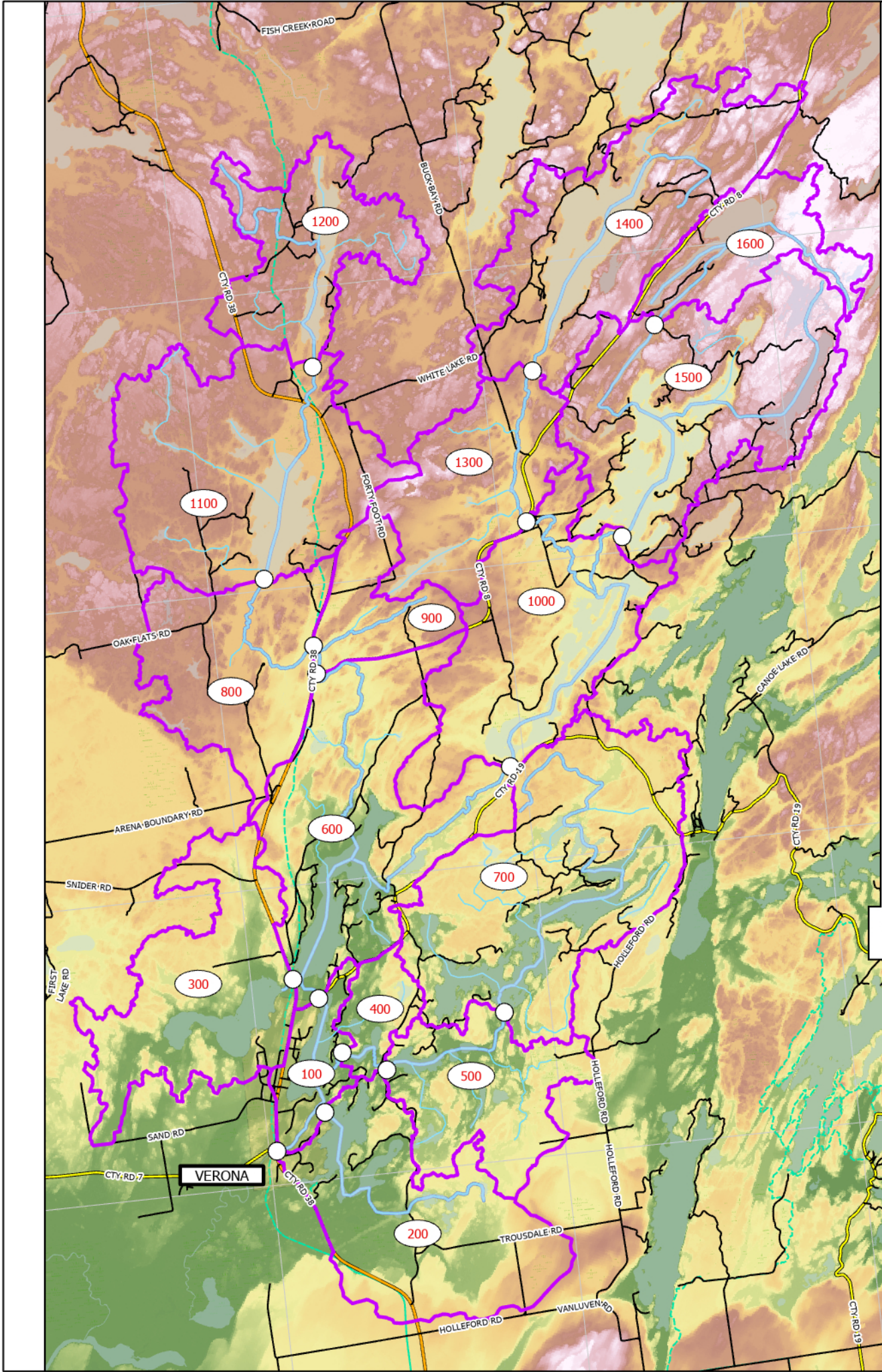


LEGEND

- HYDROLOGIC NODE
- ONT TRAIL NETWORK
- RAIL ROAD TRACKS
- RIVER NETWORK**
 - PRIMARY TRIBUTARY
 - SECONDARY TRIBUTARY
- ROADS**
 - MAJOR HIGHWAY
 - MINOR HIGHWAY
 - COUNTY ROAD
 - ROAD
- UTM 5 Km GRID LINE
- WETLANDS
- UPPER NAPANEE RIVER SUB CATCHMENTS



SCALE: 1:120,000



Appendix B: Soil and Land Cover Maps



UPPER NAPANEE RIVER LAND COVER GROUPS



LIDAR PROJECT CONTROL DATUM:
 HORIZONTAL DATUM: NAD 83 CSRS
 (EPOCH 2010.0)
 VERTICAL DATUM: CGVD 2013



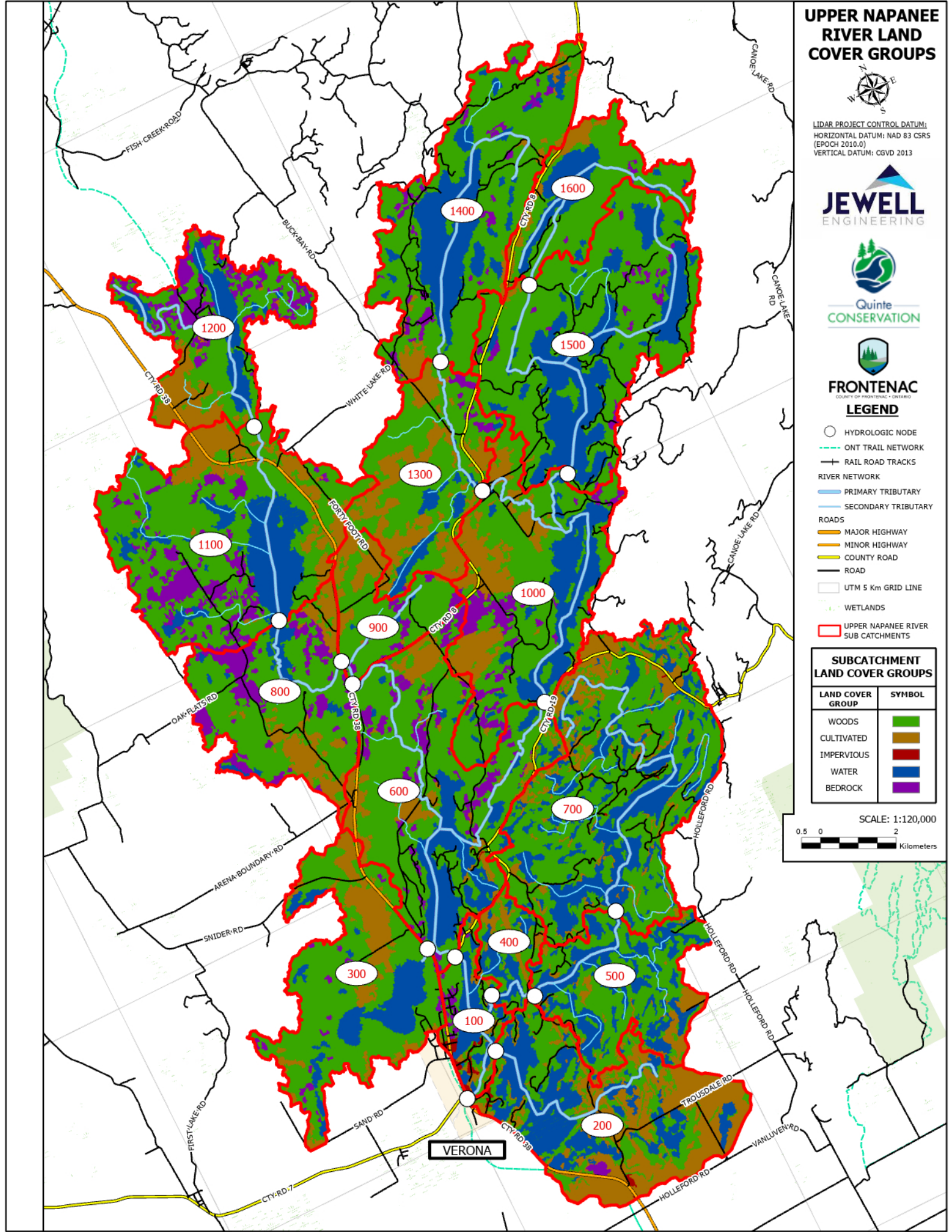
LEGEND

- HYDROLOGIC NODE
- ONT TRAIL NETWORK
- RAIL ROAD TRACKS
- RIVER NETWORK
 - PRIMARY TRIBUTARY
 - SECONDARY TRIBUTARY
- ROADS
 - MAJOR HIGHWAY
 - MINOR HIGHWAY
 - COUNTY ROAD
 - ROAD
- UTM 5 Km GRID LINE
- WETLANDS
- UPPER NAPANEE RIVER SUB CATCHMENTS

SUBCATCHMENT LAND COVER GROUPS

LAND COVER GROUP	SYMBOL
WOODS	
CULTIVATED	
IMPERVIOUS	
WATER	
BEDROCK	

SCALE: 1:120,000



UPPER NAPANEE RIVER SOIL GROUPS



LIDAR PROJECT CONTROL DATUM:
 HORIZONTAL DATUM: NAD 83 CSRS
 (EPOCH 2010.0)
 VERTICAL DATUM: CGVD 2013

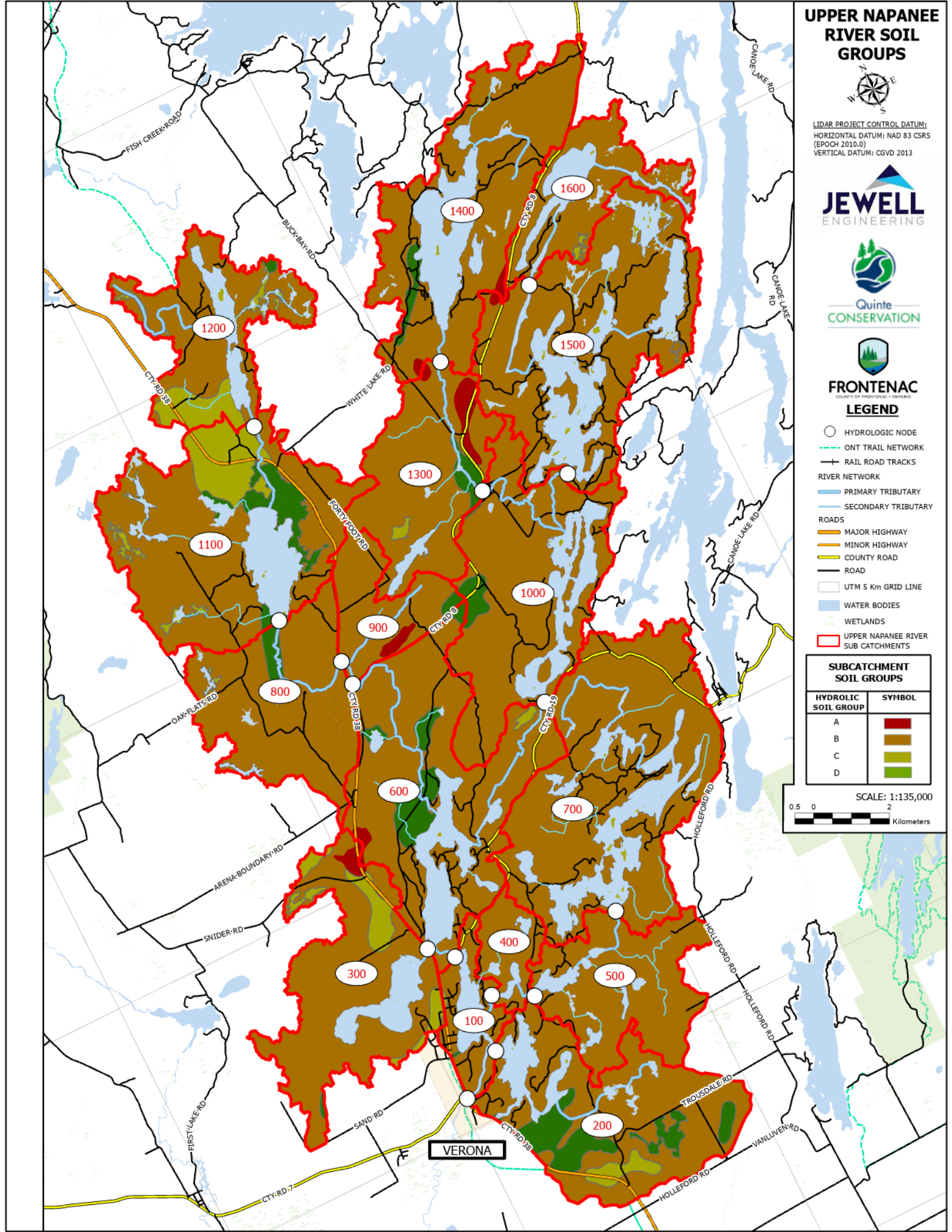


LEGEND

- HYDROLOGIC NODE
- ONT TRAIL NETWORK
- RAIL ROAD TRACKS
- RIVER NETWORK
 - PRIMARY TRIBUTARY
 - SECONDARY TRIBUTARY
- ROADS
 - MAJOR HIGHWAY
 - MINOR HIGHWAY
 - COUNTY ROAD
 - ROAD
- UTM 5 Km GRID LINE
- WATER BODIES
- WETLANDS
- UPPER NAPANEE RIVER SUB CATCHMENTS

SUBCATCHMENT SOIL GROUPS	
HYDROLOGIC SOIL GROUP	SYMBOL
A	
B	
C	
D	

SCALE: 1:135,000



VERONA

Appendix C-1: Hartington Snowmelt + Rainfall MSC Frequency Values



METEOROLOGICAL SERVICE OF CANADA
 RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
 PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 1 - Eastern Canada Forested Basin

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

YR	TOTAL % DAYS VALID	START FLAG MAX	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY	MAX SNPK
1968	198 72	** M/D	2/ 2	2/ 1	2/ 1	1/30	1/29	1/28	1/28	1/28	1/28	3/10	3/ 9	1/14	1/14	1/14	
		.1MM	297	468	468	492	586	591	591	591	591	627	721	871	871	871	
1969	273 100	M/D	11/28	1/23	1/22	1/22	1/22	11/28	11/28	1/23	1/22	1/22	1/18	1/18	1/18	1/18	
		.1MM	310	332	392	392	392	475	539	643	703	732	871	925	925	925	547
1970	242 89	** M/D	12/10	12/10	12/10	12/ 9	3/22	3/21	3/20	3/19	3/19	3/19	3/20	3/18	3/13	3/ 4	
		.1MM	340	416	465	510	556	645	764	814	842	867	1139	1246	1250	1309	
1971	273 100	M/D	4/19	4/12	4/18	4/18	4/17	4/16	4/15	4/12	4/12	4/12	4/ 7	4/ 2	3/29	3/29	
		.1MM	225	422	580	729	828	953	1038	1171	1361	1509	1828	1994	2212	2212	2098
1972	274 100	M/D	2/13	2/13	2/13	4/11	4/10	4/ 9	4/ 9	4/ 9	4/ 9	4/ 9	3/31	3/29	3/21	3/16	
		.1MM	302	381	449	517	572	611	611	611	611	611	887	1032	1337	1556	852
1973	273 100	M/D	12/31	2/ 1	1/17	1/16	1/15	1/17	1/17	1/16	1/15	1/14	1/14	1/15	1/14	12/30	
		.1MM	288	318	424	484	519	583	665	725	759	784	878	1201	1257	1320	696
1974	273 100	M/D	4/ 3	1/26	4/ 1	3/31	3/30	3/29	3/29	3/29	1/20	1/20	1/18	1/18	1/20	1/18	
		.1MM	301	385	541	646	801	847	847	847	864	919	1040	1040	1085	1090	628
1975	273 100	M/D	3/19	3/18	3/17	3/17	3/16	3/16	3/13	3/12	3/12	3/12	3/ 6	2/16	2/23	2/18	
		.1MM	421	562	622	652	681	681	710	752	782	782	827	847	1269	1525	504
1976	274 100	M/D	12/14	3/24	3/24	3/24	3/23	3/20	3/20	3/20	3/19	3/19	3/13	2/15	2/10	2/27	
		.1MM	249	433	568	747	777	809	944	1123	1218	1218	1251	1517	1727	1847	1157
1977	273 100	M/D	3/12	3/11	3/10	3/ 9	3/ 8	3/ 7	3/ 6	3/ 5	3/ 4	3/ 3	2/26	2/24	3/ 4	2/11	
		.1MM	362	542	723	863	887	937	1006	1112	1389	1402	1560	1820	1874	2024	1275
1978	273 100	M/D	1/24	1/24	4/11	4/11	4/10	4/ 9	4/ 7	4/ 6	4/ 6	4/ 4	3/31	3/27	3/20	3/14	
		.1MM	495	495	610	710	734	763	816	941	1041	1177	1614	2053	2354	2512	1664
1979	273 100	M/D	12/31	12/31	12/30	12/30	3/ 1	3/ 3	3/ 3	3/ 1	3/ 1	2/28	2/23	2/21	2/21	2/21	
		.1MM	282	535	575	575	646	719	816	874	971	993	1237	1381	1381	1381	733
1980	274 100	M/D	3/21	3/20	3/19	3/18	3/17	3/17	3/17	3/17	3/17	3/17	3/10	3/ 5	3/ 5	2/21	
		.1MM	349	529	546	600	678	678	678	678	678	678	718	726	726	797	356
1981	273 100	M/D	2/19	2/18	2/17	2/16	2/15	2/15	2/15	2/15	2/11	2/10	2/ 8	2/ 1	1/26	1/26	
		.1MM	409	590	712	860	863	863	863	863	988	1230	1239	1309	1474	1474	565
1982	273 100	M/D	3/25	3/12	3/11	3/11	3/11	3/11	3/19	3/18	3/11	3/11	3/11	3/11	3/11	3/11	
		.1MM	233	313	402	460	491	508	595	621	691	772	1197	1197	1197	1197	810
1983	273 100	M/D	12/15	12/14	12/14	4/20	4/20	4/20	4/20	4/20	4/20	12/15	12/14	11/28	11/28	11/28	
		.1MM	191	236	236	251	251	251	251	251	251	347	392	470	470	626	168
1984	274 100	M/D	2/14	2/13	2/12	2/11	2/11	2/11	2/11	2/11	2/11	2/11	12/ 5	2/ 2	1/24	11/15	
		.1MM	487	689	760	893	944	944	944	944	944	944	979	1197	1197	1319	565
1985	273 100	M/D	2/23	2/22	2/22	2/21	2/21	2/21	2/21	2/22	2/21	2/21	2/22	2/22	2/21	2/13	

1986	273	100	.1MM	410	674	796	886	939	979	979	998	1088	1168	1168	1803	1933	1991	865
			M/D	1/19	1/19	1/18	1/17	1/17	1/17	3/13	3/13	3/11	3/10	3/11	3/10	3/ 2	3/ 2	
			.1MM	290	453	573	637	663	708	752	752	819	913	1075	1168	1251	1251	798
1987	273	100	M/D	3/ 1	2/28	2/28	2/28	2/28	2/28	2/28	3/ 1	2/28	2/28	2/23	2/28	2/23	2/ 7	
			.1MM	257	377	422	422	422	448	483	638	758	758	789	800	831	842	465
1988	274	100	M/D	11/28	11/28	11/28	11/26	11/26	11/26	11/26	11/26	11/26	11/26	11/26	3/ 7	11/28	11/26	
			.1MM	380	705	705	740	740	740	740	740	740	740	783	1059	1088	1123	634
1989	273	100	M/D	3/27	3/26	3/25	3/24	3/24	3/24	3/24	3/24	3/24	3/24	3/14	3/14	3/ 4	3/ 4	
			.1MM	212	325	414	528	579	579	579	579	579	579	839	839	899	899	510
1990	273	100	M/D	2/22	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/ 8	1/ 3	12/31	
			.1MM	331	444	444	444	444	444	520	640	779	824	914	1054	1212	1362	540
1991	273	100	M/D	12/29	12/29	12/29	2/ 3	2/ 3	3/ 1	3/ 1	3/ 1	3/ 1	3/ 1	2/20	2/18	2/18	2/ 3	
			.1MM	390	390	390	394	394	498	498	498	498	498	712	854	864	1048	374
1992	274	100	M/D	3/26	3/26	3/25	3/25	3/ 6	3/26	3/25	3/25	3/25	3/25	3/25	3/ 7	3/ 6	3/ 5	
			.1MM	373	486	579	582	663	736	829	910	925	925	925	1010	1389	1610	413
1993	273	100	M/D	3/28	3/28	3/27	3/26	3/25	3/24	3/24	3/24	3/22	3/21	3/21	3/21	3/16	3/ 7	
			.1MM	195	375	501	582	672	762	821	821	866	915	1117	1158	1195	1368	891
1994	272	100	M/D	2/20	2/19	3/22	3/21	3/21	3/21	3/21	3/21	3/21	3/21	3/19	3/13	3/ 7	3/ 5	
			.1MM	209	326	423	538	578	618	654	735	806	864	992	1159	1258	1283	906
1995	273	100	M/D	1/14	1/13	1/12	1/12	1/12	1/12	1/12	3/ 6	3/ 5	3/ 5	3/ 5	1/12	2/18	12/16	
			.1MM	267	418	486	486	486	486	501	541	541	541	546	670	806		246
1996	274	100	M/D	2/20	2/20	1/16	2/20	2/20	2/19	2/19	2/19	1/16	1/16	1/14	1/14	1/16	1/14	
			.1MM	349	463	529	690	763	781	781	781	856	856	1088	1088	1306	1387	591
1997	273	100	M/D	3/25	2/20	2/19	3/25	3/25	3/25	3/25	3/25	3/25	3/25	3/20	3/14	3/10	3/ 1	
			.1MM	278	403	570	637	830	915	950	1008	1008	1008	1021	1044	1092	1232	646
1998	273	100	M/D	1/ 5	1/ 5	2/17	2/17	2/17	2/17	2/17	2/17	2/12	2/11	2/10	2/ 2	2/ 1	1/24	
			.1MM	206	242	282	344	383	383	383	420	465	507	564	577	587		335
1999	273	100	M/D	1/23	1/22	1/22	1/17	1/19	1/18	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	
			.1MM	217	293	337	372	417	584	696	741	741	741	741	1021	1092	1092	585
2000	274	100	M/D	2/24	2/23	2/23	2/24	2/23	2/22	2/22	2/22	2/22	2/22	2/22	2/22	2/22	2/16	
			.1MM	298	425	503	659	786	821	821	821	821	821	849	880	1060	1068	594
2001	273	100	M/D	2/ 9	12/16	4/ 4	4/ 3	4/ 3	4/ 2	4/ 1	3/31	3/30	3/29	3/23	3/19	3/14	3/10	
			.1MM	425	503	517	625	719	805	844	884	935	979	1068	1478	1656	1744	1338
2002	273	100	M/D	3/ 2	2/20	2/19	3/27	2/20	2/16	2/15	2/15	2/15	2/15	2/16	2/15	2/10	2/10	
			.1MM	246	301	321	326	327	406	496	496	496	522	763	853	924	994	325
2003	273	100	M/D	3/20	3/20	3/20	3/20	3/20	3/20	3/17	3/17	3/17	3/16	3/12	3/ 8	3/17	3/12	
			.1MM	272	469	613	748	869	1025	1062	1184	1340	1429	1486	1508	1563	1709	1169
2004	274	100	M/D	3/ 5	3/ 4	3/ 3	3/ 2	3/ 1	3/ 1	2/29	2/28	2/28	2/28	2/28	2/21	2/19	2/19	
			.1MM	277	405	513	653	802	883	955	1004	1016	1033	1189	1215	1297	1317	841
2005	273	100	M/D	12/23	2/14	2/14	2/12	2/12	2/12	2/12	2/ 8	2/ 7	2/ 6	2/ 4	2/ 4	12/22	12/16	
			.1MM	207	333	333	359	359	359	359	472	616	633	637	637	801	837	301
2006	273	100	M/D	11/29	11/28	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	
			.1MM	482	717	780	780	780	780	780	780	780	780	780	780	780	1070	290
2007	273	100	M/D	3/13	3/13	3/12	3/11	3/10	3/10	3/10	3/10	3/10	3/10	3/ 1	3/ 3	3/ 1	2/22	
			.1MM	144	273	353	443	470	470	470	470	470	470	592	612	668	676	486
2008	274	100	M/D	12/23	12/22	1/ 6	1/ 5	3/31	12/23	12/23	12/22	12/22	3/26	12/23	12/22	3/12	3/ 7	
			.1MM	435	488	510	556	613	667	730	783	783	818	906	1353	1400	1445	1017
2009	273	100	M/D	2/11	2/11	2/10	2/10	12/24	12/24	12/24	12/24	12/24	12/24	12/14	12/14	2/11	2/10	
			.1MM	326	497	519	519	637	705	705	705	705	705	941	1009	1203	1413	681
2010	273	100	M/D	12/26	12/25	12/25	12/25	12/25	12/25	12/25	12/25	12/25	12/25	12/13	12/13	12/25	12/25	
			.1MM	159	276	330	338	338	338	351	404	404	404	456	530	623	623	219
2011	226	83	**	M/D	3/10	3/10	3/10	3/10	3/ 5	3/ 5	3/10	3/10	3/10	3/ 5	3/ 5	2/27	2/18	
			.1MM	421	544	602	665	696	744	867	1034	1224	1264	1587	1751	1923	1934	
2012	274	100	M/D	1/17	1/23	1/23	1/23	1/23	1/23	1/23	1/17	1/23	1/23	1/17	1/13	1/12	1/ 6	

2013	273	100	.1MM	151	172	180	290	293	297	305	323	401	451	553	619	649	704	130
			M/D	3/11	3/10	3/ 9	3/ 8	3/ 7	3/ 6	3/ 6	3/ 6	1/ 6	1/ 6	2/25	1/11	1/ 6	1/ 6	
			.1MM	347	478	531	575	629	673	673	673	682	682	908	989	1122	1122	480
2014	273	100	M/D	1/ 5	2/20	2/20	2/19	2/19	2/19	3/29	3/28	3/28	3/28	3/21	3/19	3/11	3/ 8	
			.1MM	321	504	590	639	647	647	703	869	889	889	953	1167	1379	1493	981
2015	273	100	M/D	11/23	4/ 2	4/ 2	4/ 1	3/30	3/30	3/30	3/10	3/ 9	3/25	3/21	3/16	3/10	3/ 9	
			.1MM	215	385	447	464	518	580	580	606	650	670	804	975	1347	1454	1109
2016	114	42	**	M/D	12/31	12/31	12/31	12/29	12/29	12/29	12/29	12/29	12/29	12/29	12/20	12/20	12/20	12/20
			.1MM	57	84	100	118	134	134	134	134	134	134	141	141	141	141	

		1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY
MEAN EXTREME	(MM)	30.2	43.0	50.3	57.0	61.8	66.1	69.6	74.1	79.3	82.7	95.2	107.7	119.4	127.6
STD. DEV.	(MM)	9.2	12.6	14.2	16.6	18.7	19.7	20.5	22.2	25.4	27.2	30.5	37.4	41.3	43.0
YEARS ANALYSED		45	45	45	45	45	45	45	45	45	45	45	45	45	45
MAX EXTREME	(MM)	49.5	71.7	79.6	89.3	94.4	102.5	106.2	118.4	138.9	150.9	182.8	205.3	235.4	251.2
YEAR		1978	2006	1985	1984	1984	2003	2003	2003	1977	1971	1971	1978	1978	1978

** NOTE ** VALUE IN FLAG INDICATES YEAR NOT INCLUDED IN ANALYSIS BASED ON % DAYS OPERATIONAL (<90.0%)

METEOROLOGICAL SERVICE OF CANADA
 RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
 PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 1 - Eastern Canada Forested Basin

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6 NOTE : MODIFIED GUMBEL 12/82

RETURN PERIOD VALUES (MM) WITH 50% CONFIDENCE LIMITS

RETURN PERIOD YEARS	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY
2	28.66+/- 0.84	40.96+/- 1.16	47.97+/- 1.31	54.25+/- 1.53	58.76+/- 1.72
5	36.75+/- 1.42	52.09+/- 1.96	60.47+/- 2.20	68.88+/- 2.57	75.26+/- 2.90
10	42.11+/- 1.92	59.46+/- 2.64	68.75+/- 2.97	78.56+/- 3.47	86.19+/- 3.92
25	48.88+/- 2.59	68.78+/- 3.56	79.22+/- 4.00	90.80+/- 4.68	99.99+/- 5.28
50	53.91+/- 3.10	75.69+/- 4.26	86.98+/- 4.79	99.88+/- 5.60	110.23+/- 6.32
100	58.90+/- 3.61	82.55+/- 4.97	94.68+/- 5.58	108.89+/- 6.53	120.39+/- 7.36

RETURN PERIOD YEARS	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY
2	62.84+/- 1.82	66.20+/- 1.89	70.47+/- 2.05	75.09+/- 2.34	78.23+/- 2.51
5	80.29+/- 3.07	84.36+/- 3.19	90.12+/- 3.45	97.50+/- 3.94	102.28+/- 4.23

10	91.84+/- 4.14	96.38+/- 4.31	103.13+/- 4.66	112.33+/- 5.32	118.20+/- 5.71
25	106.43+/- 5.58	111.57+/- 5.81	119.56+/- 6.29	131.08+/- 7.17	138.32+/- 7.70
50	117.26+/- 6.68	122.84+/- 6.95	131.76+/- 7.52	144.99+/- 8.58	153.25+/- 9.21
100	128.00+/- 7.78	134.03+/- 8.10	143.86+/- 8.77	158.79+/-10.00	168.07+/-10.73

RETURN PERIOD
YEARS

	15 DAY	20 DAY	25 DAY	30 DAY
2	90.17+/- 2.82	101.59+/- 3.45	112.62+/- 3.81	120.51+/- 3.96
5	117.15+/- 4.74	134.65+/- 5.81	149.15+/- 6.42	158.49+/- 6.67
10	135.01+/- 6.40	156.53+/- 7.85	173.33+/- 8.67	183.63+/- 9.01
25	157.58+/- 8.63	184.18+/-10.58	203.89+/-11.69	215.40+/-12.15
50	174.32+/-10.33	204.70+/-12.66	226.55+/-13.99	238.97+/-14.54
100	190.94+/-12.04	225.06+/-14.75	249.06+/-16.30	262.36+/-16.94

METEOROLOGICAL SERVICE OF CANADA
RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 2 - Western North America Mountain Basin

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

YR	TOTAL % DAYS VALID	START FLAG MAX	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY	MAX SNPK
1968	198 72	** M/D	2/ 2	2/ 1	2/ 1	3/16	3/15	3/15	3/15	3/15	3/15	3/10	3/ 9	3/ 9	3/ 9	3/ 9	
		.1MM	315	477	477	561	604	604	604	604	604	701	807	862	862	862	
1969	273 100	M/D	3/24	3/23	3/22	3/21	3/20	3/19	3/18	3/17	3/17	3/15	3/18	3/17	3/15	3/15	
		.1MM	441	462	560	649	835	866	934	967	967	988	1115	1274	1295	1295	692
1970	242 89	** M/D	12/10	4/ 8	4/ 7	4/ 6	4/ 6	3/21	3/20	4/ 2	4/ 1	4/ 1	3/26	3/21	3/19	3/19	
		.1MM	340	449	555	597	597	607	691	817	838	838	997	1446	1553	1553	
1971	273 100	M/D	4/19	4/19	4/18	4/17	4/16	4/15	4/15	4/12	4/12	4/11	4/ 9	4/ 2	3/29	3/29	
		.1MM	351	631	869	984	1148	1235	1315	1551	1831	1957	2221	2342	2466	2466	2309
1972	274 100	M/D	2/13	4/14	4/13	4/12	4/11	4/11	4/11	4/10	4/ 9	4/ 9	4/ 3	3/29	3/22	3/21	
		.1MM	284	426	613	781	934	1022	1157	1190	1193	1193	1265	1454	1486	1737	1100
1973	273 100	M/D	12/31	1/18	1/17	3/ 4	3/ 3	3/ 2	1/17	1/16	1/16	1/17	1/16	1/16	1/16	12/30	
		.1MM	259	393	530	596	690	693	761	803	803	803	854	1172	1175	1213	863
1974	273 100	M/D	4/ 3	4/ 2	4/ 1	3/31	3/30	3/29	3/29	3/29	3/29	3/29	2/19	2/13	2/13	2/13	
		.1MM	397	470	646	738	888	934	934	934	934	934	980	993	1003	1003	692
1975	273 100	M/D	3/19	3/18	3/18	3/17	3/18	3/18	3/18	3/18	3/17	3/17	3/17	3/12	3/12	3/18	
		.1MM	470	620	718	759	861	920	1027	1116	1157	1157	1243	1323	1386	1760	733
1976	274 100	M/D	3/20	3/24	3/24	3/24	3/24	3/20	3/20	3/19	3/19	3/19	3/12	2/15	3/ 2	2/26	
		.1MM	341	640	817	817	817	1068	1245	1331	1331	1331	1399	1435	1753	2195	1243
1977	273 100	M/D	3/28	3/11	3/10	3/ 9	3/ 9	3/ 7	3/ 6	3/ 5	3/ 4	3/ 3	2/26	2/24	3/ 4	2/27	
		.1MM	358	597	863	1055	1055	1078	1138	1264	1540	1553	1672	1918	1992	2086	1317
1978	273 100	M/D	1/24	4/11	4/11	4/11	4/11	4/11	4/11	4/11	4/11	4/ 6	4/ 4	3/31	3/27	3/21	3/20
		.1MM	509	533	808	924	1030	1120	1120	1120	1178	1288	1788	2247	2419	2694	1795

1979	273	100	M/D	1/ 1	12/31	12/30	3/ 3	3/ 3	3/ 3	3/ 3	3/ 3	3/ 3	3/ 1	3/ 1	2/23	2/22	2/21	2/21		
			.1MM	278	548	588	608	671	760	923	969	1015	1062	1219	1382	1471	1536		803	
1980	274	100	M/D	3/21	3/20	3/20	3/18	3/17	3/17	3/17	3/17	3/17	3/17	3/10	3/10	3/10	2/21			
			.1MM	452	700	700	729	788	788	788	788	788	788	792	792	792	805		466	
1981	273	100	M/D	2/19	2/18	2/17	2/16	2/16	2/16	2/16	2/16	2/11	2/10	2/10	2/ 1	1/26	1/26			
			.1MM	362	607	764	937	937	937	937	937	1054	1284	1284	1354	1538	1538		632	
1982	273	100	M/D	3/25	3/30	3/29	3/11	3/11	3/25	3/25	3/24	3/24	3/22	3/17	3/12	3/11	3/11			
			.1MM	268	453	466	479	479	518	769	832	832	862	1124	1507	1604	1604		1050	
1983	273	100	M/D	1/10	1/10	1/10	1/10	1/10	1/10	1/10	1/10	1/10	1/10	12/15	1/10	12/23	12/23	12/14		
			.1MM	280	367	367	367	367	367	367	367	367	399	422	536	536	776		243	
1984	274	100	M/D	2/14	2/13	2/13	2/11	2/11	2/11	2/11	2/11	2/11	2/11	2/ 2	2/ 2	1/24	11/15			
			.1MM	580	820	909	1029	1118	1182	1182	1182	1182	1182	1340	1340	1450	1491		735	
1985	273	100	M/D	2/23	2/22	2/22	2/21	2/21	2/21	2/21	2/22	2/21	2/21	2/22	2/22	2/21	2/13			
			.1MM	437	727	872	962	992	996	1031	1121	1201	1209	1874	1964	2003			897	
1986	273	100	M/D	1/19	1/19	1/18	1/17	1/17	1/17	3/13	3/19	3/18	3/17	3/13	3/10	3/10	3/10			
			.1MM	342	507	651	683	683	696	733	813	961	974	1278	1402	1402	1402		1023	
1987	273	100	M/D	3/ 1	3/ 7	3/ 7	3/ 7	3/ 7	3/ 7	3/ 7	3/ 1	3/ 1	2/28	2/28	2/28	2/28	2/28	2/28		
			.1MM	239	372	372	372	372	372	400	624	744	744	744	943	998	998		642	
1988	274	100	M/D	3/25	11/28	11/28	3/23	3/23	3/23	3/23	3/23	3/23	3/23	3/23	11/28	3/ 7	11/28	11/28		
			.1MM	467	723	764	841	841	841	841	841	841	841	841	922	1156	1259	1288	658	
1989	273	100	M/D	3/27	3/26	3/25	3/24	3/24	3/24	3/24	3/24	3/24	3/24	3/14	3/14	3/ 4	3/ 4			
			.1MM	315	455	552	658	729	729	729	729	756	756	1006	1033	1066	1093		667	
1990	273	100	M/D	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/ 9	1/17	1/17			
			.1MM	342	507	507	507	507	507	559	680	836	849	895	967	1177	1321		540	
1991	273	100	M/D	12/29	12/29	2/ 3	2/ 3	2/ 3	3/ 1	3/ 1	3/ 1	3/ 1	3/ 1	2/20	2/18	3/ 1	2/ 3			
			.1MM	390	390	470	470	470	569	569	569	569	569	731	854	917	1124		471	
1992	274	100	M/D	3/26	3/25	3/ 8	3/ 7	3/ 6	3/ 6	3/ 6	3/25	3/25	3/25	3/25	3/ 7	3/ 7	3/ 6			
			.1MM	403	483	602	747	831	831	831	891	946	967	1008	1231	1559	1829		644	
1993	273	100	M/D	1/ 4	1/ 3	3/27	3/26	3/25	3/24	3/24	3/22	3/21	3/24	3/24	3/21	3/16	3/ 8			
			.1MM	513	627	699	780	877	974	974	987	1008	1014	1234	1268	1288	1461		984	
1994	272	100	M/D	2/20	3/22	3/22	3/21	3/21	3/21	3/22	3/21	3/21	3/22	3/21	3/14	3/15	3/14			
			.1MM	264	414	579	659	663	667	678	758	821	868	1219	1340	1422	1451		959	
1995	273	100	M/D	3/ 7	1/13	1/12	1/12	1/12	1/12	3/ 7	3/ 6	3/ 5	3/ 5	3/ 5	3/ 5	2/19	12/16			
			.1MM	284	418	486	486	486	486	575	648	688	688	688	688	785	806		392	
1996	274	100	M/D	11/11	1/18	1/16	1/16	2/20	2/20	2/19	2/20	2/20	2/19	1/16	2/ 8	2/19	1/27			
			.1MM	382	476	609	720	823	946	964	986	1024	1042	1093	1205	1321	1397		804	
1997	273	100	M/D	3/25	3/28	3/27	3/27	3/25	3/25	3/25	3/25	3/25	3/25	3/25	3/14	3/14	3/ 1			
			.1MM	278	474	656	744	989	1077	1077	1124	1157	1157	1157	1180	1180	1292		732	
1998	273	100	M/D	1/ 7	3/ 8	1/ 5	1/ 5	1/ 3	1/ 2	1/ 2	1/ 2	1/ 2	1/ 2	2/24	2/18	2/17	2/25			
			.1MM	254	363	466	486	541	587	607	617	617	617	722	856	1018	1110		395	
1999	273	100	M/D	1/23	3/17	2/10	2/10	3/17	1/18	1/17	1/17	1/17	1/17	2/ 2	1/17	1/18	1/17			
			.1MM	245	291	341	341	383	476	573	586	586	586	588	827	963	1169		585	
2000	274	100	M/D	2/24	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23			
			.1MM	339	497	562	712	838	838	838	838	838	838	883	883	1118	1154		610	
2001	273	100	M/D	2/ 9	12/16	4/ 4	4/ 4	4/ 4	4/ 3	4/ 2	4/ 1	3/31	3/30	3/29	3/21	3/15	3/12			
			.1MM	424	551	690	867	1034	1165	1254	1258	1262	1282	1295	1688	1785	1880		1493	
2002	273	100	M/D	3/ 2	2/20	2/19	3/27	1/23	1/22	2/15	2/15	2/15	3/27	2/19	2/15	2/10	1/23			
			.1MM	216	335	355	383	438	451	542	542	542	548	709	896	968	1140		398	
2003	273	100	M/D	3/17	3/20	3/20	3/20	3/20	3/17	3/17	3/17	3/16	3/16	3/16	3/16	3/17	3/16			
			.1MM	300	488	683	864	1021	1114	1296	1452	1550	1621	1621	1621	1748	1846		1303	
2004	274	100	M/D	12/10	12/10	3/ 3	3/ 2	3/ 1	3/ 1	2/29	2/28	2/28	2/28	2/29	2/28	3/ 1	2/28			
			.1MM	492	508	621	811	970	1050	1114	1135	1135	1135	1249	1316	1386	1471		930	
2005	273	100	M/D	1/13	2/14	2/14	12/30	12/30	12/30	12/30	12/30	2/ 7	2/ 7	12/30	12/22	12/22	12/22			
			.1MM	199	350	350	425	489	489	489	489	637	637	766	778	1044	1044		360	

2006	273	100	M/D	11/29	11/28	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	1/ 4	1/ 4	12/25	12/23	
			.1MM	426	730	780	780	780	780	780	780	780	780	780	933	933	1096	1223	290
2007	273	100	M/D	3/13	3/13	3/12	3/11	3/10	3/10	3/10	3/10	3/10	3/10	3/10	3/10	3/ 3	3/ 1	3/ 1	
			.1MM	198	385	465	562	572	572	572	572	572	572	572	648	687	707	707	535
2008	274	100	M/D	12/23	12/22	1/ 6	1/ 5	4/ 1	3/31	3/31	3/31	3/28	3/27	3/26	3/18	3/15	3/ 8		
			.1MM	500	530	646	666	781	907	908	908	953	1000	1082	1416	1583	1593		1200
2009	273	100	M/D	2/11	2/11	3/ 6	3/ 5	12/24	12/24	12/24	12/24	12/24	2/26	12/14	12/ 9	2/11	2/11		
			.1MM	381	574	577	583	677	725	725	725	725	743	981	1035	1318	1492		704
2010	273	100	M/D	1/24	1/24	1/24	1/24	1/24	1/24	1/19	1/19	1/19	1/16	1/14	1/14	1/ 1	12/27		
			.1MM	442	850	871	871	871	871	880	901	901	947	1045	1045	1053	1083		457
2011	226	83	**	M/D	3/10	3/10	3/16	3/16	3/16	3/ 5	3/ 5	3/10	3/10	3/10	3/ 5	2/27	2/27	2/18	
			.1MM	420	563	657	661	661	740	883	1022	1305	1309	1629	1778	1917	2032		
2012	274	100	M/D	11/24	1/31	1/31	1/23	1/23	1/23	1/26	1/26	1/24	1/23	1/23	1/17	1/17	1/17		
			.1MM	152	161	161	236	236	236	271	271	310	398	461	543	606	650		187
2013	273	100	M/D	3/11	3/10	3/10	3/ 9	3/ 8	3/ 7	3/ 6	3/ 6	3/ 6	3/ 6	2/27	1/11	1/ 6	2/25		
			.1MM	499	673	809	838	851	881	893	893	893	893	993	1091	1144	1159		677
2014	273	100	M/D	4/ 7	2/20	2/20	4/ 4	4/ 3	4/ 2	4/ 1	3/31	3/30	3/29	3/28	3/19	3/15	3/10		
			.1MM	358	500	589	727	740	854	934	1048	1086	1158	1334	1536	1633	1827		1038
2015	273	100	M/D	4/ 3	4/ 2	4/ 2	4/ 2	3/30	3/30	3/30	3/30	3/30	3/26	3/21	3/16	3/10	3/ 9		
			.1MM	291	533	588	600	613	668	681	685	731	748	866	989	1367	1498		1140
2016	114	42	**	M/D	1/ 9	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	12/31	12/29	12/29	12/20	12/20		
			.1MM	205	219	219	219	219	219	219	219	219	258	292	292	312	312		

		1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY
MEAN EXTREME	(MM)	35.6	51.5	61.3	68.5	74.9	79.7	84.2	88.3	92.8	95.5	106.8	120.8	131.6	141.6
STD. DEV.	(MM)	10.0	14.0	16.8	19.6	22.5	24.7	25.7	27.5	30.2	31.8	34.8	39.9	41.6	44.6
YEARS ANALYSED		45	45	45	45	45	45	45	45	45	45	45	45	45	45
MAX EXTREME	(MM)	58.0	85.0	90.9	105.5	114.8	123.5	131.5	155.1	183.1	195.7	222.1	234.2	246.6	269.4
YEAR		1984	2010	1984	1977	1971	1971	1971	1971	1971	1971	1971	1971	1971	1978

** NOTE ** VALUE IN FLAG INDICATES YEAR NOT INCLUDED IN ANALYSIS BASED ON % DAYS OPERATIONAL (<90.0%)

METEOROLOGICAL SERVICE OF CANADA
 RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
 PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 2 - Western North America Mountain Basin

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6 NOTE : MODIFIED GUMBEL 12/82

RETURN PERIOD VALUES (MM) WITH 50% CONFIDENCE LIMITS

RETURN PERIOD YEARS	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY
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2	33.92+/- 0.92	49.16+/- 1.29	58.55+/- 1.55	65.29+/- 1.81	71.23+/- 2.07
5	42.76+/- 1.55	61.56+/- 2.18	73.44+/- 2.62	82.65+/- 3.05	91.10+/- 3.49
10	48.62+/- 2.10	69.78+/- 2.94	83.29+/- 3.53	94.15+/- 4.12	104.25+/- 4.72
25	56.01+/- 2.83	80.15+/- 3.97	95.75+/- 4.77	108.67+/- 5.56	120.87+/- 6.36
50	61.50+/- 3.39	87.85+/- 4.75	104.99+/- 5.70	119.45+/- 6.65	133.20+/- 7.61
100	66.95+/- 3.95	95.49+/- 5.53	114.16+/- 6.64	130.14+/- 7.75	145.44+/- 8.86

RETURN PERIOD
YEARS

	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY
2	75.60+/- 2.28	79.95+/- 2.37	83.77+/- 2.54	87.85+/- 2.78	90.24+/- 2.93
5	97.40+/- 3.83	102.68+/- 3.99	108.10+/- 4.28	114.51+/- 4.69	118.30+/- 4.93
10	111.84+/- 5.18	117.72+/- 5.39	124.21+/- 5.78	132.16+/- 6.33	136.89+/- 6.66
25	130.08+/- 6.98	136.73+/- 7.27	144.56+/- 7.79	154.47+/- 8.53	160.37+/- 8.98
50	143.62+/- 8.35	150.83+/- 8.70	159.66+/- 9.32	171.02+/-10.21	177.78+/-10.75
100	157.05+/- 9.73	164.82+/-10.14	174.65+/-10.85	187.44+/-11.90	195.07+/-12.52

RETURN PERIOD
YEARS

	15 DAY	20 DAY	25 DAY	30 DAY
2	101.13+/- 3.21	114.22+/- 3.68	124.82+/- 3.83	134.25+/- 4.11
5	131.85+/- 5.40	149.51+/- 6.20	161.54+/- 6.45	173.65+/- 6.92
10	152.18+/- 7.29	172.88+/- 8.38	185.85+/- 8.72	199.74+/- 9.35
25	177.88+/- 9.83	202.41+/-11.30	216.57+/-11.75	232.70+/-12.61
50	196.94+/-11.76	224.31+/-13.52	239.36+/-14.06	257.15+/-15.09
100	215.86+/-13.70	246.05+/-15.75	261.98+/-16.38	281.42+/-17.58

** WARNING ** : 100 YEAR VALUES IN 1971 BASED ON 10 DAYS ACCUMULATION

** WARNING ** : 100 YEAR VALUES IN 1971 BASED ON 15 DAYS ACCUMULATION

METEOROLOGICAL SERVICE OF CANADA
RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 3 - Western Canada Mountain Basin

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

YR	TOTAL % DAYS VALID	START FLAG MAX	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY	MAX SNPK	
1968	198	72 **	M/D	2/ 2	2/ 1	3/18	3/16	3/16	3/15	3/15	3/15	3/15	3/15	3/ 9	3/ 9	3/ 9	3/ 9	
			.1MM	296	439	477	559	710	753	753	753	753	753	915	974	974	974	
1969	273	100	M/D	3/24	3/24	3/22	3/21	3/20	3/20	3/19	3/18	3/17	3/17	3/22	3/17	3/15	3/15	
			.1MM	463	533	545	618	779	849	866	915	933	933	1110	1428	1436	1436	713
1970	242	89 **	M/D	12/10	4/ 8	4/ 7	4/ 6	4/ 6	4/ 8	4/ 7	4/ 6	4/ 6	4/ 6	4/ 1	3/21	3/20	3/19	
			.1MM	340	485	568	593	593	764	846	872	872	872	1078	1290	1653	1662	
1971	273	100	M/D	4/19	4/19	4/19	4/18	4/18	4/17	4/16	4/15	4/12	4/12	4/ 9	4/ 3	3/29	3/29	

1972	274	100	.1MM	387	646	872	1072	1221	1313	1446	1512	1739	1966	2357	2381	2522	2522	2357
			M/D	2/13	12/ 9	4/13	4/12	4/14	4/13	4/13	4/12	4/11	4/11	4/10	4/ 1	3/29	3/22	
			.1MM	284	397	579	719	856	1040	1185	1325	1450	1541	1559	1647	1732	1964	1345
1973	273	100	M/D	12/31	1/18	1/17	3/ 4	3/ 3	3/ 2	1/17	1/16	1/16	1/17	1/16	1/16	12/30	12/26	
			.1MM	259	433	568	679	760	763	790	815	815	815	851	1169	1174	1217	913
1974	273	100	M/D	4/ 3	4/ 3	3/ 4	4/ 1	3/30	3/30	3/29	3/29	3/29	3/29	2/21	2/19	2/13	3/ 6	
			.1MM	387	489	644	693	816	917	963	963	963	963	1115	1153	1162	1163	754
1975	273	100	M/D	3/19	4/17	4/16	4/15	4/14	4/13	4/13	4/13	3/17	3/17	3/12	3/17	3/12	3/19	
			.1MM	466	599	791	941	1032	1041	1041	1041	1049	1049	1112	1170	1233	1751	883
1976	274	100	M/D	3/25	3/24	3/24	3/24	3/24	3/20	3/20	3/20	3/19	3/19	3/13	3/12	3/ 3	2/27	
			.1MM	470	759	909	1142	1142	1176	1326	1558	1625	1625	1645	1693	1995	2205	1322
1977	273	100	M/D	3/12	3/11	3/10	3/ 9	3/ 9	3/ 9	3/ 7	3/ 6	3/ 4	3/ 4	2/27	2/24	3/ 4	2/27	
			.1MM	431	656	979	1138	1258	1258	1267	1309	1554	1675	1749	2016	2132	2206	1332
1978	273	100	M/D	1/24	4/11	4/11	4/11	4/11	4/11	4/11	4/11	4/11	4/11	4/ 5	3/31	3/27	3/21	
			.1MM	477	481	735	826	909	1059	1192	1384	1664	1664	1955	2385	2687	2930	1910
1979	273	100	M/D	12/31	12/31	12/30	12/30	3/ 3	3/ 3	3/ 3	3/ 3	3/ 1	3/ 1	2/23	3/ 3	3/ 1	2/21	
			.1MM	250	497	537	537	565	633	773	803	853	883	1026	1313	1393	1641	952
1980	274	100	M/D	3/21	3/20	3/20	3/18	3/17	3/17	3/17	3/17	3/17	3/17	3/17	3/17	3/17	3/17	
			.1MM	515	767	767	782	824	824	824	824	824	824	824	824	824	824	502
1981	273	100	M/D	2/18	2/18	2/17	2/16	2/16	2/16	2/16	2/16	2/11	2/10	2/10	2/ 1	1/26	1/26	
			.1MM	330	651	828	985	985	985	985	985	1083	1313	1313	1383	1538	1538	665
1982	273	100	M/D	3/31	3/30	3/30	3/31	3/30	3/30	3/30	3/24	3/24	3/25	3/20	3/12	3/11	3/11	
			.1MM	518	691	728	818	991	991	991	1007	1044	1262	1397	1528	1903	1933	1156
1983	273	100	M/D	2/ 2	2/ 2	2/ 2	1/31	1/30	1/30	1/30	1/30	1/30	1/25	1/23	1/30	1/10	1/10	
			.1MM	380	538	538	556	566	566	566	566	566	581	623	636	1007	1007	260
1984	274	100	M/D	2/14	2/13	2/13	2/11	2/13	2/11	2/11	2/11	2/11	2/11	2/11	2/ 3	2/ 2	1/24	1/24
			.1MM	730	938	1005	1124	1209	1267	1396	1396	1396	1396	1467	1527	1637	1637	858
1985	273	100	M/D	2/23	2/22	2/22	2/21	2/21	2/21	2/21	2/22	2/21	2/21	2/21	2/22	2/21	2/13	
			.1MM	412	701	820	910	925	925	925	935	1025	1093	1093	1679	1844	1866	912
1986	273	100	M/D	3/26	3/25	3/25	3/23	3/23	3/23	3/23	3/19	3/18	3/18	3/13	3/10	3/10	3/10	
			.1MM	523	703	703	792	792	792	792	1023	1153	1153	1403	1495	1495	1495	1116
1987	273	100	M/D	11/26	4/ 3	11/24	11/23	11/23	11/23	11/23	3/ 1	2/28	2/28	2/28	2/28	2/28	3/ 7	
			.1MM	231	344	389	420	420	420	420	536	656	656	656	799	1042	1044	694
1988	274	100	M/D	3/25	3/25	3/24	3/23	3/23	3/23	3/23	3/23	3/23	3/23	3/12	3/ 7	11/28	11/28	
			.1MM	410	723	843	913	913	913	913	913	913	913	944	1175	1217	1373	658
1989	273	100	M/D	3/28	3/27	3/26	3/25	3/24	3/24	3/24	3/24	3/24	3/24	3/14	3/14	3/ 4	3/ 4	
			.1MM	435	704	816	891	978	978	978	978	1018	1018	1208	1248	1268	1308	909
1990	273	100	M/D	1/17	1/17	1/17	3/10	3/10	3/10	3/10	1/17	1/17	1/17	2/ 9	2/22	1/17	1/17	
			.1MM	310	457	457	530	530	530	530	594	724	724	772	916	997	1191	556
1991	273	100	M/D	12/29	3/ 1	2/ 3	2/ 3	2/ 3	3/ 1	3/ 1	3/ 1	3/ 1	3/ 1	12/17	2/18	3/ 1	2/ 3	
			.1MM	390	427	495	495	495	627	627	627	627	627	768	854	975	1149	495
1992	274	100	M/D	3/26	3/25	3/25	3/ 7	3/ 6	3/ 6	3/25	3/25	3/25	3/25	3/25	3/25	3/ 7	3/ 6	
			.1MM	413	493	571	656	740	740	758	818	856	863	1196	1196	1415	1619	726
1993	273	100	M/D	1/ 4	1/ 3	3/28	3/27	3/26	3/25	3/24	3/24	3/24	3/24	3/24	3/21	3/16	3/ 8	
			.1MM	549	663	754	889	949	1024	1099	1099	1099	1139	1359	1366	1386	1542	1083
1994	272	100	M/D	2/20	2/19	3/22	4/ 2	4/ 1	3/31	3/30	4/ 2	3/28	3/31	3/22	3/21	3/21	3/15	
			.1MM	281	401	480	570	592	660	682	749	787	839	1277	1537	1537	1624	1065
1995	273	100	M/D	3/ 7	1/13	1/12	1/12	1/12	1/12	3/ 7	3/ 7	3/ 6	3/ 5	3/ 5	3/ 5	2/19	2/15	
			.1MM	250	417	485	485	485	485	490	629	685	725	725	725	800	810	429
1996	274	100	M/D	11/11	1/18	1/16	1/16	2/20	2/20	2/19	2/20	2/20	2/19	1/16	2/ 8	1/16	2/20	
			.1MM	395	435	570	660	730	827	845	867	890	908	976	1056	1159	1346	887
1997	273	100	M/D	3/25	3/28	3/27	3/27	3/25	3/25	3/25	3/25	3/25	3/25	3/25	3/25	3/14	3/14	
			.1MM	278	439	589	657	905	972	972	1002	1137	1249	1249	1249	1272	1272	823
1998	273	100	M/D	1/ 7	3/ 8	1/ 5	1/ 5	1/ 3	1/ 2	1/ 2	1/ 2	1/ 2	2/28	2/25	2/18	2/17	2/25	

1999	273	100	.1MM	254	380	466	486	573	604	624	634	634	678	843	931	1093	1202	395
			M/D	1/23	2/28	2/28	2/28	2/28	1/18	1/17	1/17	1/17	1/17	3/17	1/17	1/18	1/17	
			.1MM	215	280	280	320	320	400	480	480	480	480	600	682	786	954	585
2000	274	100	M/D	2/24	2/23	2/23	2/24	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	
			.1MM	336	500	547	673	838	838	838	838	838	838	883	883	1093	1154	610
2001	273	100	M/D	4/ 8	4/ 7	4/ 6	4/ 5	4/ 4	4/ 4	4/ 3	4/ 2	4/ 2	4/ 2	3/30	3/21	3/19	3/12	
			.1MM	460	607	891	1064	1229	1365	1470	1538	1538	1538	1558	1873	1888	2003	1606
2002	273	100	M/D	3/ 2	2/20	1/26	1/26	1/26	1/23	1/23	1/23	1/23	1/23	2/19	2/15	2/10	1/23	
			.1MM	216	356	464	464	464	567	567	567	595	655	753	896	968	1246	514
2003	273	100	M/D	3/17	3/20	3/20	3/20	3/20	3/20	3/17	3/17	3/17	3/16	3/16	3/16	3/17	3/16	
			.1MM	288	477	677	827	954	1161	1231	1359	1565	1640	1640	1640	1768	1865	1322
2004	274	100	M/D	12/10	12/10	3/ 3	3/ 2	3/ 1	3/ 1	2/29	2/28	2/28	2/28	2/29	2/28	3/ 1	2/28	
			.1MM	480	521	545	757	892	952	997	1005	1005	1005	1100	1137	1418	1471	930
2005	273	100	M/D	1/13	2/14	2/14	12/30	12/30	12/30	12/30	3/22	2/ 7	2/ 7	12/30	12/30	12/22	12/22	
			.1MM	211	335	350	399	444	444	444	465	581	596	721	721	979	979	496
2006	273	100	M/D	11/29	11/28	11/27	11/27	11/27	11/27	11/27	11/27	11/27	1/12	1/11	1/ 4	1/ 4	12/23	
			.1MM	468	730	780	780	780	780	780	780	824	899	945	1067	1158	1289	318
2007	273	100	M/D	3/14	3/13	3/12	3/11	3/10	3/10	3/10	3/10	3/10	3/10	3/10	3/ 3	3/ 1	3/ 1	
			.1MM	264	443	503	578	588	588	588	588	588	588	664	687	707	707	535
2008	274	100	M/D	12/23	1/ 7	1/ 6	1/ 5	4/ 1	4/ 1	3/31	3/31	3/31	3/31	3/26	3/19	3/15	3/15	
			.1MM	487	610	726	746	784	964	1067	1125	1125	1125	1245	1493	1663	1663	1281
2009	273	100	M/D	2/11	2/11	3/ 6	3/ 5	3/ 6	3/ 6	3/ 5	3/ 5	3/ 5	2/27	2/26	2/26	2/11	2/11	
			.1MM	408	594	628	634	708	773	779	779	779	790	987	987	1304	1582	779
2010	273	100	M/D	1/24	1/24	1/24	1/24	1/24	1/24	1/19	1/19	1/19	1/16	1/15	1/15	1/ 1	12/27	
			.1MM	430	816	823	823	823	823	831	838	838	871	923	923	931	946	525
2011	226	83	**	M/D	3/18	3/17	3/16	3/16	3/16	3/13	3/12	3/11	3/10	3/10	3/ 5	2/27	2/27	2/18
			.1MM	487	692	805	805	805	835	857	979	1374	1374	1665	1801	1844	2141	
2012	274	100	M/D	11/24	3/ 7	3/ 7	3/ 7	3/ 7	3/ 3	3/ 2	3/ 2	3/ 2	3/ 2	2/23	2/22	2/15	2/15	
			.1MM	152	246	246	246	246	325	377	377	377	377	444	519	646	646	273
2013	273	100	M/D	3/11	3/11	3/10	3/10	3/ 9	3/ 9	3/ 7	3/ 7	3/ 7	3/ 7	2/27	1/11	1/ 9	2/25	
			.1MM	566	744	900	915	930	930	945	945	945	945	1015	1102	1134	1157	710
2014	273	100	M/D	4/ 7	4/ 7	4/ 7	4/ 6	4/ 4	4/ 4	4/ 2	4/ 2	3/31	3/31	3/28	3/21	3/19	3/11	
			.1MM	408	676	760	813	984	1069	1074	1159	1224	1309	1538	1553	1708	1940	1102
2015	273	100	M/D	4/ 3	4/ 2	4/ 2	4/ 2	3/30	4/ 2	4/ 2	4/ 3	4/ 2	4/ 2	3/30	3/25	3/17	3/14	
			.1MM	314	520	558	558	580	640	685	892	1098	1098	1158	1280	1355	1532	1140
2016	114	42	**	M/D	1/10	1/ 9	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	12/29	12/29	12/20	12/20	
			.1MM	293	489	503	503	503	503	503	503	503	503	503	560	560	580	580

		1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY
MEAN EXTREME	(MM)	38.2	55.1	64.8	72.4	78.9	84.0	87.6	92.3	98.1	101.8	112.8	124.3	136.4	146.6
STD. DEV.	(MM)	11.8	15.5	18.4	21.9	24.6	25.6	28.0	30.0	33.8	36.3	39.1	43.1	44.9	47.5
YEARS ANALYSED		45	45	45	45	45	45	45	45	45	45	45	45	45	45
MAX EXTREME	(MM)	73.0	93.8	100.5	114.2	125.8	136.5	147.0	155.8	183.1	196.6	235.7	238.5	268.7	293.0
YEAR		1984	1984	1984	1976	1977	2001	2001	1976	1971	1971	1971	1978	1978	1978

** NOTE ** VALUE IN FLAG INDICATES YEAR NOT INCLUDED IN ANALYSIS BASED ON % DAYS OPERATIONAL (<90.0%)

METEOROLOGICAL SERVICE OF CANADA
 RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
 PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 3 - Western Canada Mountain Basin

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

RETURN PERIOD VALUES (MM) WITH 50% CONFIDENCE LIMITS

RETURN PERIOD
YEARS

	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY
2	36.25+/- 1.09	52.61+/- 1.43	61.74+/- 1.69	68.80+/- 2.02	74.85+/- 2.27
5	46.65+/- 1.83	66.29+/- 2.41	77.97+/- 2.85	88.18+/- 3.41	96.63+/- 3.83
10	53.54+/- 2.47	75.35+/- 3.25	88.71+/- 3.85	101.01+/- 4.60	111.05+/- 5.17
25	62.24+/- 3.33	86.80+/- 4.38	102.28+/- 5.19	117.22+/- 6.20	129.27+/- 6.97
50	68.70+/- 3.98	95.29+/- 5.24	112.35+/- 6.21	129.24+/- 7.42	142.79+/- 8.34
100	75.11+/- 4.64	103.72+/- 6.11	122.34+/- 7.24	141.18+/- 8.64	156.21+/- 9.72

RETURN PERIOD
YEARS

	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY
2	79.78+/- 2.36	82.99+/- 2.59	87.38+/- 2.77	92.59+/- 3.11	95.88+/- 3.35
5	102.44+/- 3.98	107.78+/- 4.36	113.92+/- 4.66	122.42+/- 5.24	127.97+/- 5.64
10	117.44+/- 5.38	124.19+/- 5.88	131.50+/- 6.30	142.17+/- 7.08	149.21+/- 7.62
25	136.40+/- 7.25	144.92+/- 7.93	153.70+/- 8.50	167.13+/- 9.55	176.06+/-10.27
50	150.46+/- 8.68	160.30+/- 9.49	170.17+/-10.17	185.64+/-11.42	195.97+/-12.29
100	164.42+/-10.11	175.57+/-11.06	186.52+/-11.84	204.02+/-13.31	215.74+/-14.32

RETURN PERIOD
YEARS

	15 DAY	20 DAY	25 DAY	30 DAY
2	106.35+/- 3.60	117.19+/- 3.98	129.03+/- 4.14	138.75+/- 4.38
5	140.87+/- 6.07	155.29+/- 6.70	168.70+/- 6.97	180.71+/- 7.37
10	163.73+/- 8.19	180.52+/- 9.04	194.97+/- 9.42	208.49+/- 9.96
25	192.61+/-11.05	212.39+/-12.19	228.15+/-12.70	243.58+/-13.43
50	214.04+/-13.22	236.04+/-14.59	252.77+/-15.19	269.62+/-16.07
100	235.30+/-15.40	259.51+/-17.00	277.21+/-17.70	295.46+/-18.72

** WARNING ** : 100 YEAR VALUES IN 1971 BASED ON 15 DAYS ACCUMULATION
 METEOROLOGICAL SERVICE OF CANADA
 RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
 PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 4 - Southern Ontario

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

YR	TOTAL %		START FLAG	MAX	START															MAX SNPK
	DAYS	VALID			1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY		
1968	199	73	**	M/D	2/ 2	2/ 1	1/31	3/19	1/29	3/16	3/16	3/15	3/15	3/15	3/ 8	3/ 8	3/ 1	2/29		
				.1MM	243	385	401	442	540	582	674	762	762	762	929	949	969	989		
1969	273	100		M/D	3/24	3/23	3/22	3/22	3/20	3/20	3/19	3/17	3/17	3/19	3/18	3/13	3/13	3/ 3		
				.1MM	359	415	471	516	663	708	742	788	833	857	1055	1182	1185	1208	715	
1970	242	89	**	M/D	12/10	12/10	12/ 8	12/ 8	12/ 8	12/ 7	3/20	3/19	3/18	3/18	3/19	3/20	3/17	3/12		
				.1MM	360	398	424	462	472	477	561	643	688	709	933	1200	1347	1368		
1971	273	100		M/D	4/12	4/19	4/18	4/18	4/18	4/18	4/17	4/18	4/17	4/18	4/12	4/ 8	4/ 3	3/29		
				.1MM	160	293	425	504	590	707	823	899	1007	1083	1564	1917	2070	2269	2104	
1972	274	100		M/D	2/13	2/13	2/13	4/15	4/14	4/13	4/12	4/11	4/10	4/ 9	4/ 4	3/30	3/28	3/21		
				.1MM	294	334	372	482	573	682	799	875	926	972	1013	1218	1342	1614	966	
1973	273	100		M/D	12/31	2/ 1	2/ 1	2/ 1	3/ 3	12/30	3/ 1	12/ 5	12/ 5	12/ 5	1/19	1/16	1/15	12/ 5		
				.1MM	330	399	409	424	463	486	537	624	654	654	840	1018	1052	1257	596	
1974	273	100		M/D	4/ 3	3/ 3	4/ 1	3/31	3/30	3/29	3/29	3/29	3/29	3/26	2/21	2/19	2/12	3/ 3		
				.1MM	319	376	497	564	719	765	799	799	799	815	967	1033	1096	1104	741	
1975	273	100		M/D	3/19	3/18	3/18	3/19	3/18	3/17	3/18	3/18	3/17	3/16	3/16	3/15	3/12	3/18		
				.1MM	357	490	545	609	741	792	884	940	991	1031	1168	1312	1416	1749	797	
1976	274	100		M/D	3/27	1/25	3/25	3/24	3/24	3/23	3/21	3/20	3/19	3/19	3/18	2/15	2/10	2/28		
				.1MM	254	380	478	657	733	794	825	1008	1094	1170	1211	1408	1643	1793	1116	
1977	273	100		M/D	3/12	3/12	3/11	3/10	3/ 9	3/ 9	3/ 9	3/ 9	3/ 8	3/ 4	3/ 3	2/24	2/24	2/24		
				.1MM	309	467	604	711	818	900	994	1055	1095	1200	1480	1804	1890	2013	1190	
1978	273	100		M/D	1/24	1/24	4/18	4/17	4/16	4/16	4/15	4/13	4/11	4/11	4/ 6	4/ 1	3/27	3/23		
				.1MM	451	461	559	656	763	843	919	1027	1129	1349	1595	1910	2348	2474	1685	
1979	273	100		M/D	1/25	12/31	12/30	12/30	12/30	12/30	3/ 3	3/ 3	3/ 1	3/ 1	2/28	3/ 1	2/21	2/21		
				.1MM	249	447	496	506	506	506	581	613	702	734	966	1186	1328	1600	866	
1980	274	100		M/D	3/21	3/20	3/20	3/18	3/17	3/17	3/17	3/17	3/17	3/17	3/ 9	3/ 4	3/ 4	2/24		
				.1MM	357	475	512	562	646	683	698	698	698	698	767	790	790	817	377	
1981	273	100		M/D	2/19	2/19	2/18	2/19	2/18	2/17	2/16	2/15	2/15	2/15	2/ 8	2/ 8	2/ 1	1/25		
				.1MM	325	562	664	760	862	916	1011	1034	1034	1034	1454	1454	1561	1721	493	
1982	273	100		M/D	1/ 4	3/30	3/29	3/29	12/31	3/25	3/25	3/24	3/24	3/23	3/18	3/12	3/10	3/ 4		
				.1MM	215	336	386	428	436	454	639	703	744	767	964	1290	1391	1410	867	
1983	273	100		M/D	1/10	1/10	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	12/15	12/11	12/23	12/23	12/14	
				.1MM	303	303	312	312	312	312	312	312	312	312	357	399	481	481	707	219
1984	274	100		M/D	2/14	2/13	2/13	2/11	2/13	2/13	2/13	2/13	2/11	2/10	2/10	2/ 5	2/ 2	1/27		
				.1MM	394	556	602	653	788	837	920	952	1017	1054	1319	1328	1512	1521	744	
1985	273	100		M/D	2/23	2/22	2/21	2/21	2/21	2/21	2/21	2/21	2/21	2/21	2/21	2/21	2/21	2/21		
				.1MM	357	558	685	763	809	841	850	886	960	988	1024	1517	1650	1782	857	
1986	273	100		M/D	1/19	1/19	3/25	3/25	3/25	3/23	3/23	3/22	3/18	3/18	3/15	3/10	3/ 4	3/ 2		
				.1MM	224	342	421	531	575	636	680	717	793	884	1176	1496	1498	1592	1093	
1987	273	100		M/D	11/26	2/28	2/27	11/23	11/23	11/23	2/28	2/28	2/28	2/27	2/22	2/26	2/21	2/21		
				.1MM	229	347	374	405	420	420	438	521	630	658	722	817	881	881	516	
1988	274	100		M/D	11/28	11/28	11/28	11/28	11/28	11/26	11/28	11/28	11/28	11/28	11/26	3/ 6	11/28	11/26		
				.1MM	380	691	746	755	755	764	764	764	774	774	844	897	1119	1271	533	
1989	273	100		M/D	1/ 7	3/27	3/26	3/25	3/24	3/23	3/23	3/23	3/23	3/20	3/23	3/14	3/ 9	3/ 4	3/ 4	
				.1MM	174	277	350	414	501	538	538	538	547	569	810	856	943	974	480	
1990	273	100		M/D	2/22	1/17	2/21	2/21	2/19	2/18	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17		
				.1MM	305	366	387	387	401	410	452	538	651	678	838	895	1163	1290	531	
1991	273	100		M/D	12/29	12/29	12/29	12/29	12/29	3/ 1	2/28	2/28	2/28	2/28	2/20	2/18	3/ 1	2/ 5		

1992	274	100	.1MM	412	412	412	412	412	538	557	557	557	557	701	854	886	1128	388
			M/D	3/26	3/25	3/25	3/24	3/25	3/25	3/25	3/25	3/24	3/24	3/19	3/ 8	3/ 6	3/ 3	
			.1MM	306	441	537	556	615	688	761	806	825	825	880	1016	1353	1586	389
1993	273	100	M/D	1/ 4	1/ 3	1/ 3	1/ 3	12/31	12/30	12/29	3/24	3/24	3/22	3/23	3/21	3/16	3/ 8	
			.1MM	482	637	637	637	657	687	764	784	824	866	1195	1291	1329	1411	928
1994	272	100	M/D	4/ 2	4/ 2	4/ 2	4/ 2	4/ 1	3/31	3/30	3/29	3/28	3/27	3/22	3/20	3/15	3/10	
			.1MM	221	267	344	498	534	594	639	685	722	750	1116	1397	1509	1564	981
1995	273	100	M/D	1/14	1/13	1/12	1/12	1/12	1/12	1/12	1/12	3/ 5	3/ 4	3/ 1	2/16	2/17	12/16	
			.1MM	245	333	466	466	466	466	466	499	517	536	584	705	763	264	
1996	274	100	M/D	2/20	2/20	1/16	1/16	2/20	2/20	2/20	2/20	2/20	2/19	1/14	2/ 7	2/19	2/19	
			.1MM	336	407	464	576	665	734	766	806	838	865	987	1014	1186	1337	694
1997	273	100	M/D	3/25	2/20	2/19	2/18	3/25	3/24	3/25	3/25	3/24	2/18	2/18	2/15	2/18	2/18	
			.1MM	300	414	518	587	657	671	693	716	729	789	973	1018	1078	1133	508
1998	273	100	M/D	1/ 7	1/ 7	1/ 5	1/ 5	1/ 5	1/ 5	1/ 2	1/ 5	1/ 5	1/ 5	12/24	12/24	12/24	12/15	
			.1MM	254	274	470	490	509	523	534	561	576	576	691	782	796	827	326
1999	273	100	M/D	1/23	1/22	1/21	1/21	1/20	1/18	1/17	1/17	1/16	1/16	1/16	1/16	1/17	1/17	
			.1MM	183	259	282	300	319	451	551	569	583	583	583	817	909	1110	585
2000	274	100	M/D	2/24	2/23	2/24	2/24	2/23	2/22	2/24	2/23	2/22	2/21	2/20	2/16	2/21	2/16	
			.1MM	236	301	357	474	538	584	689	754	800	818	868	886	1125	1148	555
2001	273	100	M/D	2/ 9	12/16	12/16	4/ 4	4/ 3	4/ 2	4/ 2	4/ 1	3/31	3/30	3/25	3/19	3/15	3/10	
			.1MM	427	474	474	511	593	653	710	729	742	767	872	1282	1472	1663	1300
2002	273	100	M/D	3/ 2	3/ 1	2/28	2/28	1/23	1/22	1/21	1/20	1/20	1/23	1/20	1/22	2/ 6	1/22	
			.1MM	246	273	296	296	313	350	359	382	382	461	548	693	801	1000	306
2003	273	100	M/D	3/20	3/20	3/20	3/20	3/20	3/20	3/20	3/20	3/19	3/17	3/15	3/ 8	3/17	3/12	
			.1MM	255	390	472	563	650	816	921	1016	1034	1136	1377	1418	1456	1625	1082
2004	274	100	M/D	12/10	12/10	12/10	12/10	3/ 1	2/29	2/28	2/28	2/27	2/27	12/10	2/25	2/20	2/25	
			.1MM	496	496	496	496	574	638	716	762	798	816	1041	1164	1230	1368	857
2005	273	100	M/D	12/23	12/22	12/21	12/21	2/11	12/18	12/18	12/23	12/22	2/ 6	2/ 1	2/ 1	12/21	12/16	
			.1MM	217	316	334	334	343	357	357	392	490	536	637	637	810	861	293
2006	273	100	M/D	11/29	11/28	11/27	11/27	11/27	11/27	11/27	11/27	11/27	11/27	1/ 4	1/ 4	12/24	12/22	
			.1MM	469	688	780	780	780	780	780	780	780	780	871	882	1052	1098	290
2007	273	100	M/D	3/14	3/13	3/12	3/11	3/10	3/10	3/10	3/10	3/10	3/10	3/ 1	2/27	2/27	2/21	
			.1MM	166	258	317	376	432	450	450	450	460	460	530	562	629	670	456
2008	274	100	M/D	12/23	12/22	1/ 7	1/ 6	1/ 7	1/ 6	4/ 1	3/31	3/30	3/30	3/25	12/23	3/14	3/11	
			.1MM	373	414	456	524	571	640	730	813	841	862	1013	1212	1438	1482	1090
2009	273	100	M/D	2/11	2/11	2/10	12/24	12/24	12/24	3/ 5	3/ 5	3/ 6	3/ 6	2/25	2/25	2/25	2/10	
			.1MM	255	369	419	457	552	581	590	590	603	699	887	1059	1059	1334	811
2010	273	100	M/D	1/24	1/24	1/24	1/24	1/24	1/24	1/19	1/19	1/17	1/16	1/14	1/14	1/ 1	12/27	
			.1MM	494	780	794	794	794	794	798	812	835	853	922	922	926	958	392
2011	226	83	**	M/D	3/10	3/10	3/10	3/10	3/10	3/ 5	3/ 5	3/10	3/10	3/10	3/ 4	3/10	3/ 5	
			.1MM	386	461	484	511	539	703	778	809	901	937	1254	1396	1569	1886	
2012	274	100	M/D	11/24	1/31	1/31	1/23	1/22	1/23	1/17	1/17	1/23	1/23	1/17	1/16	1/12	1/ 6	
			.1MM	152	157	157	257	262	262	273	286	359	419	499	572	625	679	170
2013	273	100	M/D	1/30	1/29	1/29	3/ 9	3/ 8	3/ 8	3/ 6	3/ 6	3/ 5	3/ 6	2/26	1/12	1/ 8	1/ 6	
			.1MM	347	460	524	542	584	606	643	666	675	689	770	968	1096	1115	595
2014	273	100	M/D	4/ 7	4/ 7	4/ 7	4/ 7	4/ 4	4/ 4	4/ 4	4/ 3	4/ 2	4/ 1	3/27	3/22	3/18	3/14	
			.1MM	367	486	559	711	789	862	1014	1060	1115	1179	1452	1479	1698	1798	1054
2015	273	100	M/D	4/10	4/ 9	4/ 8	4/ 7	4/ 6	4/ 5	4/ 4	4/ 3	4/ 2	4/ 1	3/29	3/24	3/17	3/12	
			.1MM	250	401	447	511	534	580	621	749	928	965	1070	1220	1302	1554	1067
2016	114	42	**	M/D	12/29	1/ 7	1/ 6	1/ 6	1/ 6	1/ 3	1/ 2	1/ 2	12/31	12/29	12/29	12/20	12/20	12/20
			.1MM	34	54	77	77	77	91	95	95	109	116	148	168	168	181	

1 DAY 2 DAY 3 DAY 4 DAY 5 DAY 6 DAY 7 DAY 8 DAY 9 DAY 10 DAY 15 DAY 20 DAY 25 DAY 30 DAY

MEAN EXTREME	(MM)	30.7	41.3	47.3	52.8	58.3	62.9	68.0	72.2	76.4	80.2	96.0	110.1	121.8	133.9
STD. DEV.	(MM)	9.2	12.7	13.3	13.7	15.7	16.7	18.7	20.1	20.9	22.7	29.3	34.7	38.6	41.3
YEARS ANALYSED		45	45	45	45	45	45	45	45	45	45	45	45	45	45
MAX EXTREME	(MM)	73.0	93.8	100.5	114.2	125.8	136.5	147.0	155.8	183.1	196.6	235.7	238.5	268.7	293.0
YEAR		1984	1984	1984	1976	1977	2001	2001	1976	1971	1971	1971	1978	1978	1978

** NOTE ** VALUE IN FLAG INDICATES YEAR NOT INCLUDED IN ANALYSIS BASED ON % DAYS OPERATIONAL (<90.0%)

METEOROLOGICAL SERVICE OF CANADA
 RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
 PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 4 - Southern Ontario

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

RETURN PERIOD VALUES (MM) WITH 50% CONFIDENCE LIMITS

RETURN PERIOD
YEARS

	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY
2	29.23+/- 0.85	39.20+/- 1.17	45.16+/- 1.23	50.52+/- 1.26	55.69+/- 1.44
5	37.38+/- 1.43	50.45+/- 1.98	56.93+/- 2.07	62.64+/- 2.13	69.52+/- 2.43
10	42.77+/- 1.93	57.89+/- 2.67	64.72+/- 2.79	70.66+/- 2.88	78.68+/- 3.28
25	49.59+/- 2.61	67.30+/- 3.60	74.56+/- 3.77	80.80+/- 3.88	90.25+/- 4.43
50	54.65+/- 3.12	74.28+/- 4.31	81.87+/- 4.51	88.32+/- 4.64	98.84+/- 5.30
100	59.67+/- 3.64	81.21+/- 5.02	89.11+/- 5.25	95.78+/- 5.41	107.36+/- 6.17

RETURN PERIOD
YEARS

	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY
2	60.14+/- 1.54	64.91+/- 1.72	68.89+/- 1.85	72.98+/- 1.93	76.49+/- 2.09
5	74.91+/- 2.60	81.39+/- 2.90	86.61+/- 3.12	91.44+/- 3.24	96.50+/- 3.52
10	84.69+/- 3.51	92.31+/- 3.91	98.35+/- 4.21	103.66+/- 4.38	109.76+/- 4.75
25	97.05+/- 4.73	106.10+/- 5.28	113.18+/- 5.67	119.09+/- 5.91	126.50+/- 6.41
50	106.22+/- 5.66	116.33+/- 6.31	124.18+/- 6.79	130.55+/- 7.07	138.93+/- 7.67
100	115.32+/- 6.59	126.49+/- 7.36	135.10+/- 7.91	141.92+/- 8.23	151.26+/- 8.93

RETURN PERIOD
YEARS

	15 DAY	20 DAY	25 DAY	30 DAY
2	91.18+/- 2.70	104.38+/- 3.20	115.50+/- 3.56	127.13+/- 3.81
5	117.07+/- 4.55	135.05+/- 5.39	149.63+/- 6.00	163.62+/- 6.41
10	134.22+/- 6.15	155.36+/- 7.28	172.23+/- 8.10	187.78+/- 8.66
25	155.88+/- 8.29	181.02+/- 9.82	200.78+/-10.92	218.30+/-11.68

50
100

171.95+/- 9.92 200.05+/-11.75 221.96+/-13.07 240.95+/-13.98
187.90+/-11.55 218.94+/-13.68 242.99+/-15.23 263.43+/-16.28

METEOROLOGICAL SERVICE OF CANADA
RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 5 - Modification of Model 4

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

YR	TOTAL % DAYS VALID	START FLAG MAX	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY	MAX SNPK
1968	198 72	** M/D .1MM	2/ 2 303	2/ 1 454	2/ 1 454	3/16 605	3/15 648	3/15 648	3/15 648	3/15 648	3/15 648	3/10 740	3/ 9 840	3/ 9 899	3/ 9 899	3/ 9 899	
1969	273 100	M/D .1MM	3/24 466	3/24 543	3/22 567	3/21 648	3/20 826	3/20 902	3/19 921	3/18 981	3/17 1003	3/17 1003	3/18 1162	3/17 1457	3/15 1466	3/15 1466	735
1970	242 89	** M/D .1MM	12/10 340	4/ 8 526	4/ 7 627	4/ 6 658	4/ 6 658	4/ 7 725	4/ 6 756	4/ 2 862	4/ 1 871	4/ 1 871	3/26 1018	3/21 1424	3/19 1617	3/19 1617	
1971	273 100	M/D .1MM	4/19 365	4/19 654	4/18 897	4/18 1084	4/17 1195	4/16 1358	4/15 1439	4/12 1552	4/12 1840	4/12 2027	4/ 9 2318	4/ 2 2422	3/29 2511	3/29 2511	2336
1972	274 100	M/D .1MM	2/13 284	4/14 420	4/13 600	4/12 762	4/11 914	4/13 1064	4/12 1225	4/11 1377	4/10 1399	4/10 1399	4/10 1399	4/10 1578	3/29 1601	3/21 1872	1224
1973	273 100	M/D .1MM	3/ 7 261	1/18 389	1/17 523	3/ 4 688	3/ 3 769	3/ 2 772	3/ 1 797	3/ 1 797	3/ 1 797	3/ 1 797	1/16 818	1/16 1136	1/16 1139	12/26 1179	896
1974	273 100	M/D .1MM	4/ 3 431	4/ 2 496	4/ 1 664	3/31 746	3/30 896	3/29 942	3/29 961	3/29 961	3/29 961	3/29 961	2/21 1060	2/19 1098	2/13 1109	3/ 5 1127	732
1975	273 100	M/D .1MM	3/19 453	3/18 596	3/18 687	3/17 718	3/18 827	3/18 877	3/18 984	3/18 1065	3/17 1096	3/17 1096	3/12 1165	3/12 1227	3/12 1290	3/19 1772	866
1976	274 100	M/D .1MM	3/20 344	3/24 657	3/24 840	3/24 1022	3/24 1022	3/20 1078	3/20 1261	3/20 1443	3/19 1519	3/19 1519	3/13 1539	3/12 1587	3/ 3 1917	2/26 2193	1254
1977	273 100	M/D .1MM	3/12 398	3/11 672	3/10 947	3/ 9 1141	3/ 9 1141	3/ 7 1152	3/ 6 1203	3/ 5 1325	3/ 4 1589	3/ 3 1602	2/26 1712	2/24 1948	3/ 4 2009	3/ 3 2099	1327
1978	273 100	M/D .1MM	1/24 491	4/11 526	4/11 810	4/11 921	4/11 1022	4/11 1205	4/11 1327	4/11 1327	4/11 1327	4/11 1327	4/ 4 1763	3/29 2220	3/27 2457	3/20 2784	1856
1979	273 100	M/D .1MM	1/ 1 268	12/31 525	12/30 565	3/ 3 579	3/ 3 633	3/ 3 716	3/ 3 869	3/ 3 905	3/ 1 949	3/ 1 985	2/23 1132	3/ 1 1332	2/24 1387	2/21 1593	892
1980	274 100	M/D .1MM	3/21 496	3/20 747	3/20 747	3/18 765	3/17 812	3/17 812	3/17 812	3/17 812	3/17 812	3/17 812	3/17 812	3/17 812	3/17 812	3/17 812	491
1981	273 100	M/D .1MM	2/19 381	2/18 628	2/17 784	2/16 952	2/16 952	2/16 952	2/16 952	2/16 952	2/11 1059	2/10 1289	2/10 1289	2/ 1 1359	1/26 1538	1/26 1538	637
1982	273 100	M/D .1MM	3/31 404	3/30 601	3/30 641	3/30 641	3/30 641	3/30 641	3/25 895	3/24 950	3/24 990	3/24 990	3/17 1197	3/12 1555	3/11 1686	3/11 1686	1133
1983	273 100	M/D .1MM	2/ 2 380	2/ 2 533	2/ 2 533	1/31 552	1/30 562	1/30 562	1/30 562	1/30 562	1/30 562	1/25 581	1/23 623	1/30 632	1/10 1000	1/10 1000	260
1984	274 100	M/D .1MM	2/14 558	2/13 794	2/13 876	2/11 987	2/13 1112	2/13 1180	2/11 1306	2/11 1373	2/11 1373	2/11 1373	2/ 3 1379	2/ 2 1506	1/24 1549	1/24 1616	828

1985	273	100	M/D	2/23	2/22	2/22	2/21	2/21	2/21	2/21	2/22	2/21	2/21	2/22	2/22	2/21	2/13	
			.1MM	421	700	842	932	950	950	950	980	1070	1143	1145	1797	1975	2003	908
1986	273	100	M/D	3/26	3/25	1/18	3/23	3/23	3/23	3/23	3/19	3/18	3/18	3/13	3/10	3/10	3/10	
			.1MM	384	603	628	709	709	709	709	976	1113	1113	1389	1493	1493	1493	1113
1987	273	100	M/D	3/ 8	3/ 7	11/24	11/23	11/23	11/23	11/23	3/ 1	2/28	2/28	2/28	2/28	2/28	2/28	
			.1MM	228	374	385	420	420	420	420	602	722	722	722	896	1033	1033	680
1988	274	100	M/D	3/25	11/28	3/24	3/23	3/23	3/23	3/23	3/23	3/23	3/23	11/28	3/ 7	11/28	11/28	
			.1MM	456	710	807	877	877	877	877	877	877	877	933	1165	1240	1369	658
1989	273	100	M/D	3/27	3/27	3/26	3/25	3/24	3/24	3/24	3/24	3/24	3/24	3/14	3/14	3/ 4	3/ 4	
			.1MM	324	539	676	768	865	865	865	865	905	905	1127	1167	1187	1227	795
1990	273	100	M/D	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/17	1/ 9	1/17	1/17	
			.1MM	329	494	494	494	494	494	534	647	797	797	806	881	1071	1285	540
1991	273	100	M/D	12/29	3/ 1	2/ 3	2/ 3	2/ 3	3/ 1	3/ 1	3/ 1	3/ 1	3/ 1	2/20	2/18	3/ 1	2/ 3	
			.1MM	390	402	484	511	511	602	602	602	602	602	742	854	950	1165	492
1992	274	100	M/D	3/26	3/25	3/ 8	3/ 7	3/ 6	3/ 6	3/ 6	3/25	3/25	3/25	3/25	3/ 7	3/ 7	3/ 6	
			.1MM	388	468	588	727	811	811	811	848	894	903	1093	1196	1502	1733	694
1993	273	100	M/D	1/ 4	1/ 3	3/27	3/26	3/25	3/24	3/24	3/24	3/24	3/24	3/24	3/21	3/16	3/ 8	
			.1MM	553	667	718	792	883	974	1030	1030	1030	1070	1290	1299	1319	1487	1016
1994	272	100	M/D	2/20	3/22	3/22	3/21	3/21	3/21	3/21	3/21	3/21	3/22	3/21	3/21	3/15	3/15	
			.1MM	262	420	585	665	665	665	675	748	803	832	1297	1407	1472	1509	1029
1995	273	100	M/D	3/13	1/13	1/12	1/12	1/12	1/12	3/ 7	3/ 6	3/ 5	3/ 5	3/ 5	3/ 5	2/19	2/15	
			.1MM	292	418	485	485	485	485	568	631	671	705	705	705	796	806	409
1996	274	100	M/D	11/11	1/18	1/16	1/16	2/20	2/20	2/19	2/20	2/20	2/19	1/16	2/ 8	2/19	2/19	
			.1MM	395	465	596	700	789	908	926	948	975	993	1042	1146	1264	1471	854
1997	273	100	M/D	3/25	3/28	3/27	3/27	3/25	3/25	3/25	3/25	3/25	3/25	3/25	3/14	3/14	3/ 1	
			.1MM	278	477	660	742	984	1066	1066	1103	1197	1197	1197	1220	1220	1264	771
1998	273	100	M/D	1/ 7	3/ 8	1/ 5	1/ 5	1/ 3	1/ 2	1/ 2	1/ 2	1/ 2	1/ 2	2/25	2/18	2/17	2/25	
			.1MM	254	380	466	486	561	597	617	627	627	627	811	912	1074	1206	395
1999	273	100	M/D	1/23	2/28	2/10	2/28	3/17	1/18	1/17	1/17	1/17	1/17	2/ 2	1/17	1/18	1/17	
			.1MM	238	292	324	332	353	432	519	519	519	519	554	744	880	1069	585
2000	274	100	M/D	2/24	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	2/23	
			.1MM	328	484	540	687	837	837	837	837	837	837	882	882	1112	1153	610
2001	273	100	M/D	2/ 9	12/16	4/ 6	4/ 5	4/ 4	4/ 3	4/ 2	4/ 2	4/ 2	3/30	3/30	3/21	3/15	3/12	
			.1MM	408	538	783	994	1195	1323	1405	1405	1405	1425	1425	1783	1847	1933	1546
2002	273	100	M/D	3/ 2	2/20	1/26	3/27	1/23	1/23	2/15	2/15	1/23	1/23	2/19	2/15	2/10	1/23	
			.1MM	216	348	400	406	430	519	542	542	547	607	722	896	968	1221	467
2003	273	100	M/D	3/17	3/20	3/20	3/20	3/20	3/17	3/17	3/17	3/17	3/16	3/16	3/16	3/17	3/16	
			.1MM	310	472	668	851	1007	1107	1290	1446	1537	1628	1628	1628	1762	1853	1311
2004	274	100	M/D	12/10	12/10	3/ 3	3/ 2	3/ 1	3/ 1	2/29	2/28	2/28	2/28	2/29	2/28	3/ 1	2/28	
			.1MM	480	521	606	798	949	1022	1077	1086	1086	1086	1191	1237	1406	1471	930
2005	273	100	M/D	1/13	2/14	2/14	12/30	12/30	12/30	12/30	12/30	2/ 7	2/ 7	12/30	12/30	12/22	12/22	
			.1MM	236	350	368	420	475	475	475	475	624	642	778	778	1044	1044	384
2006	273	100	M/D	11/29	11/28	11/27	11/27	11/27	11/27	11/27	1/11	1/12	1/11	1/ 4	1/ 4	1/ 4	12/23	
			.1MM	428	730	780	780	780	780	780	789	793	871	957	1039	1130	1289	290
2007	273	100	M/D	4/12	3/13	3/12	3/11	3/10	3/10	3/10	3/10	3/10	3/10	3/10	3/ 3	3/ 1	3/ 1	
			.1MM	208	408	482	573	583	583	583	583	583	583	659	687	707	707	535
2008	274	100	M/D	12/23	1/ 7	1/ 6	1/ 5	4/ 1	4/ 1	3/31	3/31	3/31	3/28	3/26	3/18	3/15	3/ 8	
			.1MM	482	573	705	725	772	898	1018	1018	1018	1054	1164	1483	1634	1644	1251
2009	273	100	M/D	2/11	2/11	3/ 6	3/ 5	3/ 6	3/ 6	3/ 5	3/ 5	3/ 5	2/27	2/26	12/ 9	2/11	2/11	
			.1MM	369	556	658	664	738	770	776	776	776	820	990	1006	1289	1547	748
2010	273	100	M/D	1/24	1/24	1/24	1/24	1/24	1/24	1/19	1/19	1/19	1/16	1/15	1/15	1/ 1	12/27	
			.1MM	430	826	835	835	835	835	844	853	853	890	954	954	963	981	483
2011	226	83	**	M/D	3/10	3/10	3/16	3/16	3/16	3/ 5	3/ 5	3/10	3/10	3/10	3/ 5	2/27	2/27	2/18
			.1MM	404	543	659	659	659	712	851	974	1266	1266	1574	1715	1834	1971	

2012	274	100	M/D	11/24	11/24	11/24	1/23	1/23	3/ 2	1/26	1/26	1/24	1/23	1/23	1/17	1/17	1/17	
			.1MM	152	152	152	225	225	249	253	253	280	368	423	503	558	595	208
2013	273	100	M/D	3/11	3/10	3/10	3/10	3/ 9	3/ 9	3/ 7	3/ 7	3/ 7	3/ 7	2/27	1/12	1/ 9	2/27	
			.1MM	483	657	812	830	849	849	867	867	867	867	944	1097	1134	1147	703
2014	273	100	M/D	4/ 7	4/ 7	4/ 6	4/ 4	4/ 4	4/ 4	4/ 2	4/ 1	3/31	3/30	3/28	3/20	3/15	3/10	
			.1MM	440	578	642	780	918	918	1027	1100	1210	1238	1469	1551	1743	1926	1072
2015	273	100	M/D	4/ 3	4/ 2	4/ 2	4/ 2	3/30	4/ 2	4/ 2	4/ 2	4/ 2	4/ 2	3/26	3/21	3/10	3/11	
			.1MM	301	540	586	586	613	686	741	890	890	890	1037	1145	1303	1571	1140
2016	114	42	**	M/D	1/10	1/ 9	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	1/ 8	12/29	12/29	12/20	12/20	
			.1MM	276	484	498	498	498	498	498	498	498	498	560	560	580	580	

		1 DAY	2 DAY	3 DAY	4 DAY	5 DAY	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY	15 DAY	20 DAY	25 DAY	30 DAY
MEAN EXTREME	(MM)	36.5	53.2	63.2	71.1	77.5	82.0	87.1	91.8	96.4	99.0	109.9	123.2	134.5	145.4
STD. DEV.	(MM)	9.8	13.8	16.7	20.0	22.7	25.0	27.7	29.5	32.2	33.4	36.2	41.3	42.1	45.4
YEARS ANALYSED		45	45	45	45	45	45	45	45	45	45	45	45	45	45
MAX EXTREME	(MM)	73.0	93.8	100.5	114.2	125.8	136.5	147.0	155.8	184.0	202.7	235.7	242.2	268.7	293.0
YEAR		1984	1984	1984	1976	1977	2001	2001	1976	1971	1971	1971	1971	1978	1978

** NOTE ** VALUE IN FLAG INDICATES YEAR NOT INCLUDED IN ANALYSIS BASED ON % DAYS OPERATIONAL (<90.0%)

METEOROLOGICAL SERVICE OF CANADA
 RAIN+SNOWMELT DEPTH, DURATION, FREQUENCY VALUES
 PREPARED BY THE ENGINEERING CLIMATE SERVICES UNIT

STATION : HARTINGTON IHD

STATION NUMBER 6103367

LATITUDE: 44.43N LONGITUDE: 76.69W ELEVATION (M): 160

ARCHIVE: DLY04 (15/12/1967 - 31/12/2006) DLY44 (01/01/2007 - 22/01/2016)

SNOWMELT MODEL 5 - Modification of Model 4

CRITICAL PERIOD : 1ST OF MONTH 10 (PRECEDING YEAR) TO THE END OF MONTH 6

NOTE : MODIFIED GUMBEL 12/82

RETURN PERIOD VALUES (MM) WITH 50% CONFIDENCE LIMITS

RETURN PERIOD
YEARS

	1 DAY	2 DAY	3 DAY	4 DAY	5 DAY
2	34.94+/- 0.90	50.89+/- 1.27	60.45+/- 1.54	67.82+/- 1.85	73.79+/- 2.10
5	43.57+/- 1.52	63.11+/- 2.15	75.20+/- 2.59	85.51+/- 3.11	93.89+/- 3.53
10	49.28+/- 2.05	71.19+/- 2.90	84.96+/- 3.50	97.22+/- 4.20	107.20+/- 4.77
25	56.50+/- 2.76	81.41+/- 3.91	97.30+/- 4.72	112.02+/- 5.66	124.02+/- 6.43
50	61.86+/- 3.31	88.99+/- 4.68	106.45+/- 5.65	123.00+/- 6.77	136.49+/- 7.70
100	67.17+/- 3.85	96.51+/- 5.45	115.54+/- 6.58	133.89+/- 7.89	148.87+/- 8.97

RETURN PERIOD
YEARS

	6 DAY	7 DAY	8 DAY	9 DAY	10 DAY
2	77.94+/- 2.30	82.56+/- 2.55	86.94+/- 2.72	91.12+/- 2.97	93.55+/- 3.08

5	100.00+/- 3.88	107.01+/- 4.30	113.03+/- 4.58	119.56+/- 5.00	123.11+/- 5.20
10	114.61+/- 5.24	123.20+/- 5.80	130.30+/- 6.19	138.39+/- 6.75	142.69+/- 7.02
25	133.06+/- 7.06	143.65+/- 7.83	152.12+/- 8.35	162.18+/- 9.10	167.41+/- 9.46
50	146.75+/- 8.45	158.83+/- 9.36	168.31+/- 9.99	179.83+/-10.89	185.76+/-11.32
100	160.34+/- 9.84	173.89+/-10.91	184.37+/-11.64	197.35+/-12.69	203.97+/-13.19

RETURN PERIOD

YEARS	15 DAY	20 DAY	25 DAY	30 DAY
2	103.93+/- 3.34	116.38+/- 3.81	127.63+/- 3.89	137.99+/- 4.18
5	135.91+/- 5.62	152.85+/- 6.41	164.86+/- 6.54	178.08+/- 7.05
10	157.09+/- 7.59	177.00+/- 8.66	189.51+/- 8.84	204.62+/- 9.52
25	183.84+/-10.24	207.51+/-11.67	220.66+/-11.92	238.16+/-12.83
50	203.69+/-12.25	230.14+/-13.97	243.76+/-14.26	263.04+/-15.35
100	223.39+/-14.27	252.61+/-16.27	266.70+/-16.61	287.74+/-17.89

** WARNING ** : 100 YEAR VALUES IN 1971 BASED ON 15 DAYS ACCUMULATION

Appendix C-2:
Sinusoidal Hourly Snowmelt + Rainfall Runoff for 1%, 3-Day AEP



Time (hr)	Fraction	Snowmelt (mm/hr)
1	0.260	1.67
2	0.500	1.72
3	0.707	1.77
4	0.866	1.81
5	0.966	1.83
6	1.000	1.84
7	0.966	1.83
8	0.866	1.81
9	0.707	1.77
10	0.500	1.72
11	0.260	1.67
12	0.000	1.60
13	-0.260	1.54
14	-0.500	1.49
15	-0.707	1.44
16	-0.866	1.40
17	-0.966	1.38
18	-1.000	1.37
19	-0.966	1.38
20	-0.866	1.40
21	-0.707	1.44
22	-0.500	1.49
23	-0.260	1.54
24	0.000	1.60

100-Yr

3 # of days

115.5 total depth from Hartington AES Frequency Values

Daily snowmelt +rain= 38.5 mm

April:	Daily min temp	0.9
	Daily avg	6.3
	Daily max temp	11.6

Average hourly snowmelt+rain 1.60 mm

Center 1.60 mm

Amplitude 0.24 mm

Repeat 24-hr pattern for 3 days in HEC-HMS hyetograph.

Appendix D:
**Historical Maximum Snow Water Content and Cumulative Runoff from
Hartington Climate Data & Schroeter Snowmelt Routine**



Year	Maximum SWC (mm)	Cumulative Snowmelt + Rain Runoff (mm)
1975	92	342
1976	190	426
1977	83	278
1978	205	361
1979	112	411
1980	23	367
1981	53	355
1982	79	239
1983	15	439
1984	101	465
1985	46	341
1986	68	299
1987	47	321
1988	56	423
1989	89	289
1990	57	418
1991	34	436
1992	62	297
1993	98	366
1994	67	302
1995	32	362
1996	127	350
1997	89	374
1998	23	307
1999	35	258
2000	27	382
2001	154	304
2002	64	432
2003	204	394
2004	221	527
2005	28	367
2006	26	388
2007	19	342
2008	131	373
2009	104	355
2010	45	313
2011	93	391
2012	45	319
2013	48	289
2014	80	351
2015	49	214
2016	73	396
2017	63	400
2018	65	395
2019	100	418
2020	31	395
2021	42	277
2022	56	316
2023	38	397

Appendix E: Federal Climate Data Portal: ΔT Adjustment



time	rcp26_tg_mean_p10	rcp26_tg_mean_p50	rcp26_tg_mean_p90	rcp45_tg_mean_p10	rcp45_tg_mean_p50	rcp45_tg_mean_p90	rcp85_tg_mean_p10	rcp85_tg_mean_p50	rcp85_tg_mean_p90	rcp26_tg_mean_delta7100_p10	rcp26_tg_mean_delta7100_p50	rcp26_tg_mean_delta7100_p90	rcp45_tg_mean_delta7100_p10	rcp45_tg_mean_delta7100_p50	rcp45_tg_mean_delta7100_p90	rcp85_tg_mean_delta7100_p10	rcp85_tg_mean_delta7100_p50	rcp85_tg_mean_delta7100_p90
1/1/1951	6.9	7	7.2	6.9	7	7.2	6.9	7	7.2	-0.6	-0.3	-0.1	-0.6	-0.3	-0.1	-0.6	-0.3	-0.1
1/1/1961	7	7.1	7.2	7	7.1	7.2	7	7.1	7.2	-0.5	-0.3	-0.1	-0.5	-0.3	-0.1	-0.5	-0.3	-0.1
1/1/1971	7.2	7.3	7.5	7.2	7.3	7.5	7.2	7.3	7.5	0	0	0	0	0	0	0	0	0
1/1/1981	7.5	7.7	7.9	7.5	7.7	7.9	7.5	7.7	7.9	0.2	0.3	0.4	0.2	0.4	0.5	0.2	0.3	0.5
1/1/1991	7.9	8	8.4	7.8	8.1	8.4	7.9	8.2	8.4	0.5	0.7	1	0.5	0.8	1	0.6	0.8	1
1/1/2001	8.2	8.4	9	8.2	8.6	9	8.2	8.7	9	0.8	1.2	1.6	0.7	1.2	1.6	0.9	1.3	1.6
1/1/2011	8.3	8.8	9.5	8.4	9	9.7	8.5	9.2	9.8	1	1.4	2.1	1.1	1.7	2.3	1.3	1.8	2.4
1/1/2021	8.5	9.2	10	8.8	9.3	10.2	8.9	9.6	10.3	1.2	1.8	2.6	1.4	2	2.9	1.7	2.2	3
1/1/2031	8.5	9.4	10.3	9.1	9.7	10.8	9.4	10.2	11.3	1.3	2	2.9	1.7	2.4	3.4	2.2	2.8	3.9
1/1/2041	8.6	9.4	10.6	9.2	10.1	11.3	10.1	10.9	12.2	1.3	2.1	3.3	1.8	2.8	4	2.8	3.5	4.8
1/1/2051	8.6	9.4	10.8	9.3	10.3	11.8	10.8	11.6	13.3	1.2	2.1	3.4	1.9	2.9	4.5	3.4	4.2	6
1/1/2061	8.6	9.4	10.8	9.4	10.4	12.2	11.4	12.4	14.2	1.2	2.1	3.4	2	3.2	4.8	4.1	5	6.8
1/1/2071	8.6	9.4	10.7	9.5	10.6	12.1	11.8	13.1	15.1	1.2	2.1	3.3	2.2	3.3	4.7	4.6	5.7	7.7

Appendix F:
Kingston Environment Canada IDFs



Environment and Climate Change Canada
 Environnement et Changement climatique Canada

Short Duration Rainfall Intensity-Duration-Frequency Data
 Données sur l'intensité, la durée et la fréquence des chutes
 de pluie de courte durée

Gumbel - Method of moments/Méthode des moments

2022/10/31

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=====
KINGSTON PUMPING STATION                               ON          6104175
(composite)
Latitude: 44 14'N   Longitude: 76 29'W   Elevation/Altitude: 76      m
Years/Années : 1914 - 2007           # Years/Années : 63
=====
```

Table 1 : Annual Maximum (mm)/Maximum annuel (mm)

Year Année	5 min	10 min	15 min	30 min	1 h	2 h	6 h	12 h	24 h
1914	5.8	7.9	9.7	12.2	19.3	25.4	29.2	37.1	40.1
1915	3.6	5.3	7.6	10.2	16.0	28.2	45.2	58.9	65.0
1921	8.1	16.0	17.0	18.0	18.3	18.3	25.9	27.9	45.0
1922	4.8	6.9	9.7	15.7	20.6	22.6	32.3	32.3	36.1
1926	8.6	13.0	18.0	23.4	33.5	48.8	54.6	54.6	57.9
1927	8.6	13.0	16.5	21.8	39.1	39.9	40.1	40.1	49.8
1928	10.9	16.8	22.6	29.0	34.8	35.8	41.1	41.7	48.0
1929	10.7	14.7	14.7	18.8	23.9	28.4	30.2	39.6	52.8
1930	8.4	11.4	11.4	13.2	15.7	17.0	21.8	34.8	44.4
1931	6.9	10.7	14.2	17.5	18.8	25.9	27.7	39.4	39.4
1932	8.9	15.7	18.3	21.3	21.3	21.3	26.9	34.3	50.8
1933	7.1	11.2	12.2	14.7	15.5	18.3	38.4	47.0	51.1
1934	5.1	9.1	11.4	15.2	18.5	20.1	26.7	40.4	45.0
1935	10.2	14.2	19.8	34.5	40.4	55.4	59.4	59.4	64.5
1936	4.6	8.1	8.4	10.9	15.0	22.6	32.8	47.5	49.5
1937	10.2	15.0	19.8	28.7	42.4	52.3	62.5	78.7	79.8
1938	8.9	13.0	13.0	13.0	19.0	33.5	42.7	42.7	57.9
1961	5.3	9.4	13.2	13.2	13.2	20.6	32.3	34.5	35.8
1962	10.7	13.2	19.8	24.1	24.4	25.4	30.2	40.6	42.2
1963	7.6	8.9	12.4	17.8	21.6	27.9	32.3	32.8	39.6
1964	4.1	6.3	6.6	7.6	10.4	16.3	23.6	32.8	33.3
1965	5.6	8.1	9.4	16.0	23.4	26.2	26.7	33.5	37.3
1966	5.1	9.1	10.9	15.0	23.6	24.1	46.7	47.0	47.0
1967	12.4	14.2	16.3	24.9	35.8	37.1	37.1	46.2	51.3
1968	10.4	13.2	16.0	19.3	21.1	22.9	30.2	43.7	45.0
1969	6.9	8.4	9.9	10.4	15.7	19.6	32.0	39.9	47.8
1970	5.6	9.1	12.4	21.6	24.1	26.7	26.9	28.2	28.4
1971	8.1	10.2	10.7	14.0	15.2	17.5	25.4	30.0	47.0
1972	9.7	11.4	12.7	15.5	20.8	36.1	54.1	79.2	79.5
1973	8.6	10.4	12.2	16.5	24.1	26.9	33.5	34.8	43.4
1974	4.8	6.3	7.1	7.6	8.9	14.0	17.3	20.8	23.9
1975	7.6	13.2	15.2	16.3	17.0	22.6	40.1	52.1	65.0
1976	10.9	14.5	16.0	16.3	27.4	35.8	37.1	38.4	41.7
1977	8.1	9.7	11.4	13.0	14.2	19.0	42.2	45.2	45.5
1978	7.4	10.0	12.8	16.4	17.1	23.8	36.0	40.2	43.0

1979	7.4	10.2	12.4	14.1	22.4	37.9	100.6	126.0	127.1
1980	8.6	11.7	14.8	19.9	28.0	43.9	44.8	44.8	53.0
1981	6.4	8.1	9.5	17.6	28.8	39.3	56.3	57.0	59.1
1982	9.3	14.4	15.8	16.0	16.0	23.7	39.7	45.0	45.0
1983	15.4	18.2	18.6	21.0	26.9	34.1	50.8	55.4	70.6
1984	6.4	6.6	7.1	10.4	15.7	22.5	30.6	38.6	45.4
1985	15.8	19.8	26.7	34.3	39.0	50.5	53.6	53.6	53.7
1986	7.0	9.0	9.7	15.0	19.8	26.7	50.2	65.3	74.2
1987	4.0	6.5	9.7	10.5	14.3	16.8	30.0	48.4	56.6
1988	6.6	10.3	12.8	16.2	25.5	28.1	28.8	28.8	30.2
1989	3.8	6.6	8.4	11.5	12.0	15.2	30.5	33.7	42.0
1990	7.4	10.8	14.3	14.3	16.5	20.7	37.9	38.7	41.4
1991	11.4	12.7	13.1	19.3	33.9	34.6	37.4	37.6	43.4
1992	7.6	10.8	14.2	24.0	35.1	41.9	44.7	46.2	46.2
1993	8.0	12.8	16.0	24.6	28.6	31.7	32.5	32.9	64.6
1994	11.1	14.1	16.4	20.3	25.6	29.4	47.7	51.8	52.8
1995	4.4	5.7	7.2	8.5	13.0	18.3	33.7	49.2	56.1
1996	5.7	7.7	8.5	10.9	13.0	18.0	28.0	33.8	44.5
1997	9.4	13.2	15.5	21.2	21.2	28.3	33.1	34.0	39.2
1998	10.4	14.1	16.6	19.0	20.8	21.0	28.9	32.9	35.0
1999	6.5	9.8	10.8	12.0	16.2	22.0	28.3	41.2	44.7
2000	7.7	8.1	9.8	11.4	17.8	20.8	28.1	35.7	49.4
2001	5.8	8.8	11.9	19.3	20.8	21.2	27.4	36.9	44.8
2002	8.0	10.0	10.4	12.6	15.2	19.7	26.4	36.9	43.1
2003	8.4	15.4	17.7	25.1	28.0	31.6	38.2	43.2	45.0
2005	9.2	13.8	14.9	20.0	20.8	24.3	49.2	63.2	65.1
2006	9.0	11.6	12.9	15.7	22.8	25.8	28.8	35.0	39.1
2007	7.1	12.0	13.4	14.5	19.6	21.2	24.4	34.5	36.6

# Yrs.	63	63	63	63	63	63	63	63	63
Années									
Mean	7.9	11.1	13.3	17.2	22.0	27.4	37.0	43.8	49.6
Moyenne									
Std. Dev.	2.5	3.2	4.0	5.8	7.8	9.5	12.9	15.4	15.2
Écart-type									
Skew.	0.73	0.32	0.70	0.90	0.88	1.14	2.21	2.89	2.42
Dissymétrie									
Kurtosis	4.18	2.78	3.95	4.10	3.34	3.97	11.33	15.59	13.18

*-99.9 Indicates Missing Data/Données manquantes

Warning: annual maximum amount greater than 100-yr return period amount
 Avertissement : la quantité maximale annuelle excède la quantité
 pour une période de retour de 100 ans

Year/Année	Duration/Durée	Data/Données	100-yr/ans
1979	6 h	100.6	77.5
1979	12 h	126.0	92.0
1979	24 h	127.1	97.1
1985	15 min	26.7	25.9

Table 2a : Return Period Rainfall Amounts (mm)
 Quantité de pluie (mm) par période de retour

Duration/Durée	2	5	10	25	50	100	#Years
	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	Années
5 min	7.5	9.7	11.2	13.1	14.5	15.9	63
10 min	10.6	13.4	15.3	17.7	19.5	21.2	63
15 min	12.6	16.2	18.5	21.5	23.7	25.9	63
30 min	16.2	21.4	24.8	29.1	32.3	35.5	63
1 h	20.7	27.6	32.2	37.9	42.2	46.4	63

2 h	25.8	34.2	39.8	46.8	52.1	57.3	63
6 h	34.9	46.3	53.8	63.4	70.5	77.5	63
12 h	41.2	54.8	63.8	75.2	83.7	92.0	63
24 h	47.1	60.5	69.3	80.5	88.8	97.1	63

Table 2b :

Return Period Rainfall Rates (mm/h) - 95% Confidence limits
 Intensité de la pluie (mm/h) par période de retour - Limites de confiance de 95%

Duration/Durée	2		5		10		25		50		100		#Years Années
	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	
5 min	89.9	116.8	134.7	157.2	173.9	190.5	63						63
	+/- 6.9	+/- 11.6	+/- 15.7	+/- 21.2	+/- 25.4	+/- 29.5							63
10 min	63.5	80.6	91.9	106.3	116.9	127.4	63						63
	+/- 4.4	+/- 7.4	+/- 10.0	+/- 13.5	+/- 16.1	+/- 18.7							63
15 min	50.6	64.8	74.2	86.1	94.9	103.6	63						63
	+/- 3.6	+/- 6.1	+/- 8.3	+/- 11.2	+/- 13.4	+/- 15.6							63
30 min	32.5	42.8	49.6	58.2	64.6	70.9	63						63
	+/- 2.6	+/- 4.5	+/- 6.0	+/- 8.1	+/- 9.7	+/- 11.3							63
1 h	20.7	27.6	32.2	37.9	42.2	46.4	63						63
	+/- 1.8	+/- 3.0	+/- 4.0	+/- 5.4	+/- 6.5	+/- 7.5							63
2 h	12.9	17.1	19.9	23.4	26.0	28.6	63						63
	+/- 1.1	+/- 1.8	+/- 2.5	+/- 3.3	+/- 4.0	+/- 4.6							63
6 h	5.8	7.7	9.0	10.6	11.7	12.9	63						63
	+/- 0.5	+/- 0.8	+/- 1.1	+/- 1.5	+/- 1.8	+/- 2.1							63
12 h	3.4	4.6	5.3	6.3	7.0	7.7	63						63
	+/- 0.3	+/- 0.5	+/- 0.7	+/- 0.9	+/- 1.1	+/- 1.2							63
24 h	2.0	2.5	2.9	3.4	3.7	4.0	63						63
	+/- 0.1	+/- 0.2	+/- 0.3	+/- 0.4	+/- 0.5	+/- 0.6							63

Table 3 : Interpolation Equation / Équation d'interpolation: $R = A \cdot T^B$

R = Interpolated Rainfall rate (mm/h)/Intensité interpolée de la pluie (mm/h)

RR = Rainfall rate (mm/h) / Intensité de la pluie (mm/h)

T = Rainfall duration (h) / Durée de la pluie (h)

Statistics/Statistiques	2		5		10		25		50		100	
	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans	yr/ans
Mean of RR/Moyenne de RR	31.3	40.5	46.6	54.4	60.1	65.8						
Std. Dev. /Écart-type (RR)	30.8	39.6	45.5	52.9	58.3	63.8						
Std. Error/Erreur-type	5.3	6.8	7.8	9.1	10.1	11.1						
Coefficient (A)	19.1	24.9	28.8	33.6	37.3	40.9						
Exponent/Exposant (B)	-0.681	-0.677	-0.676	-0.674	-0.673	-0.672						
Mean % Error/% erreur moyenne	6.6	7.4	7.7	8.1	8.3	8.5						

Appendix G: CFA Output – Three Parameter Log Normal



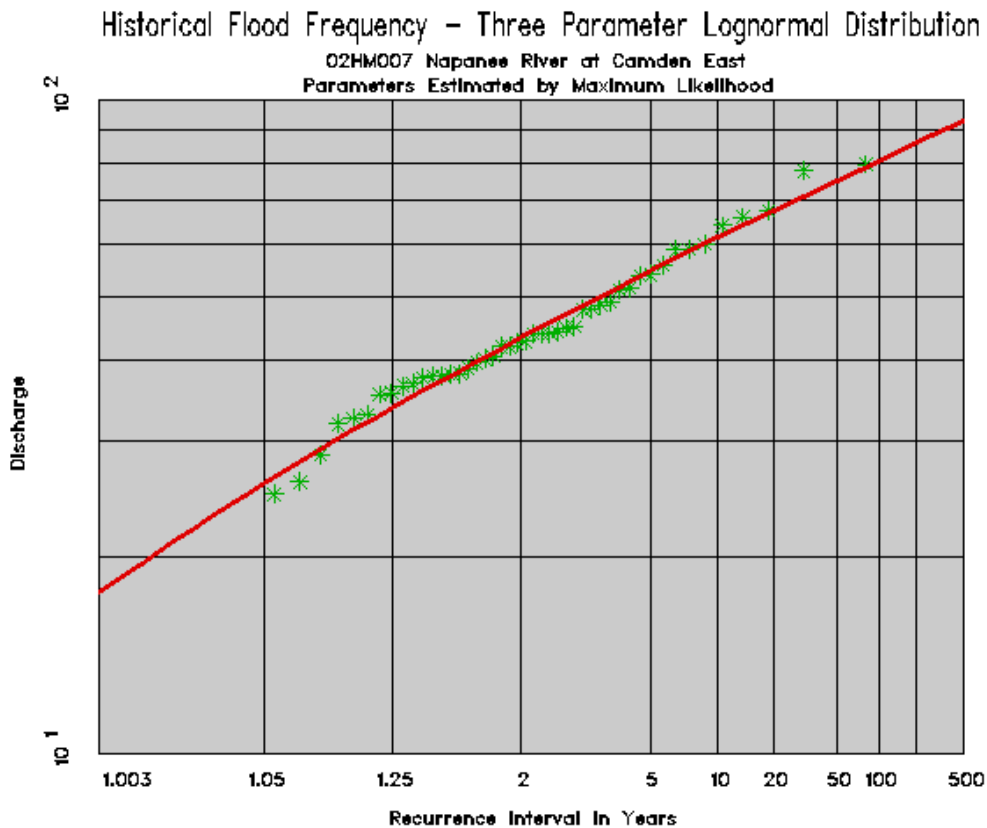
All Maximum Instantaneous Peaks (1974 – 2022):

SOLUTION OBTAINED VIA MAXIMUM LIKELIHOOD
 3LN PARAMETERS: A= -13.676 M= 4.041 S= .218

FLOOD FREQUENCY REGIME

RETURN PERIOD	EXCEEDANCE PROBABILITY	FLOOD
1.003	.997	17.6
1.050	.952	25.9
1.250	.800	33.7
2.000	.500	43.2
5.000	.200	54.7
10.000	.100	61.5
20.000	.050	67.7
50.000	.020	75.3
100.000	.010	80.7
200.000	.005	86.0
500.000	.002	92.8

Press <RETURN> to continue , <CTRL> P to obtain hard copy



Rainfall Only Maximum Instantaneous Peaks (May 1st to December 31st, 1974 – 2022):

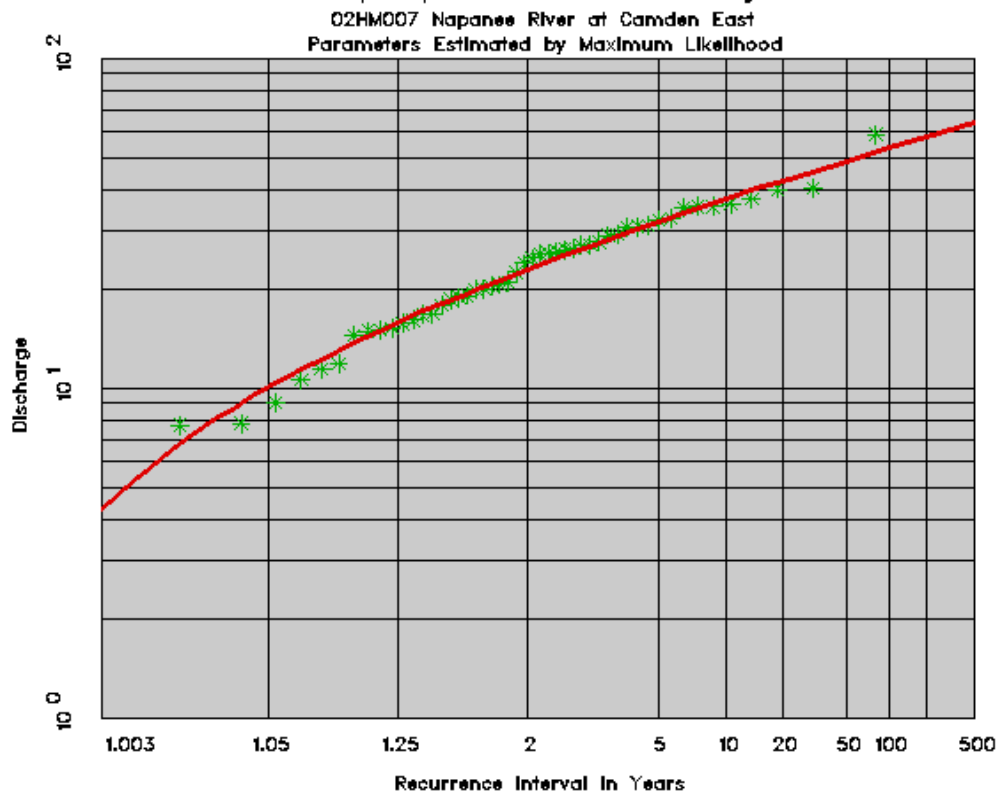
SOLUTION OBTAINED VIA MAXIMUM LIKELIHOOD
 3LN PARAMETERS: A= -13.713 M= 3.603 S= .260

FLOOD FREQUENCY REGIME

RETURN PERIOD	EXCEEDANCE PROBABILITY	FLOOD
1.003	.997	4.27
1.050	.952	10.1
1.250	.800	15.8
2.000	.500	23.0
5.000	.200	32.0
10.000	.100	37.5
20.000	.050	42.6
50.000	.020	48.9
100.000	.010	53.5
200.000	.005	58.0
500.000	.002	63.8

Press <RETURN> to continue , <CTRL> P to obtain hard copy

Historical Flood Frequency – Three Parameter Lognormal Distribution



Appendix H: Lake Outlet Locations



UPPER NAPANEE RIVER LAKE OUTLETS



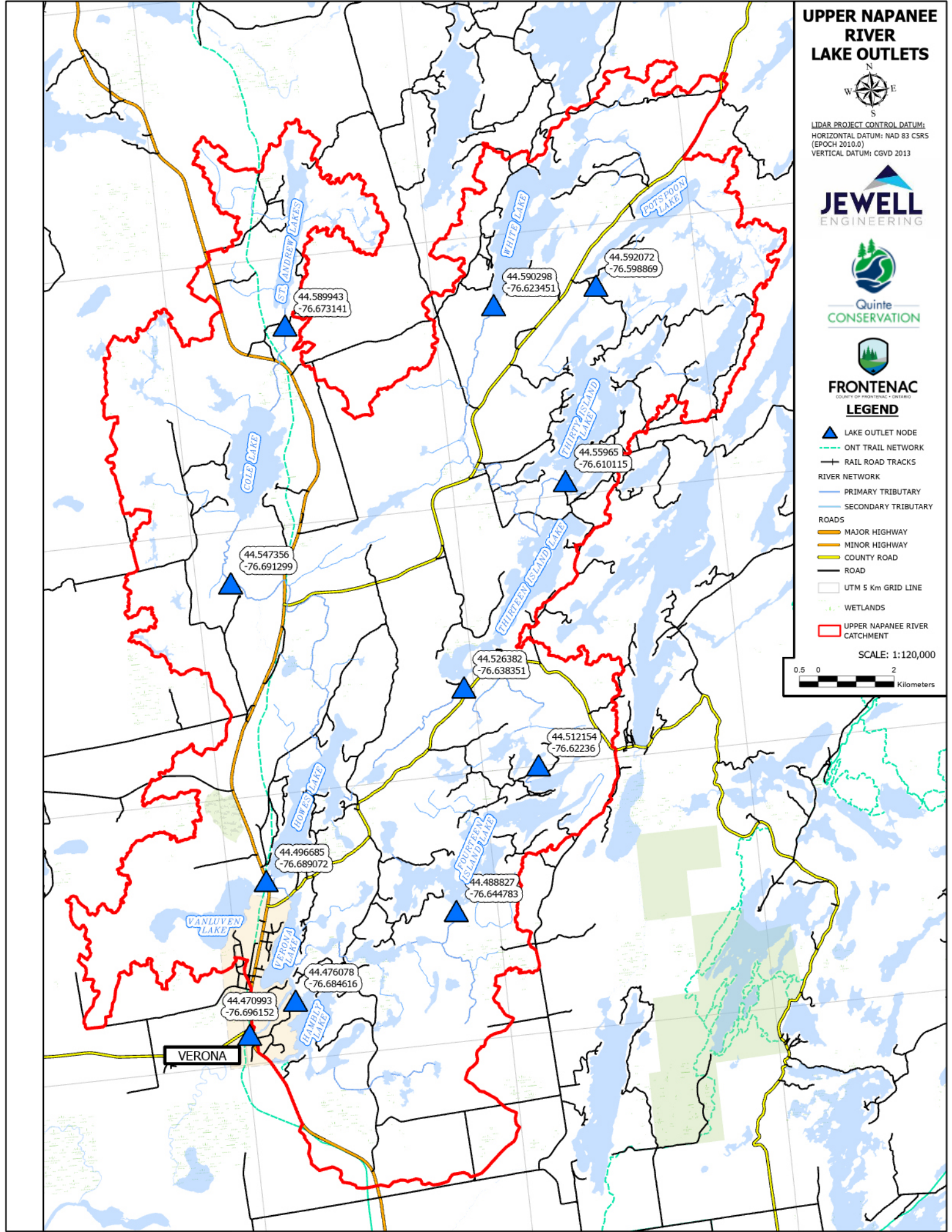
LIDAR PROJECT CONTROL DATUM:
 HORIZONTAL DATUM: NAD 83 CSRS
 (EPOCH 2010.0)
 VERTICAL DATUM: CGVD 2013



LEGEND

- LAKE OUTLET NODE
- ONT TRAIL NETWORK
- RAIL ROAD TRACKS
- RIVER NETWORK
- PRIMARY TRIBUTARY
- SECONDARY TRIBUTARY
- ROADS**
- MAJOR HIGHWAY
- MINOR HIGHWAY
- COUNTY ROAD
- ROAD
- UTM 5 Km GRID LINE
- WETLANDS
- UPPER NAPANEE RIVER CATCHMENT

SCALE: 1:120,000

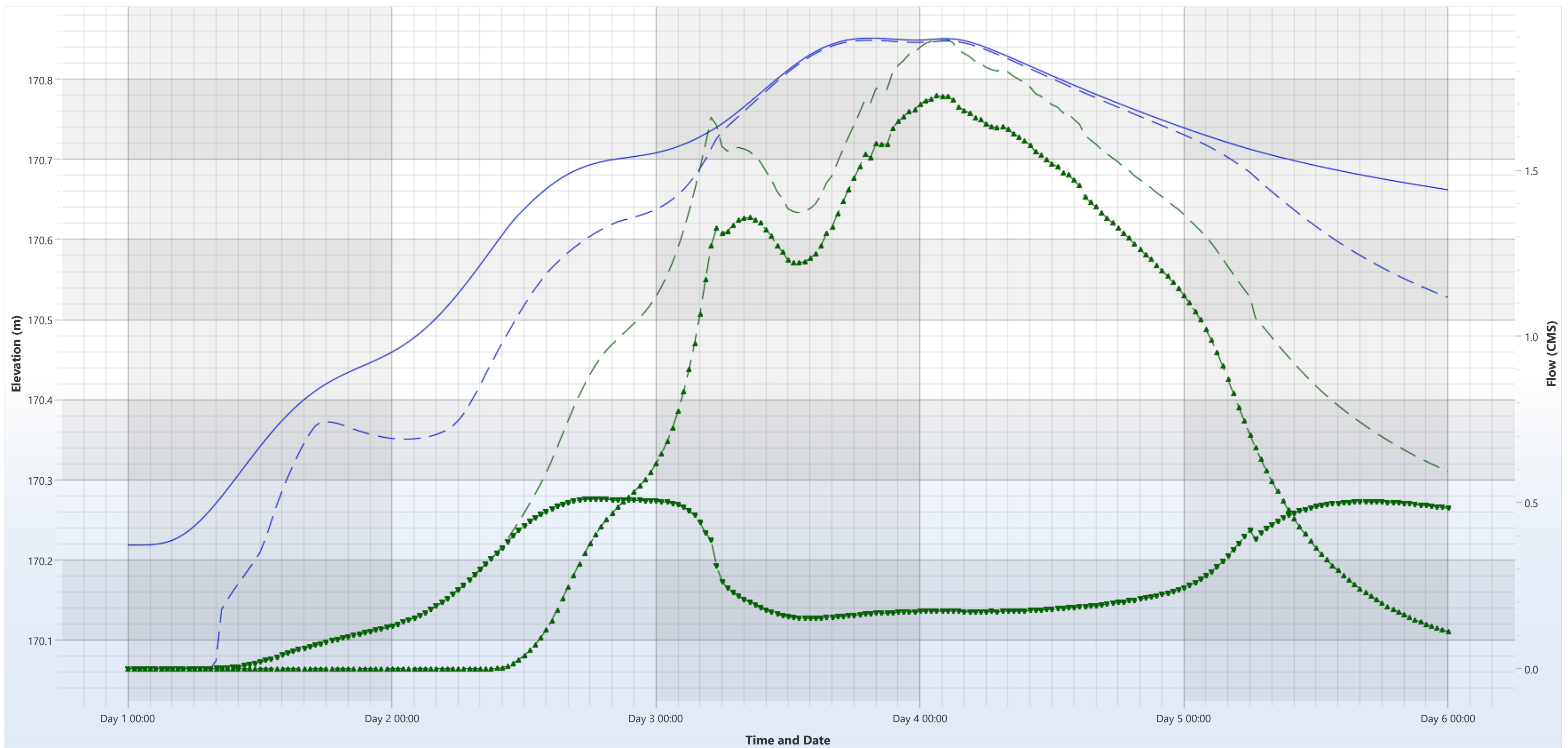


VERONA

Appendix I:
Culvert & Bridge Stage-Flow Hydrographs



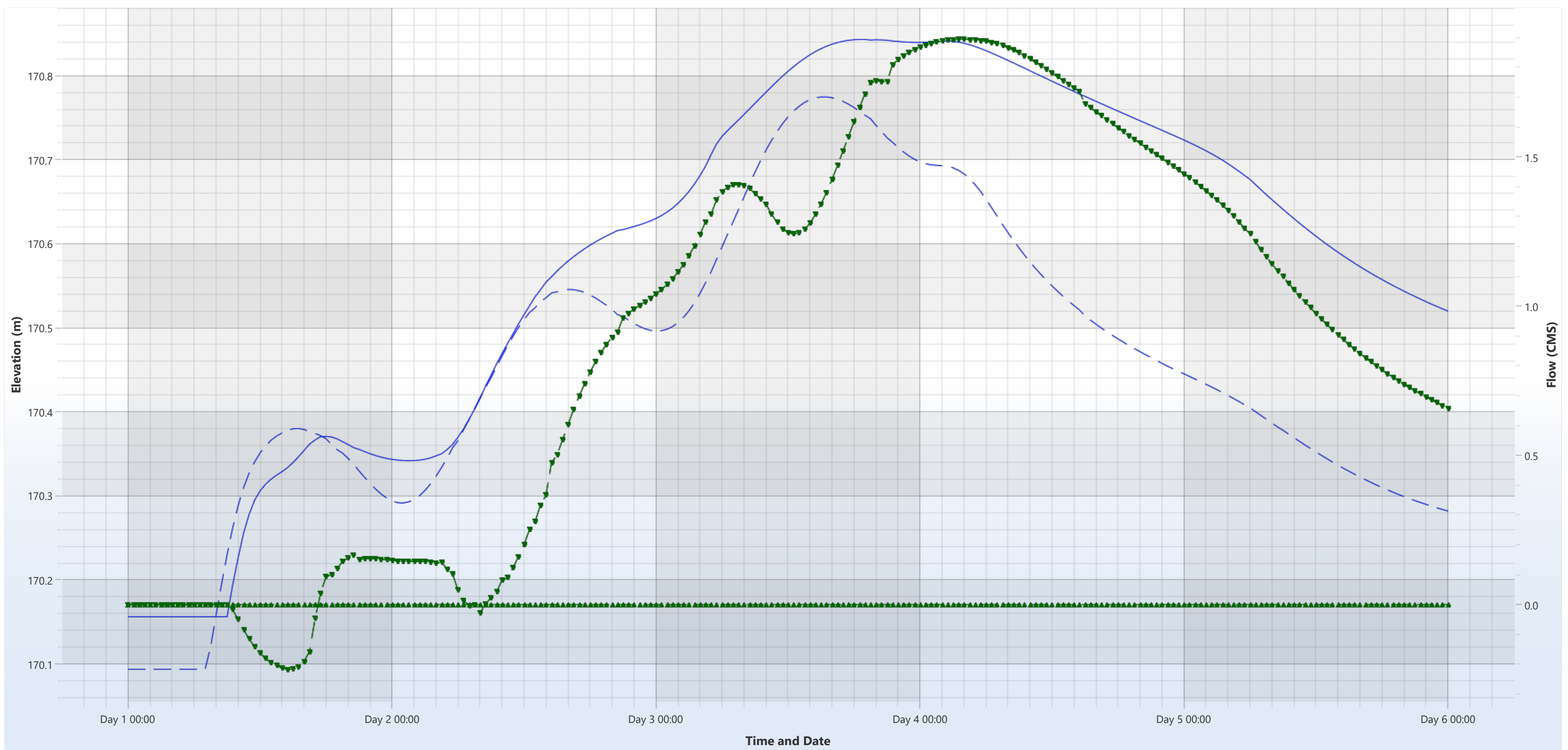
1% AEP Spring Melt - Crossing 1 - St Andrew Lakes Lane



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

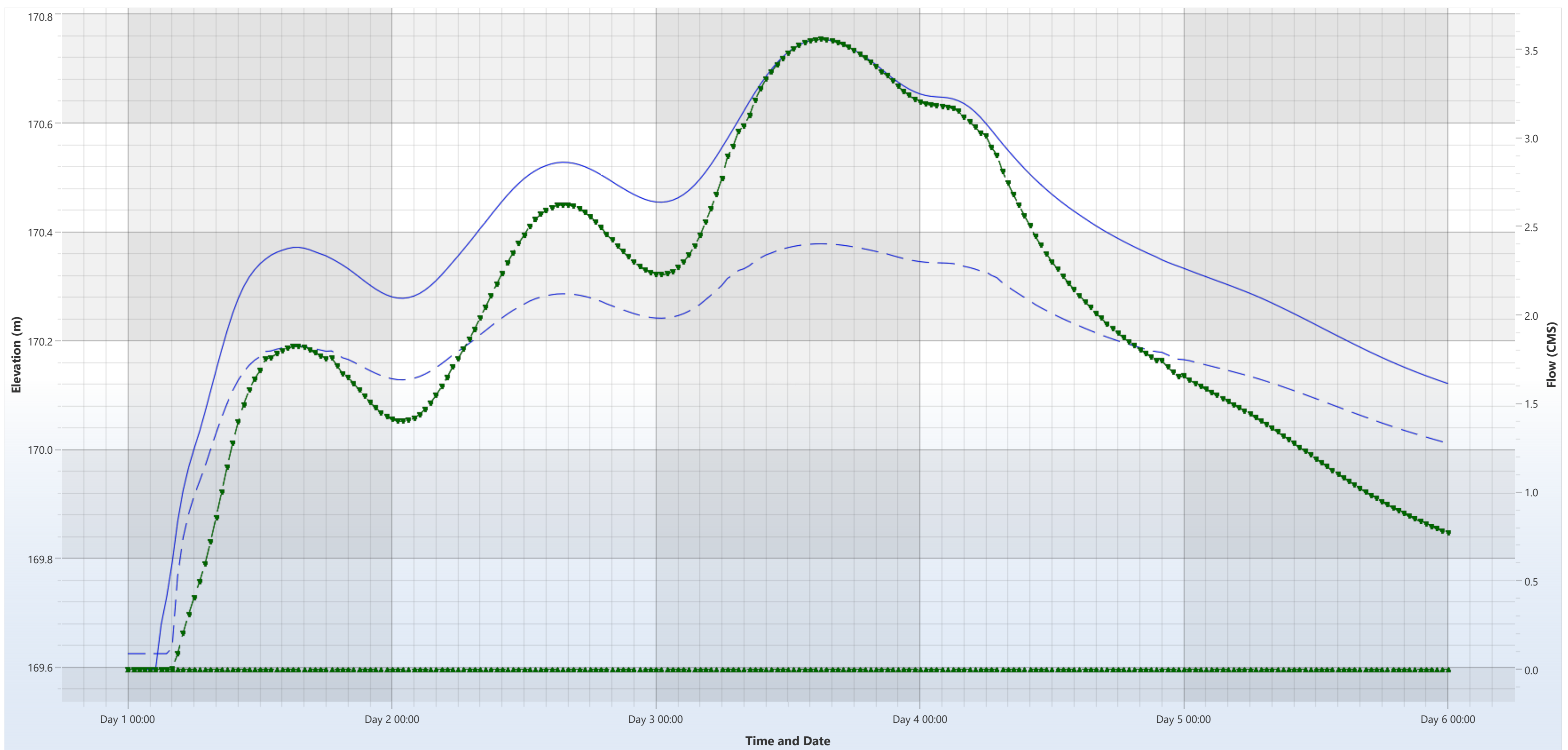
1% AEP Spring Melt - Crossing 2 - K&P Trail



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

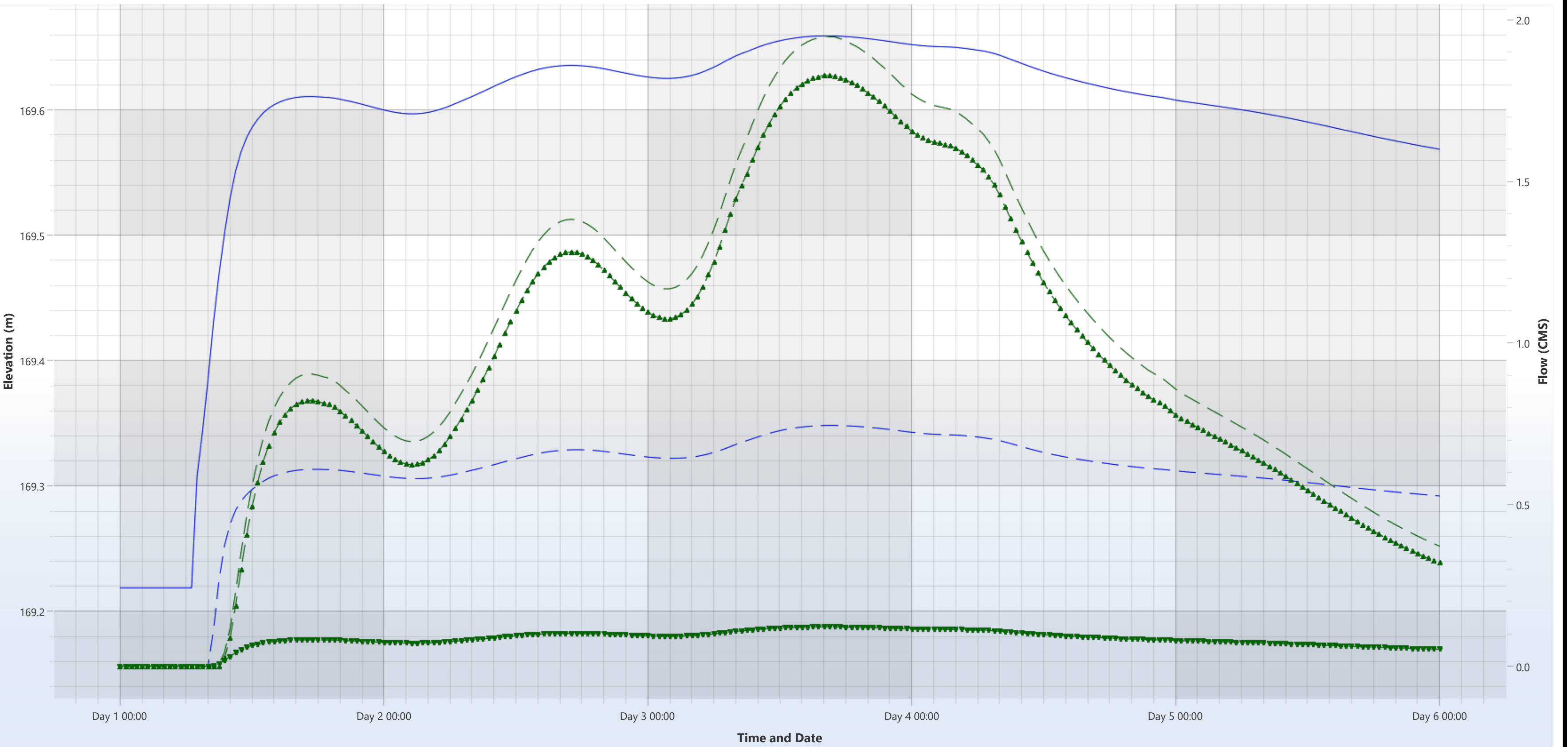
1% AEP Spring Melt - Crossing 3 - HWY 38 Near Cole Lake



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

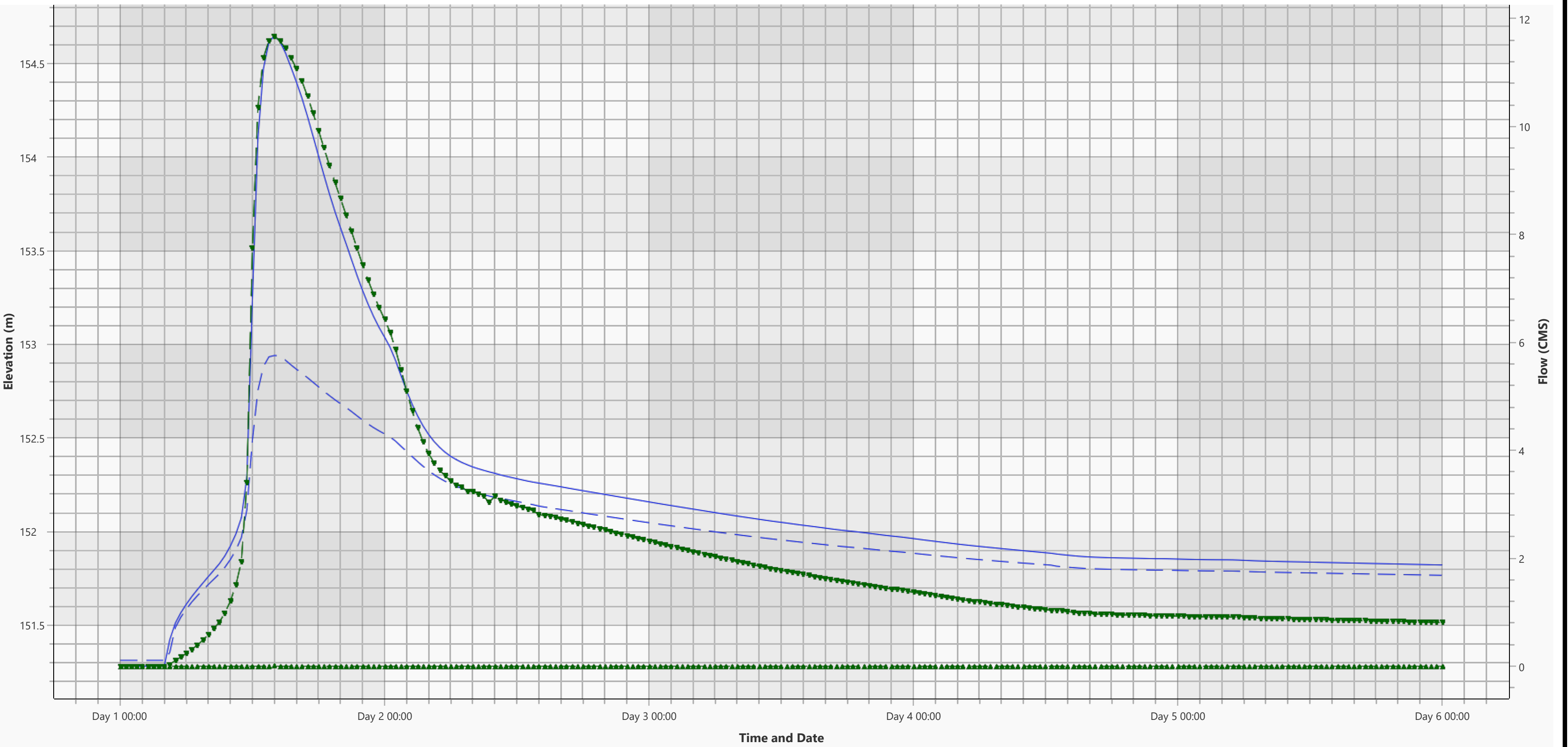
1% AEP Spring Melt - Crossing 4 - Private Crossing, 8985 Hwy 38



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

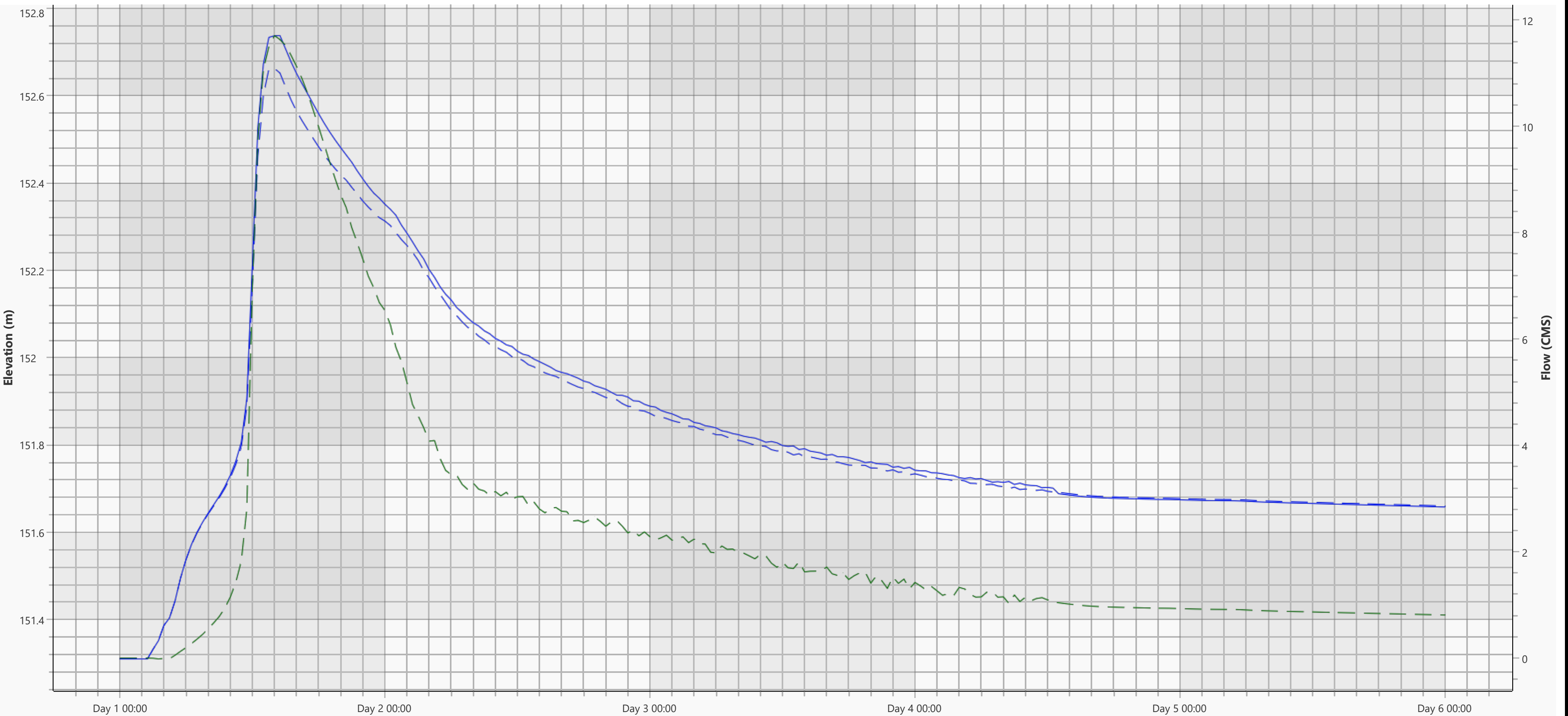
1% AEP Rainfall - Crossing 5 - HWY 38 Near Godfrey



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

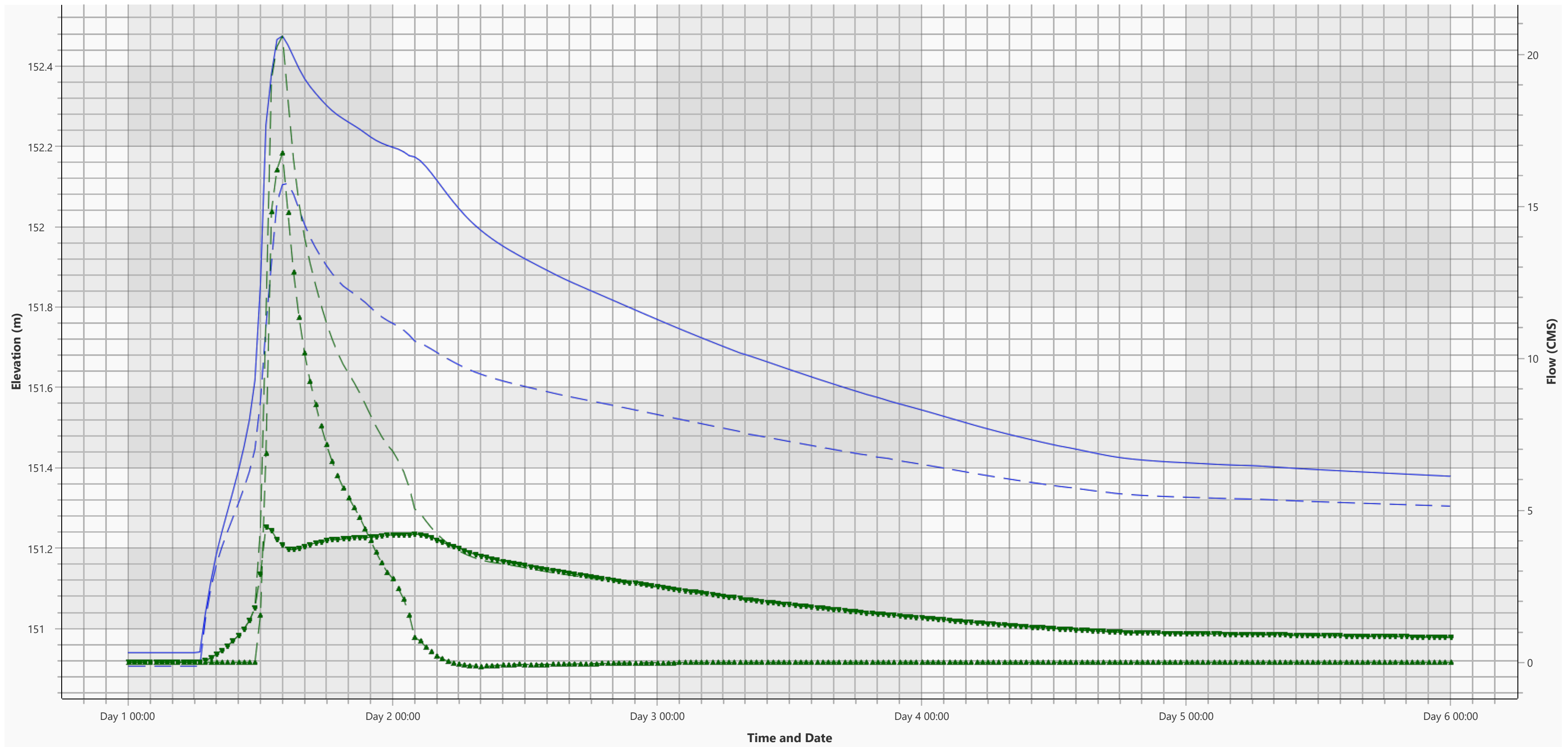
1% AEP Rainfall - Crossing 6 - K&P Trail Near Godfrey



Legend

- Stage HW
- Stage TW
- Flow

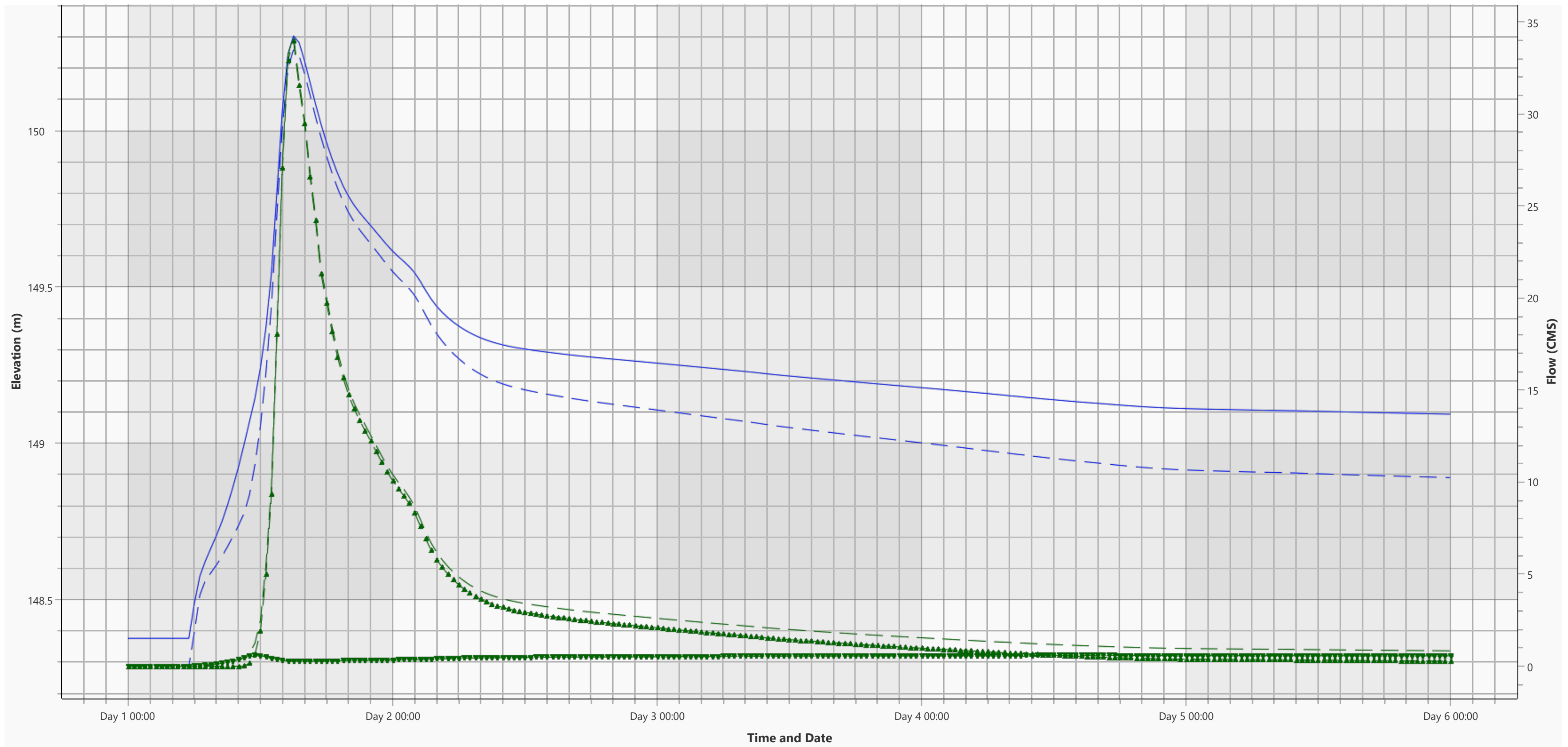
1% AEP Rainfall - Crossing 7 - Westport Rd Near Godfrey



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

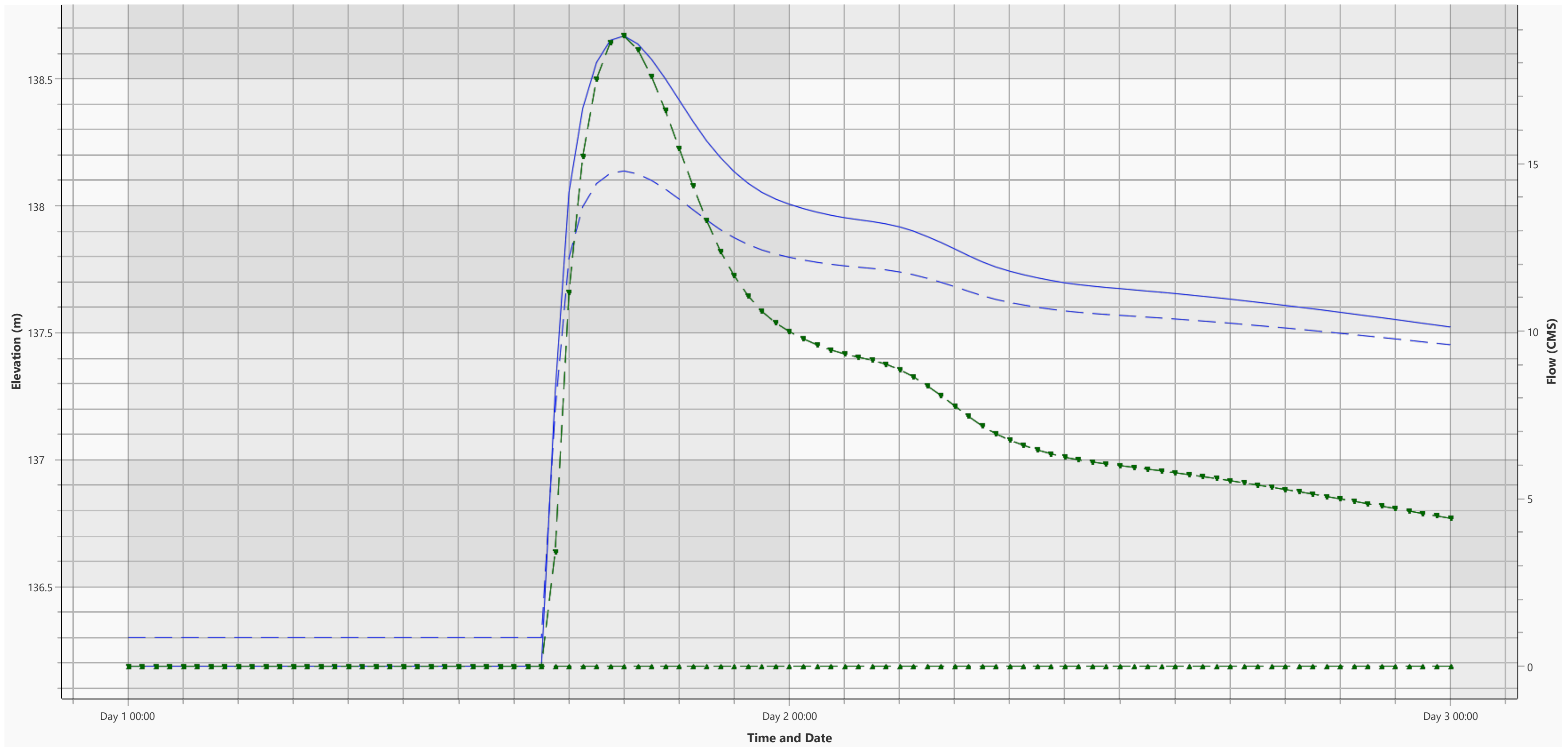
1% AEP Rainfall - Crossing 8 - Private Crossing, 7297 Hinchinbrooke Rd N



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

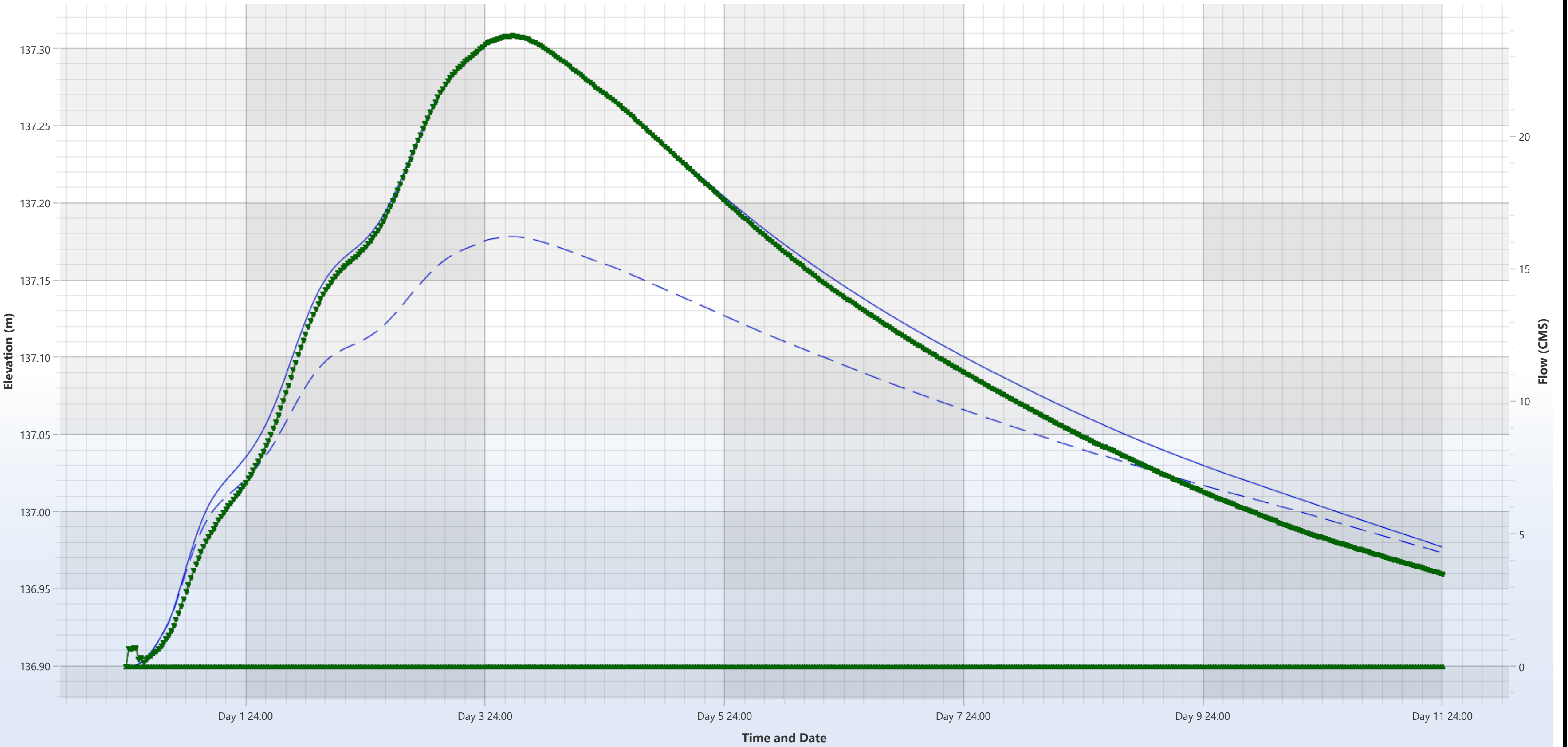
1% AEP Rainfall - Crossing 9 - Craig Rd



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

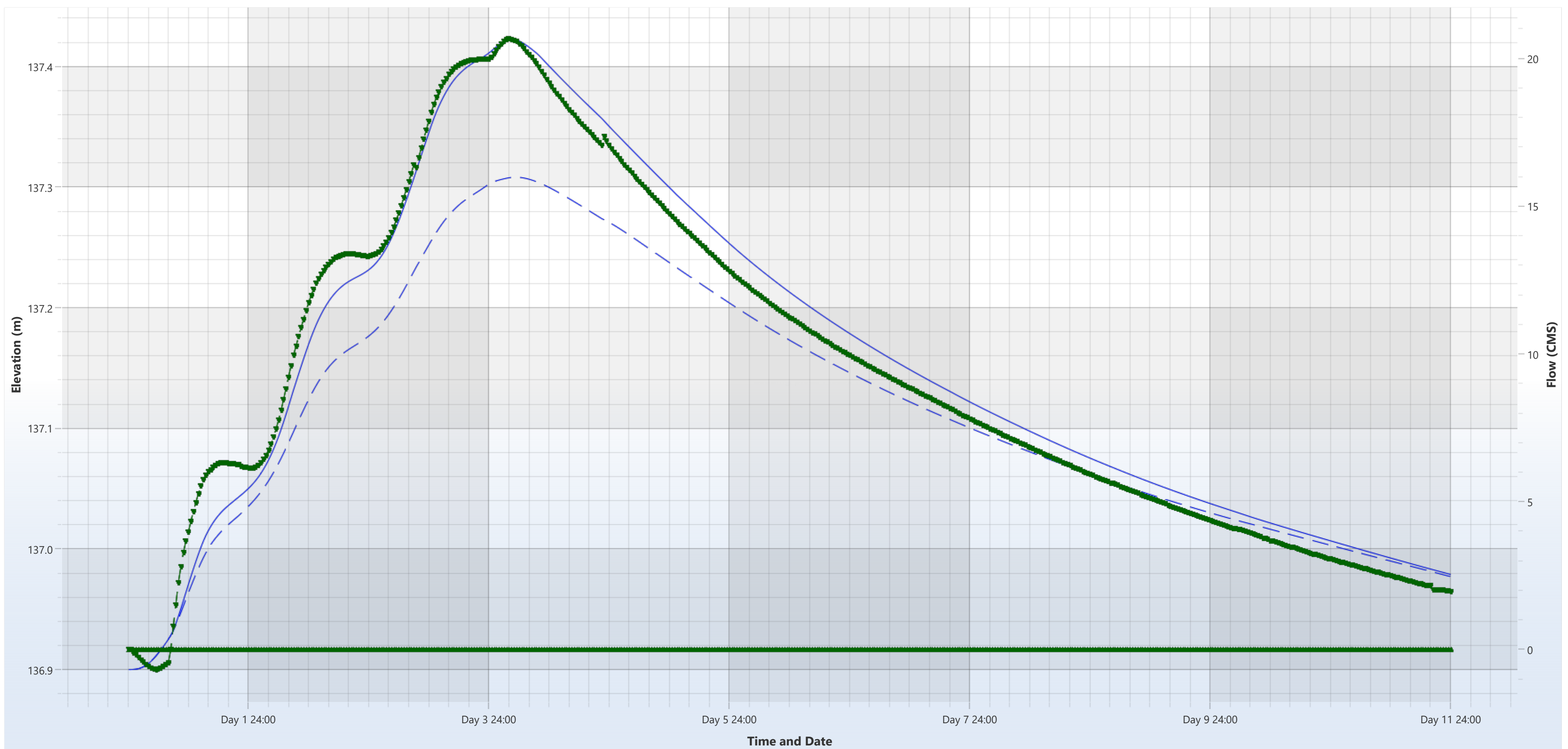
1% AEP Spring Melt - Crossing 10 - HWY 38 Near Howes Lake



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

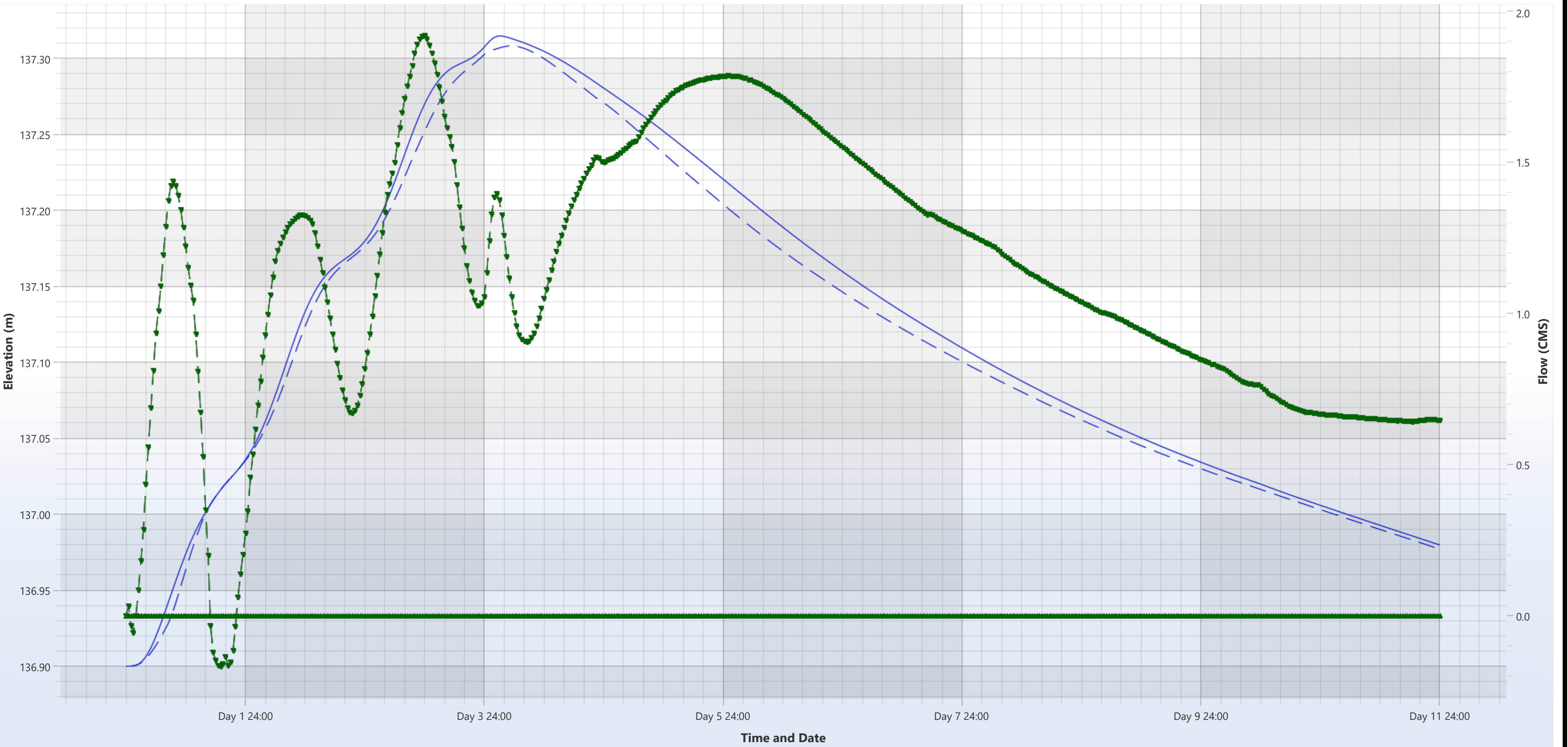
1% AEP Spring Melt - Crossing 11 - Desert Lk Rd



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

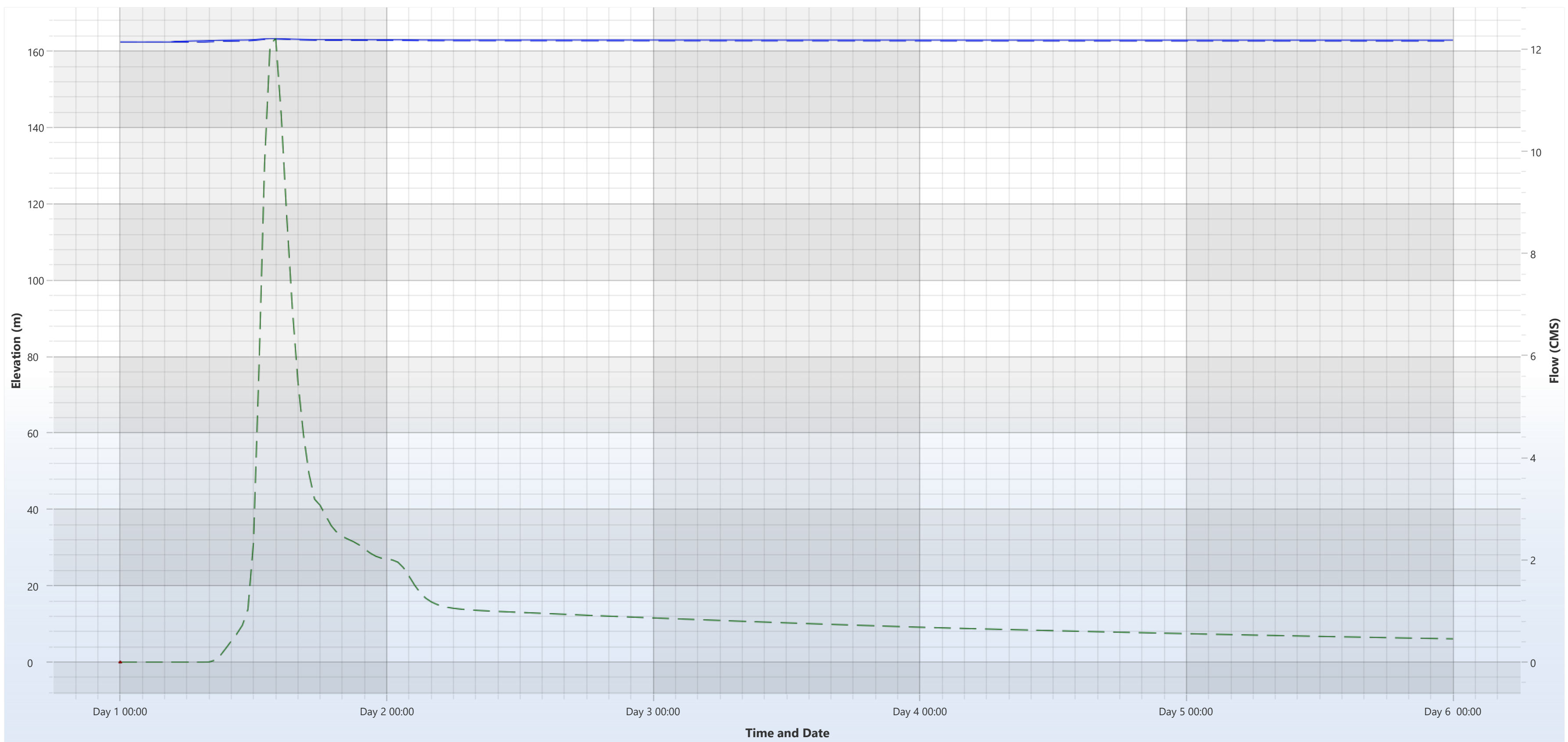
1% AEP Spring Melt - Crossing 12 - Cedarwoods Ln



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

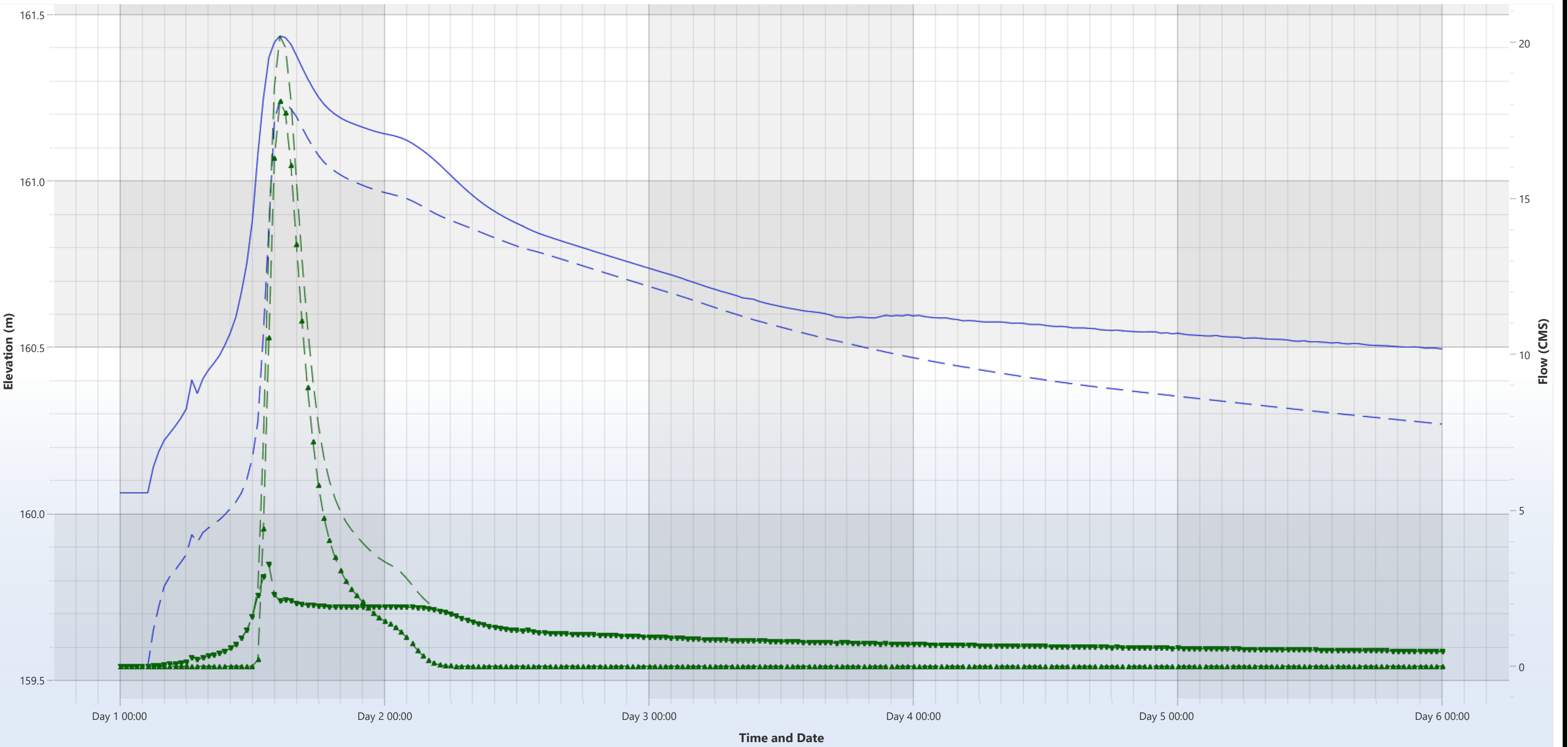
1% AEP Rainfall - Crossing 13 - Buck Bay Rd



Legend

- Stage HW
- Stage TW
- Flow

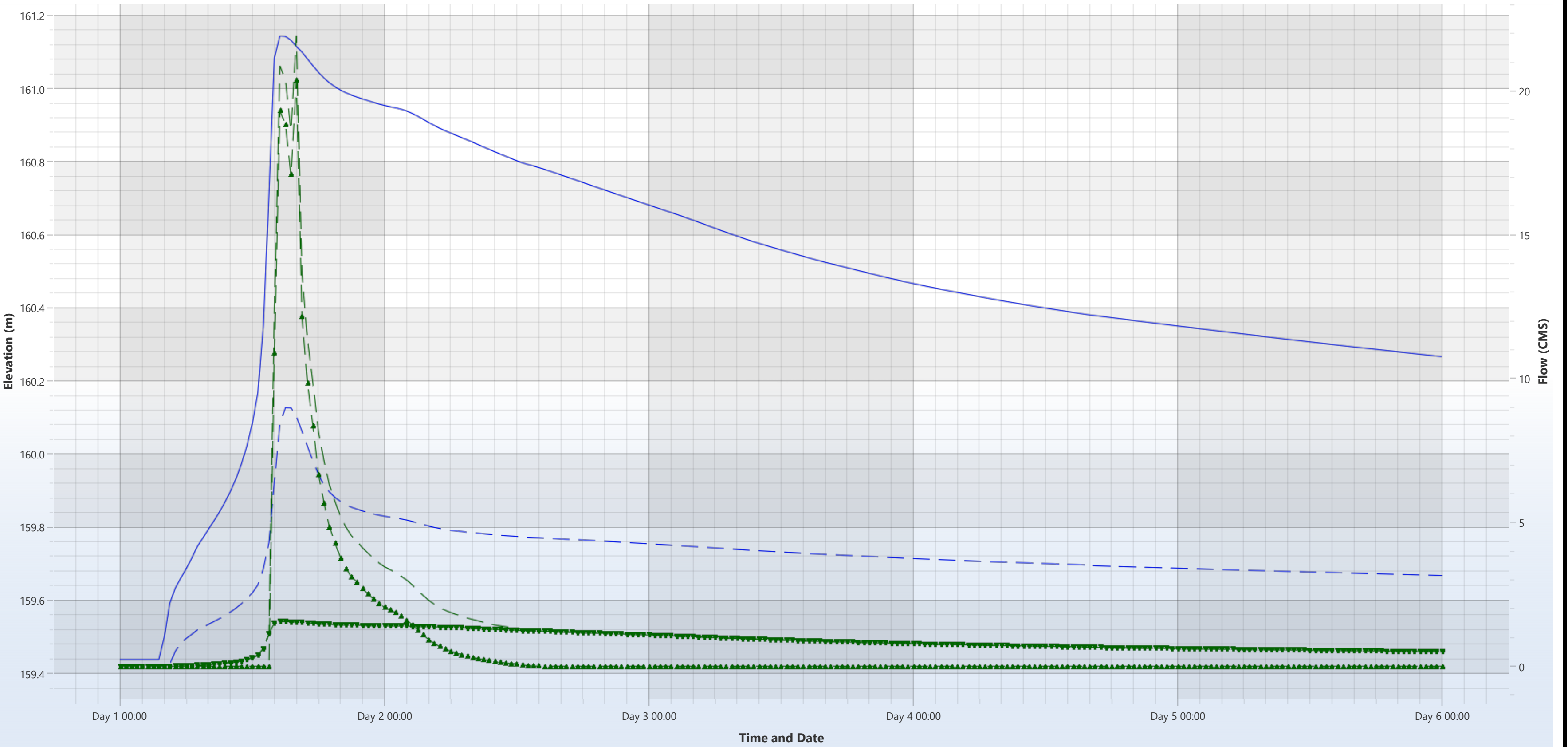
1% AEP Rainfall - Crossing 14 - Private Crossing



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

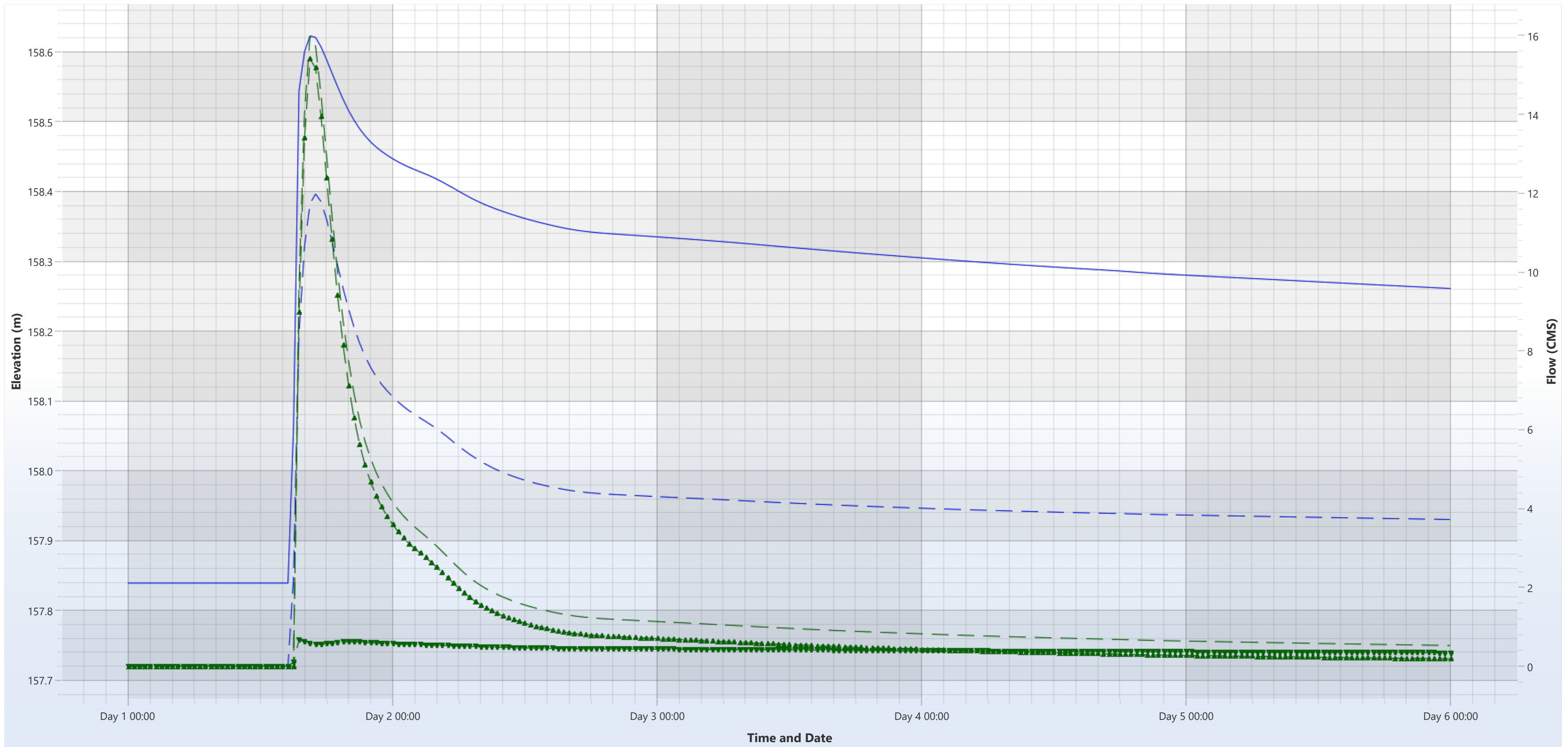
1% AEP Rainfall - Crossing 15 - Westport Rd Near Glendower



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

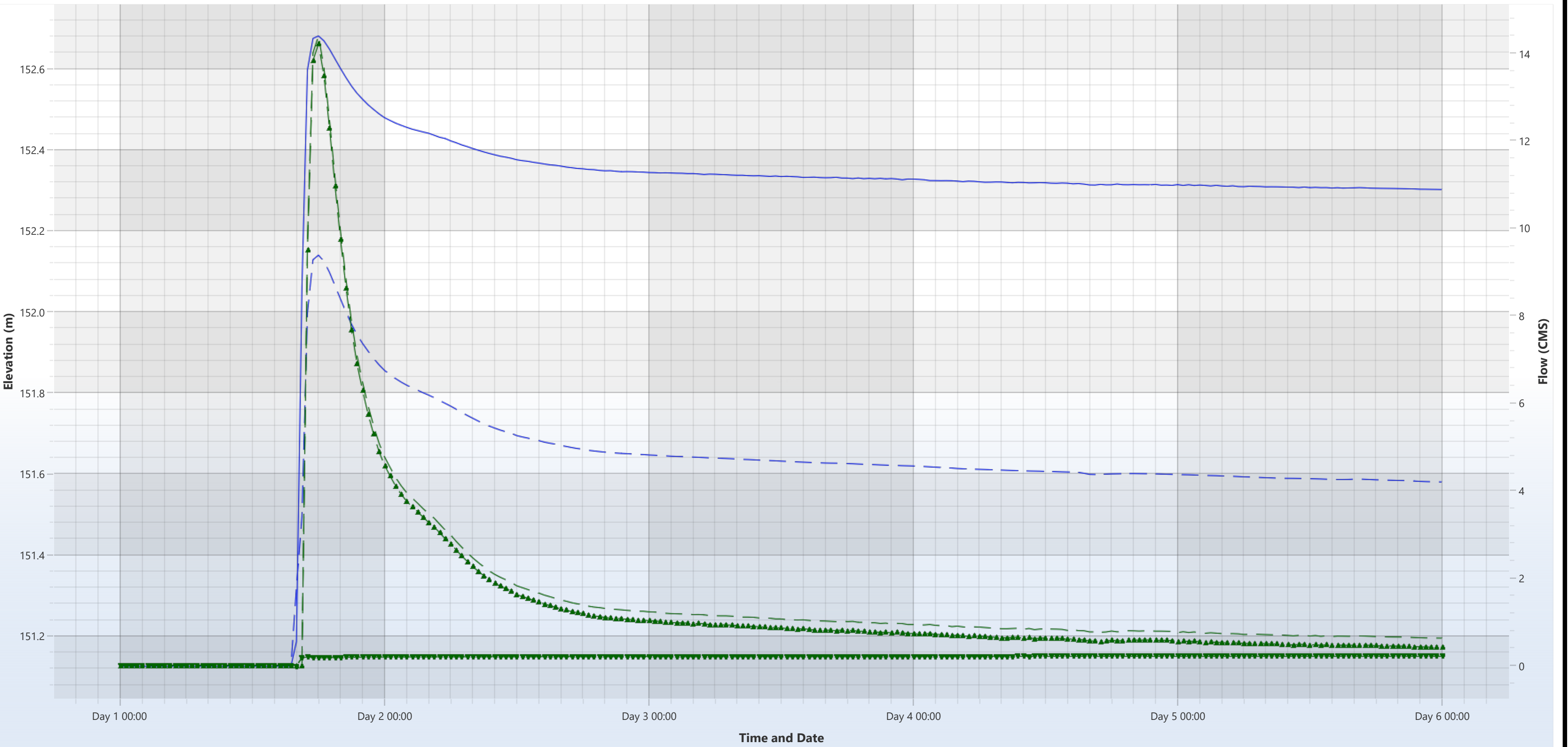
1% AEP Rainfall - Crossing 16 - Brooks Ln



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

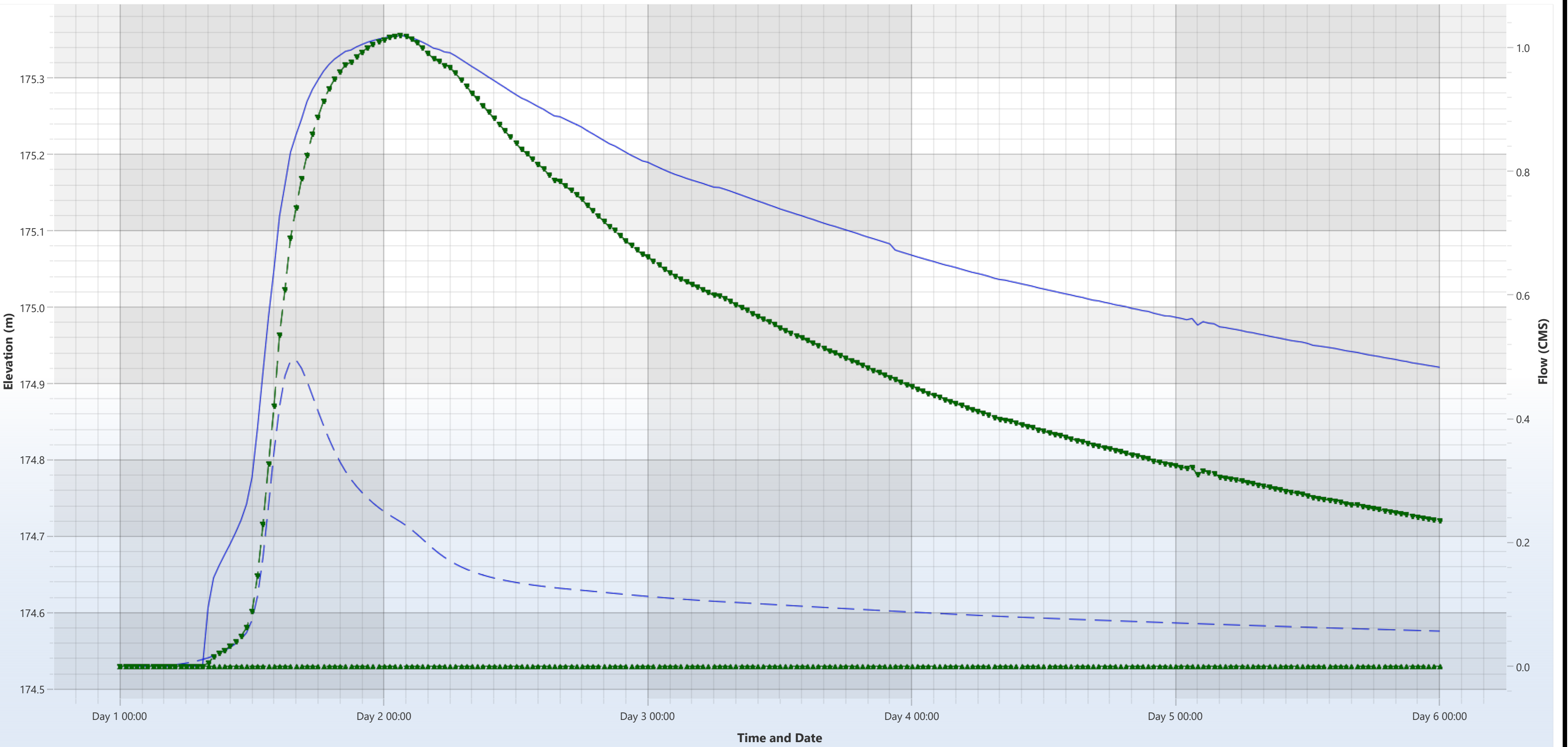
1% AEP Rainfall - Crossing 17 - Bunker Hill Rd A



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

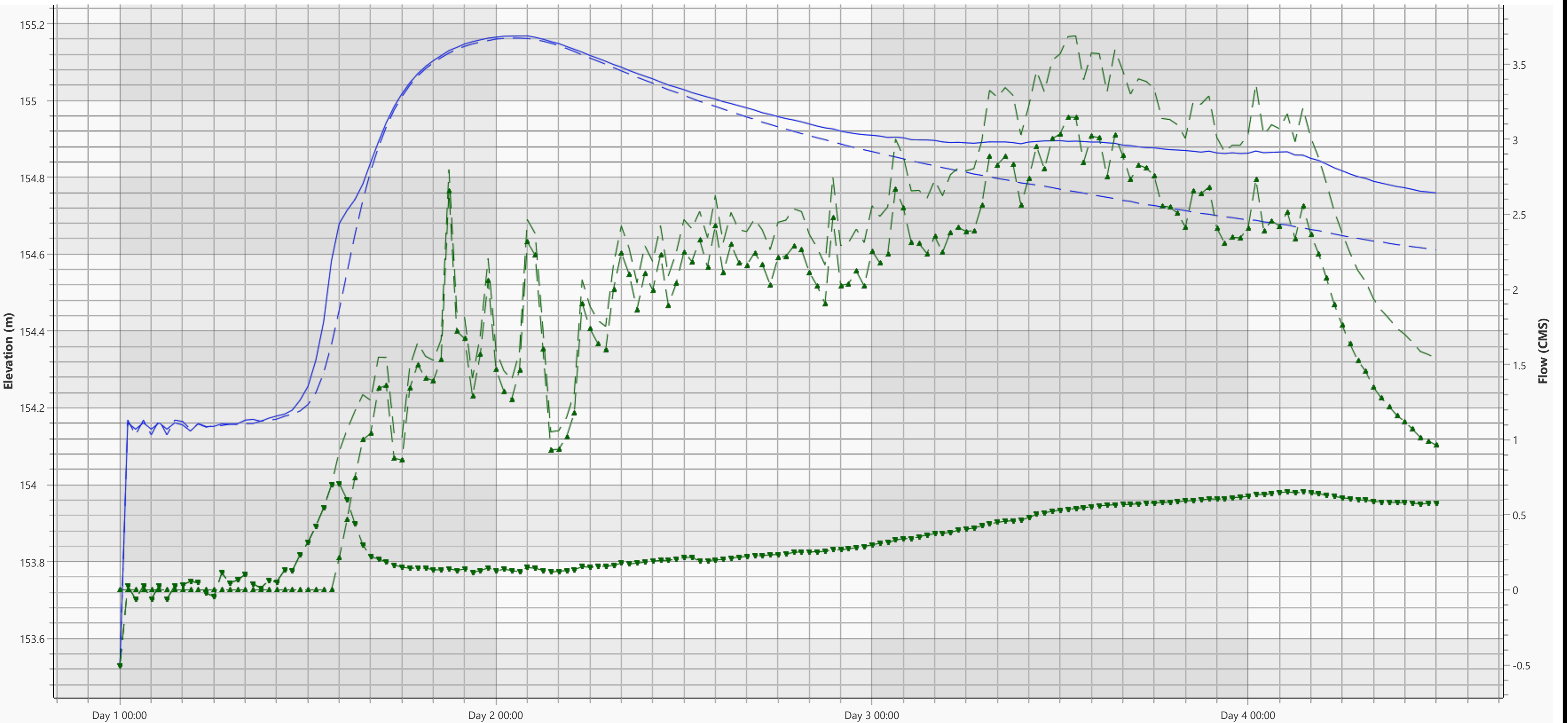
1% AEP Rainfall - Crossing 18 - Sperling Rd



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

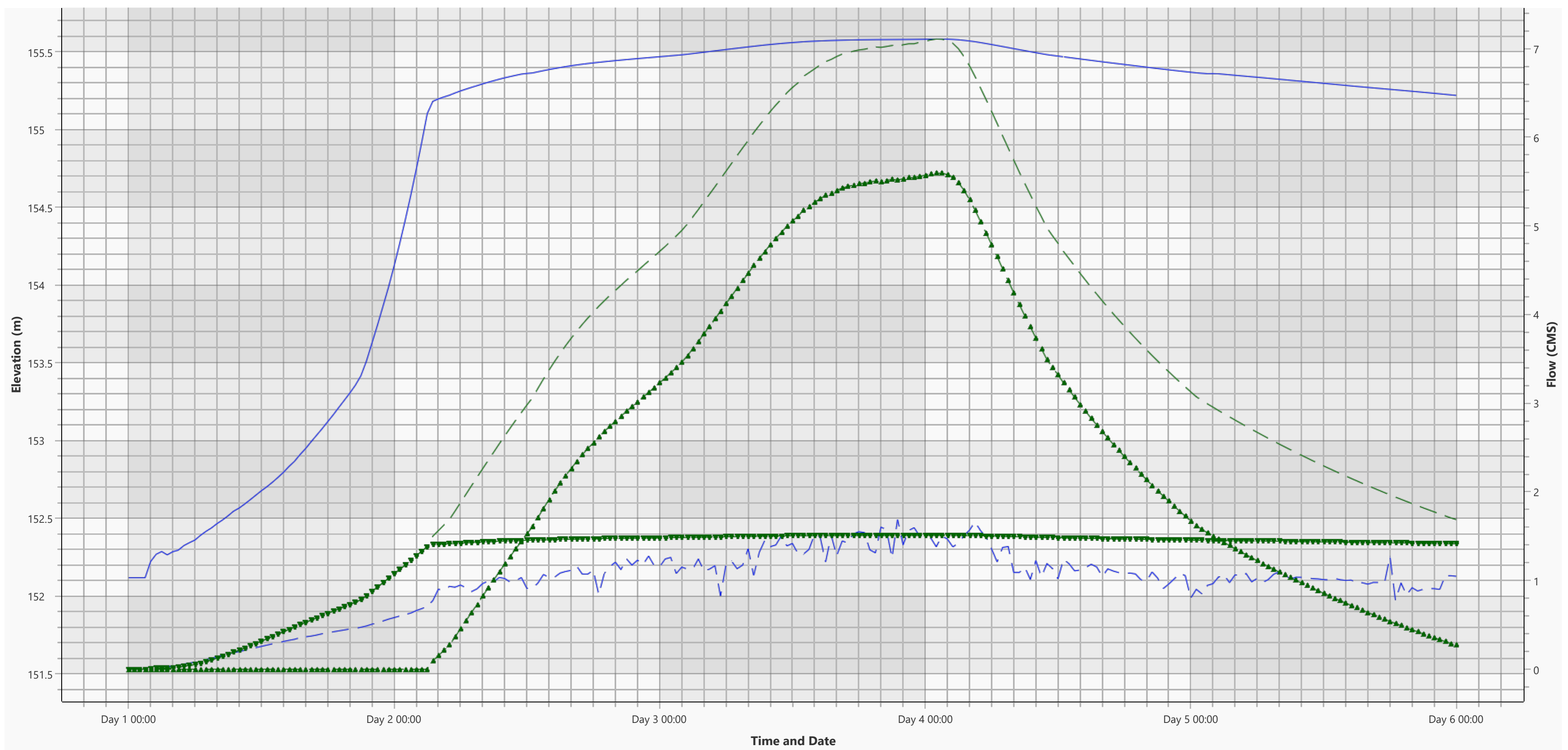
1% AEP Spring Melt - Crossing 19 - McColl Ln



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

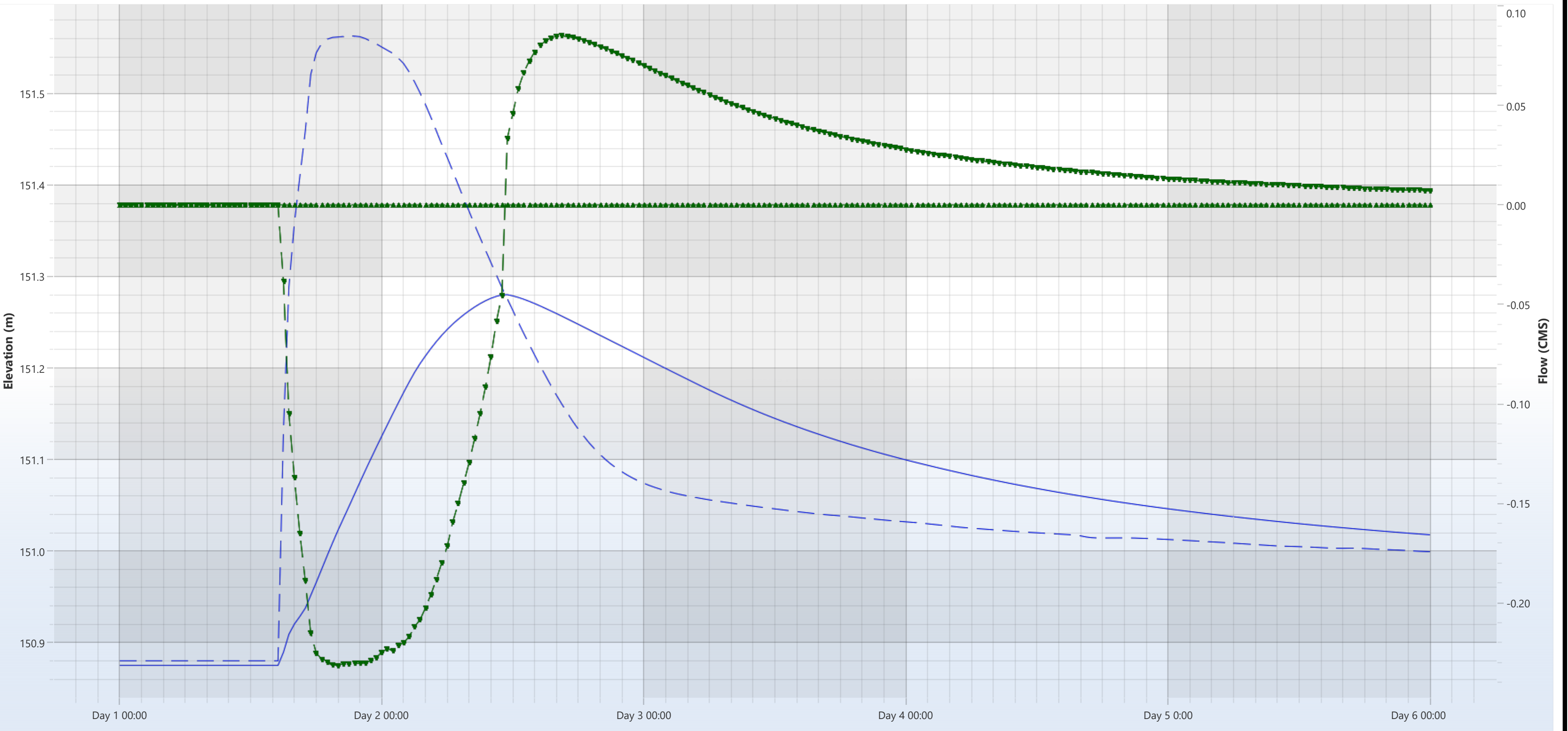
1% AEP Spring Melt - Crossing 20 - Private Crossing



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

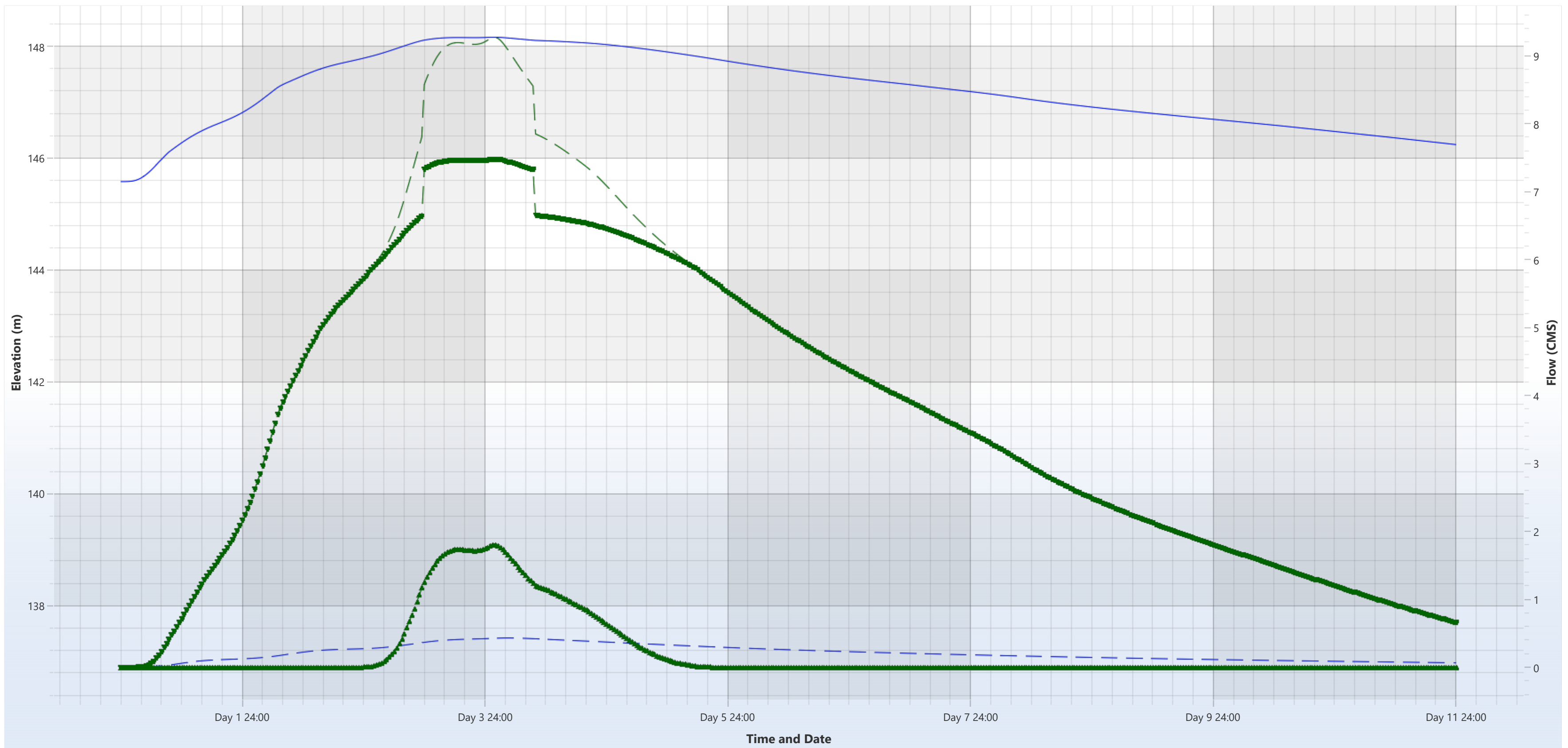
1% AEP Rainfall - Crossing 21 - Bunker Hill Rd B



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

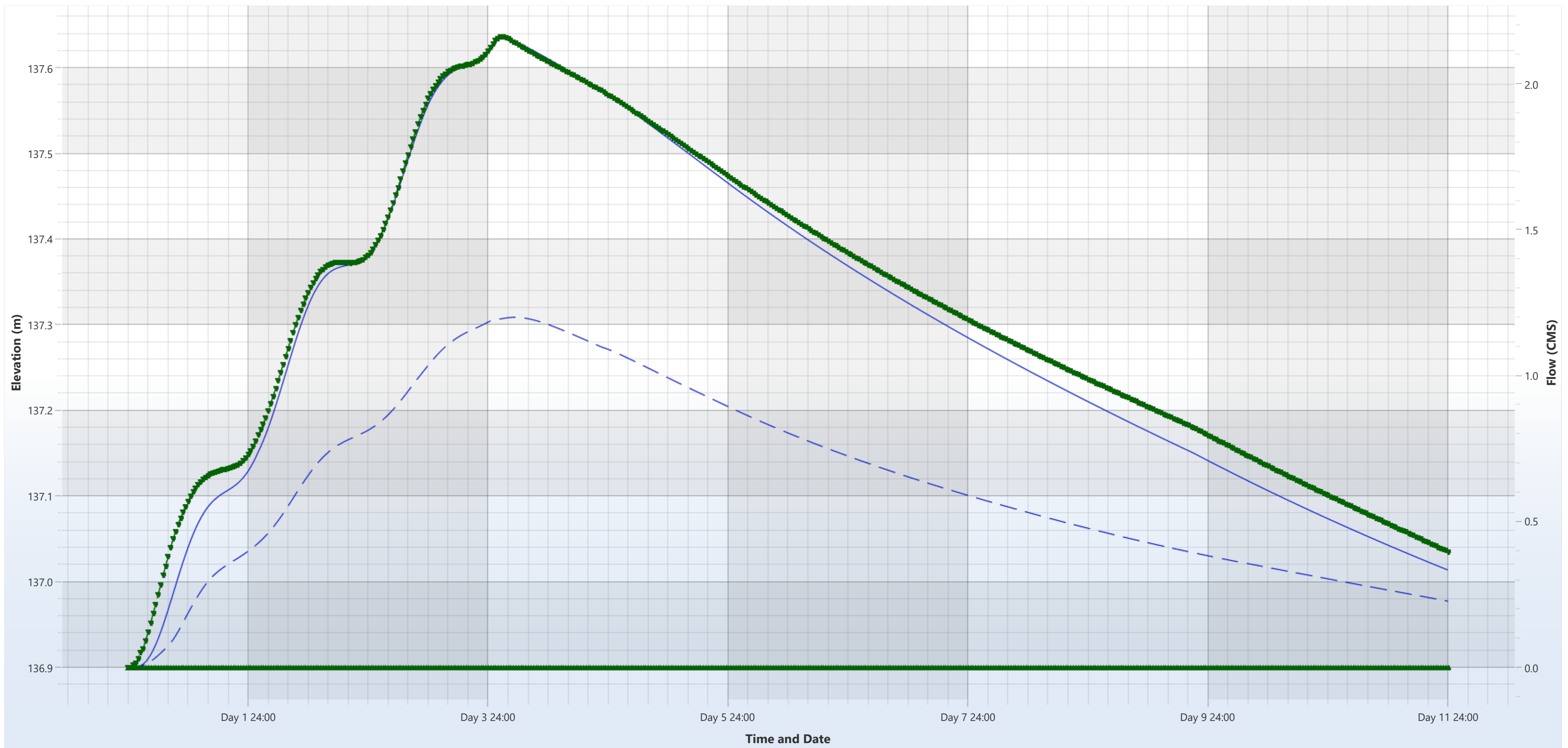
1% AEP Spring Melt - Crossing 23 - Hinchinbrooke Rd N



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

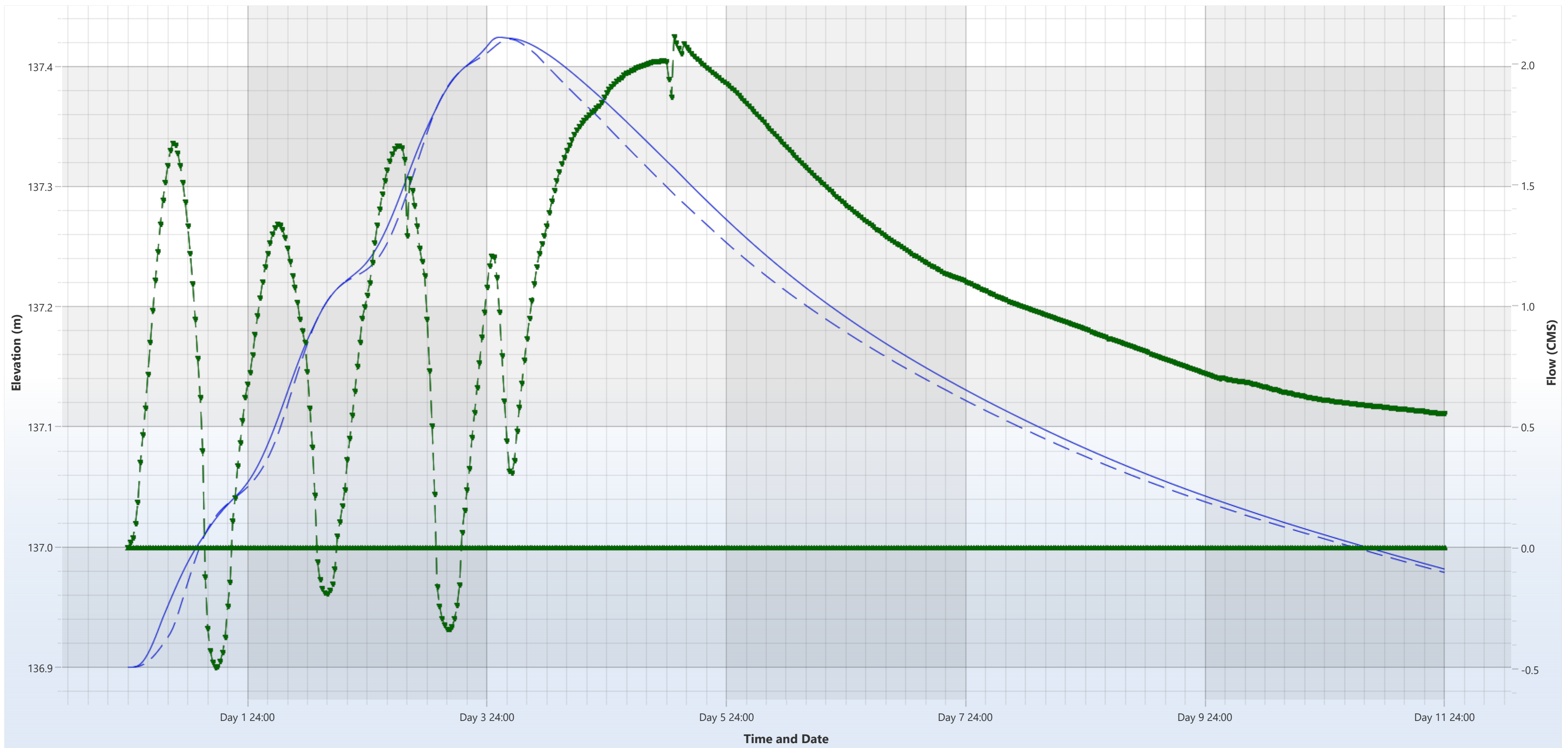
1% AEP Spring Melt - Crossing 24 - Hinchinbrooke Rd



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

1% AEP Springmelt - Crossing 25 - HWY 38 Near Verona



Legend

- Stage HW
- Stage TW
- Total Flow
- Weir Flow
- Total Culvert Flow

Appendix J: Culvert & Bridge Data Sheets



CULVERT/BRIDGE DATA

Structure ID	<u>1</u>
Location	<u>St. Andrews Lake Lane</u>
UTM Coordinates	<u>366964, 4938763</u>
Type	<u>Three HDPE Culverts</u>
Conversion to CGVD28	<u>+0.33</u>

A. Specifications

Top of Road Elevation at Structure	<u>171.25</u>	m
Invert	<u>170.20</u>	m
Number of Openings	<u>3</u>	
Low Point of Road	<u>170.59</u>	m
Governing 1% AEP Event	<u>Spring melt</u>	
Governing 1% AEP Q_{peak}	<u>1.90</u>	m^3/s
HW_{peak}	<u>170.85</u>	m
TW_{peak}	<u>170.85</u>	m
ΔH_{peak}	<u>0.00</u>	m
Max. Relief Flow Depth	<u>0.26</u>	m

B. Photographic Presentation**C. Benchmark**

Description	<u>Swamp Bottom</u>
Coordinates	<u>366954.94, 4938763.24</u>
Elevation	<u>170.42</u>

CULVERT/BRIDGE DATA

Structure ID	2
Location	K&P Trail near Cole Lake
UTM Coordinates	366876, 49338489
Type	Concrete Box Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	171.25	m
Invert	168.85	m
Number of Openings	1	
Low Point of Road	171.13	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	1.90	m^3/s
HW_{peak}	170.84	m
TW_{peak}	170.77	m
ΔH_{peak}	0.07	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation**C. Benchmark**

Description	Obvert of Culvert
Coordinates	366695.91, 4933879.69
Elevation	168.3

CULVERT/BRIDGE DATA

Structure ID	<u>3</u>
Location	<u>Highway 38 near Cole Lake</u>
UTM Coordinates	<u>367020, 4938050</u>
Type	<u>Concrete Box Culvert</u>
Conversion to CGVD28	<u>+0.33</u>

A. Specifications

Top of Road Elevation at Structure	<u>171.26</u>	m
Invert	<u>168.28</u>	m
Number of Openings	<u>1</u>	
Low Point of Road	<u>170.99</u>	m
Governing 1% AEP Event	<u>Spring melt</u>	
Governing 1% AEP Q_{peak}	<u>3.60</u>	m^3/s
HW_{peak}	<u>170.75</u>	m
TW_{peak}	<u>170.38</u>	m
ΔH_{peak}	<u>0.37</u>	m
Max. Relief Flow Depth	<u>0.00</u>	m

B. Photographic Presentation**C. Benchmark**

Description	<u>Obvert of Box Culvert</u>
Coordinates	<u>367022.21, 4938063.20</u>
Elevation	<u>171.02</u>

CULVERT/BRIDGE DATA

Structure ID	5
Location	Highway 38 near Godfrey
UTM Coordinates	366665, 4933987
Type	Concrete Box Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	154.73	m
Invert	150.30	m
Number of Openings	1	
Low Point of Road	153.52	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	11.70	m^3/s
HW_{peak}	154.65	m
TW_{peak}	152.94	m
ΔH_{peak}	1.71	m
Max. Relief Flow Depth	1.13	m

B. Photographic Presentation



C. Benchmark

Description	South East Soffit
Coordinates	366666.36, 4933989.72
Elevation	152.36

CULVERT/BRIDGE DATA

Structure ID	6
Location	K&P Trail N
UTM Coordinates	366701, 4933876
Type	Bridge
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	154.39	m
Invert	150.06	m
Number of Openings	1	
Low Point of Road	154.28	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	11.70	m^3/s
HW_{peak}	152.74	m
TW_{peak}	152.66	m
ΔH_{peak}	0.08	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation**C. Benchmark**

Description	top of conc culvert opening
Coordinates	366878.24, 4938488.39
Elevation	151.73

CULVERT/BRIDGE DATA

Structure ID	7
Location	Westport Road near Godfrey
UTM Coordinates	366673, 4933510
Type	CSP Arch Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	152.40	m
Invert	149.60	m
Number of Openings	1	
Low Point of Road	151.66	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	20.60	m ³ /s
HW_{peak}	152.47	m
TW_{peak}	152.11	m
ΔH_{peak}	0.36	m
Max. Relief Flow Depth	0.81	m

B. Photographic Presentation**C. Benchmark**

Description	Obvert of Culvert, north side
Coordinates	366669.13, 4933513.28
Elevation	152.15

CULVERT/BRIDGE DATA

Structure ID	9
Location	Craig Road
UTM Coordinates	366850, 4930999
Type	CSP Arch Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	139.32	m
Invert	135.64	m
Number of Openings	1	
Low Point of Road	139.31	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	18.80	m^3/s
HW_{peak}	138.67	m
TW_{peak}	138.14	m
ΔH_{peak}	0.53	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation



C. Benchmark

Description	Obvert of 4400 CSP
Coordinates	366855.43, 4930996.56
Elevation	138.51

CULVERT/BRIDGE DATA

Structure ID	10
Location	Highway 38 near Howes Lake
UTM Coordinates	365712, 4928432
Type	Concrete Box Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	138.65	m
Invert	138.11	m
Number of Openings	1	
Low Point of Road	138.22	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	23.80	m^3/s
HW_{peak}	137.31	m
TW_{peak}	137.18	m
ΔH_{peak}	0.13	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation



C. Benchmark

Description	Top of conc. box culvert
Coordinates	365721.54, 4928405
Elevation	137.72

CULVERT/BRIDGE DATA

Structure ID	11
Location	Desert Lake Road
UTM Coordinates	366103, 4928059
Type	Bridge
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	139.25	m
Invert	135.50	m
Number of Openings	1	
Low Point of Road	139.16	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	20.70	m^3/s
HW_{peak}	137.42	m
TW_{peak}	137.31	m
ΔH_{peak}	0.11	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation



C. Benchmark

Description	CL of Road
Coordinates	366105.11, 4928059.47
Elevation	139.5

CULVERT/BRIDGE DATA

Structure ID	12
Location	Cedarwoods Drive
UTM Coordinates	366021, 4926137
Type	Concrete Box Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	137.59	m
Invert	135.63	m
Number of Openings	1	
Low Point of Road	137.20	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	1.90	m ³ /s
HW_{peak}	137.31	m
TW_{peak}	137.31	m
ΔH_{peak}	0.00	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation**C. Benchmark**

Description	CL of Steel Deck
Coordinates	366020.04, 4926135.68
Elevation	135.4

CULVERT/BRIDGE DATA

Structure ID	13
Location	Buck Bay Road
UTM Coordinates	370464, 4936882
Type	Bridge
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	162.89	m
Invert	162.50	m
Number of Openings	1	
Low Point of Road	162.74	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	12.20	m ³ /s
HW_{peak}	163.26	m
TW_{peak}	163.17	m
ΔH_{peak}	0.09	m
Max. Relief Flow Depth	0.52	m

B. Photographic Presentation**C. Benchmark**

Description	BM cut cross top dam between north and middle logbay
Coordinates	369766.085, 4931605.917
Elevation	151.35

CULVERT/BRIDGE DATA

Structure ID	15
Location	Westport Road near Glendower
UTM Coordinates	370467, 4935677
Type	CSP Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	160.85	m
Invert	158.64	m
Number of Openings	1	
Low Point of Road	160.68	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	21.90	m^3/s
HW_{peak}	161.14	m
TW_{peak}	160.13	m
ΔH_{peak}	1.01	m
Max. Relief Flow Depth	0.46	m

B. Photographic Presentation**C. Benchmark**

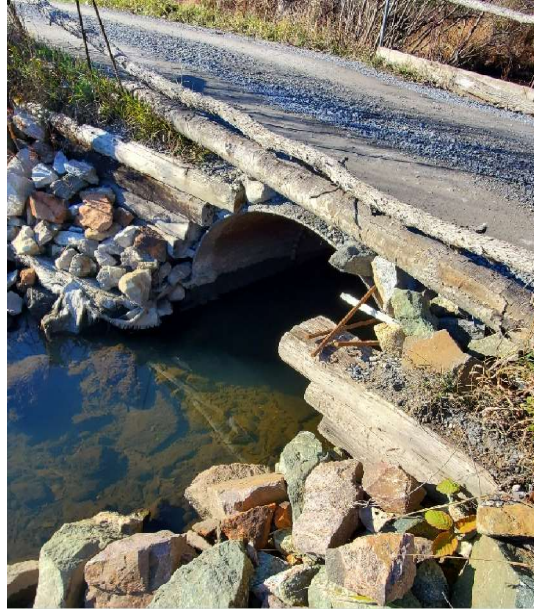
Description	Obvert of 1800 CSP
Coordinates	370466.81, 4935672.82
Elevation	160.29

CULVERT/BRIDGE DATA

Structure ID	16
Location	Brooks Lane
UTM Coordinates	370892, 4935286
Type	CSP Arch Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	158.54	m
Invert	157.11	m
Number of Openings	1	
Low Point of Road	158.13	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	16.00	m^3/s
HW_{peak}	158.62	m
TW_{peak}	158.40	m
ΔH_{peak}	0.22	m
Max. Relief Flow Depth	0.49	m

B. Photographic Presentation**C. Benchmark**

Description	Soffit
Coordinates	370894.06, 4935290.12
Elevation	158.57

CULVERT/BRIDGE DATA

Structure ID	17
Location	Bunker Hill Road
UTM Coordinates	371226, 4934750
Type	CSP Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	152.90	m
Invert	150.34	m
Number of Openings	1	
Low Point of Road	152.22	m
Governing 1% AEP Event	Rainfall	
Governing 1% AEP Q_{peak}	14.40	m ³ /s
HW_{peak}	152.68	m
TW_{peak}	152.14	m
ΔH_{peak}	0.54	m
Max. Relief Flow Depth	0.46	m

B. Photographic Presentation**C. Benchmark**

Description	1800 CSP Obvert
Coordinates	371230.56, 4934754.90
Elevation	152.48

CULVERT/BRIDGE DATA

Structure ID	18
Location	Sperling Road
UTM Coordinates	372984, 4938747
Type	CSP Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	176.24	m
Invert	174.53	m
Number of Openings	1	
Low Point of Road	175.41	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	1.00	m ³ /s
HW_{peak}	175.35	m
TW_{peak}	174.93	m
ΔH_{peak}	0.42	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation**C. Benchmark**

Description	Obvert of 1600 CSP
Coordinates	372993.41, 4938736.61
Elevation	176.39

CULVERT/BRIDGE DATA

Structure ID	19
Location	McColl Lane
UTM Coordinates	372339, 4937417
Type	CSP Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	154.93	m
Invert	153.53	m
Number of Openings	2	
Low Point of Road	154.59	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	3.70	m ³ /s
HW_{peak}	155.17	m
TW_{peak}	155.16	m
ΔH_{peak}	0.01	m
Max. Relief Flow Depth	0.58	m

B. Photographic Presentation**C. Benchmark**

Description	"good" Obvert of Culvert circle csp
Coordinates	372375.59, 4937441.71
Elevation	154.62

CULVERT/BRIDGE DATA

Structure ID	21
Location	Bunker Hill Road
UTM Coordinates	371480, 4934711
Type	CSP Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	151.90	m
Invert	150.20	m
Number of Openings	1	
Low Point of Road	151.84	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	0.10	m ³ /s
HW_{peak}	151.56	m
TW_{peak}	151.56	m
ΔH_{peak}	0.00	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation



C. Benchmark

Description	Obvert of 1200 CSP
Coordinates	371483.02, 4934715.16
Elevation	151.66

CULVERT/BRIDGE DATA

Structure ID	22
Location	Hamilton Lane
UTM Coordinates	369758, 4931596
Type	Dam with three stoplog bays
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	151.70	m
Invert	150.18	m
Number of Openings	3	
Low Point of Road	151.40	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	8.10	m^3/s
HW_{peak}	151.50	m
TW_{peak}	150.43	m
ΔH_{peak}	1.07	m
Max. Relief Flow Depth	0.10	m

B. Photographic Presentation



C. Benchmark

Description	Centerline Hamilton Lane
Coordinates	369702.38, 4931605.48
Elevation	152.03

CULVERT/BRIDGE DATA

Structure ID	<u>23</u>
Location	<u>Hichinbrooke Road North</u>
UTM Coordinates	<u>367504, 4929867</u>
Type	<u>CSP Culvert</u>
Conversion to CGVD28	<u>+0.33</u>

A. Specifications

Top of Road Elevation at Structure	<u>148.31</u>	m
Invert	<u>145.60</u>	m
Number of Openings	<u>1</u>	
Low Point of Road	<u>147.76</u>	m
Governing 1% AEP Event	<u>Spring melt</u>	
Governing 1% AEP Q_{peak}	<u>9.30</u>	m^3/s
HW_{peak}	<u>148.16</u>	m
TW_{peak}	<u>137.42</u>	m
ΔH_{peak}	<u>10.74</u>	m
Max. Relief Flow Depth	<u>0.00</u>	m

B. Photographic Presentation

Upstream



Downstream

**C. Benchmark**

Description	<u>Obvert of Culvert</u>
Coordinates	<u>367505.24, 4929866.03</u>
Elevation	<u>147.16</u>

CULVERT/BRIDGE DATA

Structure ID	24
Location	Hichinbrooke Road
UTM Coordinates	367001, 4926814
Type	CSP Culvert
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	138.53	m
Invert	135.44	m
Number of Openings	1	
Low Point of Road	137.81	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	2.20	m ³ /s
HW_{peak}	137.40	m
TW_{peak}	137.31	m
ΔH_{peak}	0.09	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation (Coming)**C. Benchmark**

Description	Obvert of Culvert
Coordinates	367010.43, 4926832.95
Elevation	138.18

CULVERT/BRIDGE DATA

Structure ID	26
Location	Highway 38 near Verona
UTM Coordinates	365713, 4928437
Type	Bridge
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	139.05	m
Invert	135.23	m
Number of Openings	1	
Low Point of Road	137.86	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	2.10	m^3/s
HW_{peak}	137.42	m
TW_{peak}	137.42	m
ΔH_{peak}	0.00	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation



C. Benchmark

Description	CL of Road
Coordinates	365133.29, 4925571.37
Elevation	139.27

CULVERT/BRIDGE DATA

Structure ID	27
Location	K&P Trail near Verona
UTM Coordinates	365059, 4925559
Type	Bridge
Conversion to CGVD28	+0.33

A. Specifications

Top of Road Elevation at Structure	139.09	m
Invert	134.68	m
Number of Openings	1	
Low Point of Road	138.45	m
Governing 1% AEP Event	Spring melt	
Governing 1% AEP Q_{peak}	33.20	m ³ /s
HW_{peak}	136.48	m
TW_{peak}	136.48	m
ΔH_{peak}	0.00	m
Max. Relief Flow Depth	0.00	m

B. Photographic Presentation**C. Benchmark**

Description	Centerline of top of bridge
Coordinates	365060.71, 4925558.19
Elevation	139.35

Appendix K: Computational Mesh Spacing for 2D Flow Areas



FID	Name	Near Spacing	Near Repeats	Far Spacing
0	Reach 2 Creek 6	5	2	
1	Reach 2 Creek 7	5	1	
2	Reach 6 Creek 2	5	2	10
3	Reach 6 Creek 4	5	1	
4	Reach 6 Creek 6	10	1	
5	Reach 6 Creek 7	5	1	
6	Reach 6 Creek 9	5	2	20
7	Reach 6 Creek 10	5	1	
8	Reach 9 Creek 1	10	1	
9	Reach 9 Creek 2	10	1	
10	Reach 1 Creek 1	10	1	
11	Reach 1 Creek 2	5	2	
12	Reach 6 Creek 8	5	1	20
13	Reach 2 Creek 4	5	2	10
14	Cty Rd 38 CL	5	2	20
15	Private Crossing US Cole Lake	5	1	
16	Inflow Channel to Cole Lake	10	2	
17	Reach 2 Creek 2	5	2	
18	Cty Rd 38 CL2	5	2	10
19	Westport Rd CL	5	2	10
20	Private Driveaway CL	5	3	
21	Craig Rd CL	5	4	10
22	Reach 6 Creek 3	5	1	
23	Private Rd CL Off Dsrt Lk Rd	5	1	
24	Bunker Hill Rd CL	5	3	10
25	Breakline 4	3	2	10
26	Weir 10 CL	3	1	10
27	Breakline 44	5	1	
28	McCull Ln CL	5	2	
29	Sterling Rd CL	5	1	10
30	Breakline 46	1	1	
31	Breakline 50	5	1	
32	Breakline 55	5	1	10
33	Breakline 54	5	1	10
34	Buck Bay Rd CL	6	3	20
35	Bunker Hill Rd CL2	5	2	10
36	Brooks Ln CL	5	2	10
37	Bunker Hill Rd CL3	5	1	20
38	S Frontenac Rd 8 CL	5	3	10
39	Private Crossing 12 CL	5	1	10
40	Private Crossing 11 CL	5	1	
41	Private Crossing 9 CL	10	1	10
42	Reach 10 Crossing	5	2	20
43	Reach 1 Creek 3	2	2	

FID	Name	Near Spacing	Near Repeats	Far Spacing
44	Breakline 28	5	1	
45	St Andrews Lake Ln CL	5	1	20
46	K&P Trail Near Cole Lake	5	1	20
47	Private Crossing DS White Lake	5	2	10
48	Breakline 58	3	3	
49	Bvr Dam DS Buck Bay Rd	10	1	10
50	Reach 6 Creek 5	10	1	20
51	Bvr Dam US 13 Ist Lk Inlet	5	1	10
52	Reach 6 Creek 11	5	1	10
53	Reach 2 Creek 1	5	2	
54	Reach 6 Creek 1	5	2	10
55	Breakline 86	10	1	
56	Reach 2 Creek 8	5	1	10
57	Breakline 21	5	1	
58	Breakline 42	10	3	
59	Breakline 45	10	1	
60	Reach 2 Creek 3	10	1	
61	Reach 3 Crossing 1		0	
62	Breakline 57	3	1	5
63	Reach 3 Crossing 2	5	3	20
64	14 Island Lake Weir	3	1	
65	K&P Trail near Godfrey2	2.5	3	10
66	13 Island Lk Dm	5	1	10
67	Breakline 34	3	3	10
68	Breakline 43	5	2	
69	Reach 2 Creek 5	10	2	
70	Old Mine Ln	4	3	
71	Private Crossing 9	4	3	
72	Breakline 72	3	2	
73	Breakline 74	3	2	
74	Breakline 78	5	3	
75	Breakline 79	10	4	
76	Breakline 9	2	1	
77	Breakline 6	2	1	
78	Breakline 26	2	1	
79	Breakline 36	2	1	
80	Breakline 39	1	1	
81	Breakline 13	1	1	
82	Breakline 81	1	1	
83	Breakline 11	2	1	
84	Breakline 12	2	1	
85	Reach 11 Breakline	1	1	
86	Breakline 2	2	1	